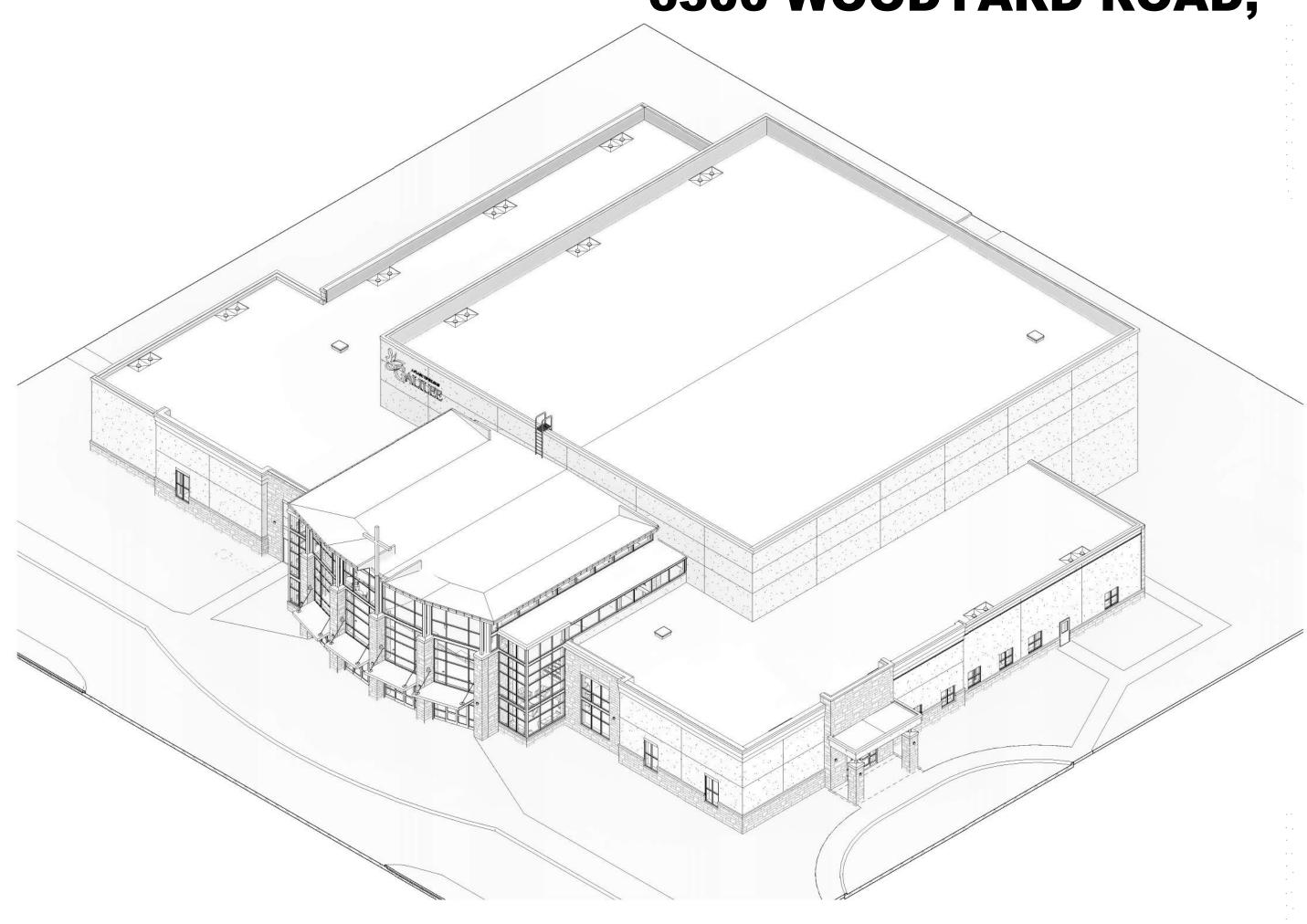
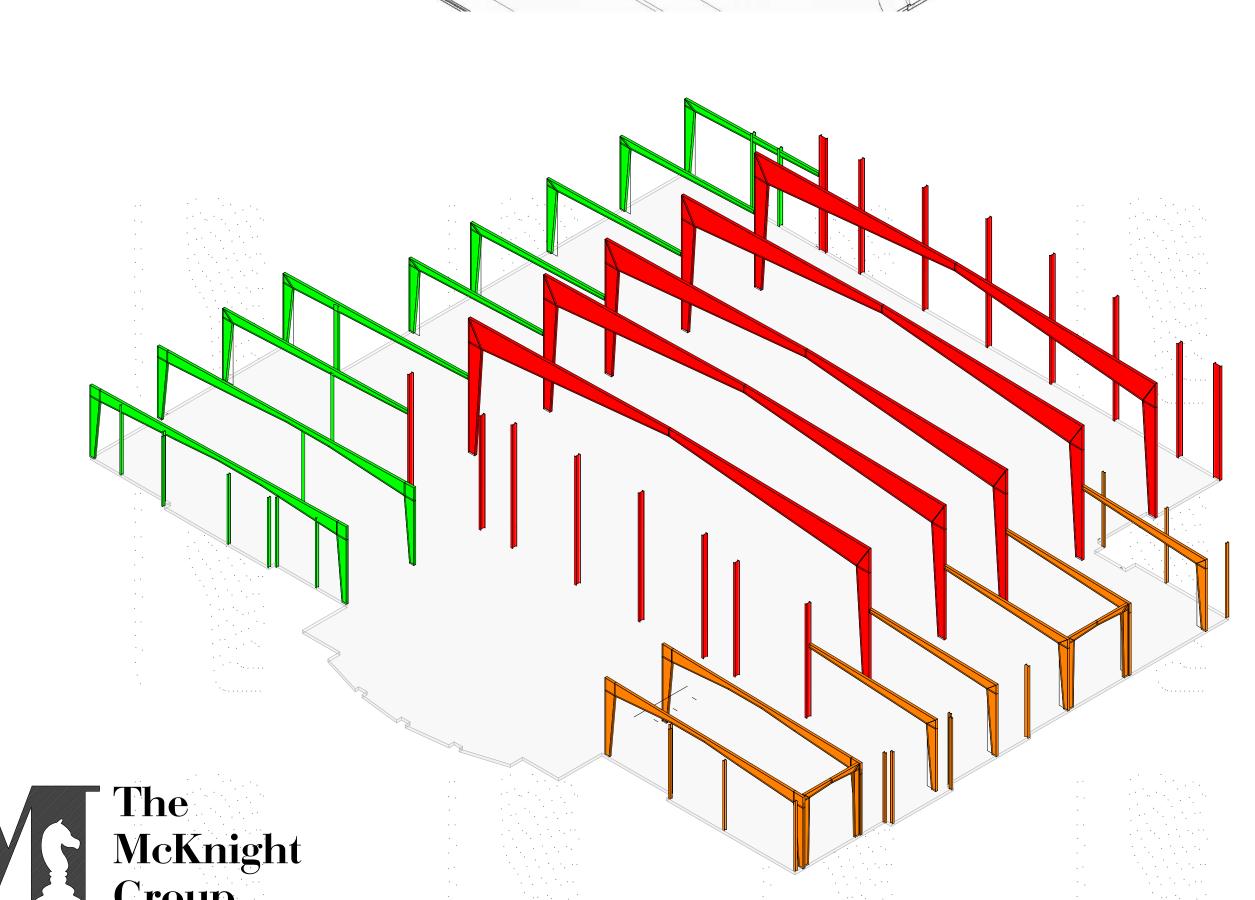
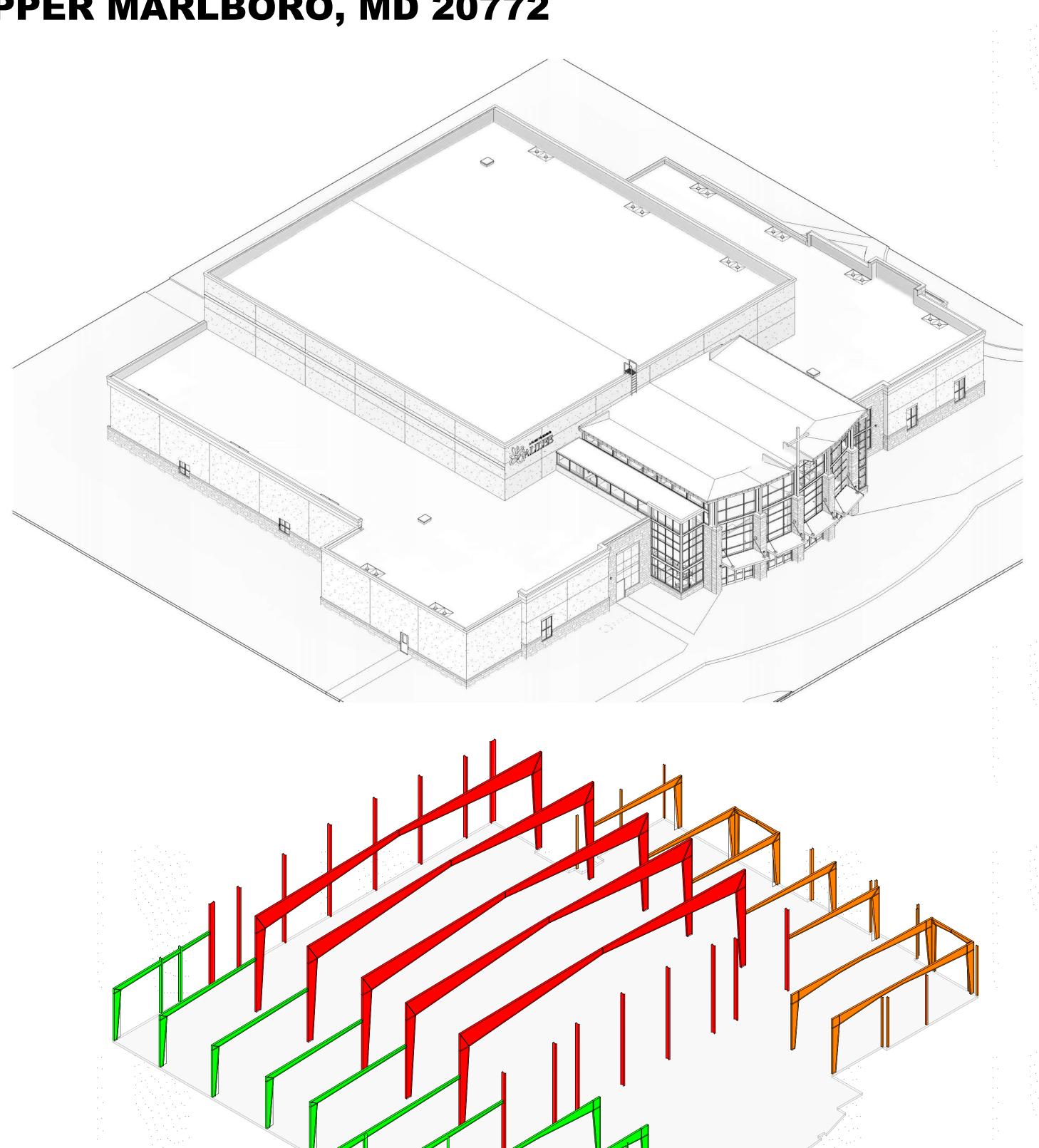
GALILEE BAPTIST CHURCH

6300 WOODYARD ROAD,

UPPER MARLBORO, MD 20772







CODED NOTES - PEMB CURTAIN LEG - 6' @ 17 plf = 100LB TOTAL LIGHT BAR; APPROX WEIGHT 30 PLF (DESIGN ONLY) 46' LONG = 1,380LB TOTAL X-BRACING PROPOSED FOR THIS FRAME PORTAL FRAME PROPOSED FOR THIS BAY CHOIR MONITOR SPEAKER -50LB (2 POINT LOADS) SPEAKER ARRAY - 250LB (2 POINT LOADS) SUBWOOFER ARRAY - 300LB (2 POINT LOADS) PROJECTOR - 100LB POINT LOAD OPERABLE PARTITION 9'-0" HIGH (8LB/SF = 72LBS/ LF) 10 PIPE BATTON - 35' @ 25 plf = 750LB TOTAL PROVIDE NON-TAPERED "SUPERMARKET" COLUMNS FOR THIS MAINFRAME WALLS IN THIS AREA WITH LOADS THAT ARE DIRECTED INTO THE PEMB FRAME. SEE STRUCTURAL DRAWINGS FOR LOADS BEING TRANSFERRED TO THE PEMB FRAME MEMBRANE ROOF OVER RIGID INSULATION OVER METAL DECK 3/8"/1'-0" SLOPE ROOF IN THIS AREA WITH LOADS THAT ARE DIRECTED INTO THE PEMB FRAME. SEE STRUCTURAL DRAWINGS FOR LOADS BEING TRANSFERRED TO THE PEMB STRUCTURE ROOF TOP UNIT ABOVE, LOADS AS INDICATED ROOF ACCESS HATCH - 36" X 30" 16 17 SIGNAGE BY OTHERS TRADITIONALLY FRAMED PORTION OF BUILDING IN THIS AREA, SEE STRUCTURAL DRAWINGS FOR LOADS BEING TRANSFERRED TO THE PEMB STRUCTURE OVERHEAD SECTIONAL DOOR BY OTHERS, SIZED AS INDICATED 20 MAN DOOR BY OTHERS, SIZED AS INDICATED 8" OUTSET WALL GIRTS. PEMB FINISH IS EIFS OVER 5/8" EXTERIOR SHEATHING, OVER 1 1/2" METAL DECK (BY OTHERS), WEIGHT 15 PSF, MAX DEFLECT L/240 8" OUTSET WALL GIRTS, STARTING AT 21"-1". PEMB FINISH IS EIFS OVER 5/8" EXTERIOR SHEATHING, OVER 1 1/2" METAL DECK (BY OTHERS), WEIGHT 15 PSF, MAX DEFLECT L/240 8" OUTSET WALL GIRTS, STARTING AT 21-6". PEMB FINISH IS EIFS OVER 5/8" EXTERIOR SHEATHING, OVER 1 1/2" METAL DECK (BY OTHERS), WEIGHT 15 PSF, MAX DEFLECT L/240 8" OUTSET WALL GIRTS, STARTING AT 100"-0". PEMB FINISH IS CULTURED STONE VENEER OVER 5/8" EXTERIOR SHEATHING, OVER 1 1/2" METAL DECK (BY OTHERS), 8" OUTSET WALL GIRTS, STARTING AT 100-0". PEMB FINISH IS 1 1/2" METAL WALL PANELS, MAX DEFLECT L/240 SPEAKER ARRAY - 350LB (2 POINT LOADS) LIGHT BAR; APPROX WEIGHT 30 PLF (DESIGN ONLY) 86' LONG = 2,580LB TOTAL LIGHT BAR; APPROX WEIGHT 30 PLF (DESIGN ONLY) 48' LONG = 1,440LB TOTAL 29 LIGHT BAR; APPROX WEIGHT 30 PLF (DESIGN ONLY) 60' LONG = 1,800LB TOTAL

GENERAL NOTES

A. ALL WALL CONSTRUCTION TO BE GIRTS, U.N.O. B. ALL END WALL COLUMNS TO BE STRAIGHT, U.N.O. C. ALL MAINFRAMES TO BE TAPERED COLUMNS, U.N.O. D. ALL PURLINS ASSUMED TO BE MAX. 10". E. FIRST GIRT TO BE "C" GIRT, TOES UP, U.N.O.

F. AT FRAMED WINDOW OPENINGS, BOTTOM GIRT TO BE TOES DOWN.

G. ALL CHANNELS AT TOP OF PARAPET WALLS TO BE TOES DOWN, U.N.O.

H. X-BRACING AND PORTAL FRAME LOCATIONS SHALL BE AS SHOWN ON PLANS. IF ADDITIONAL FRAMES OR BRACING ARE REQUIRED OR THEIR LOCATIONS CHANGED, ARCHITECT SHALL BE NOTIFIED IMMEDIATELY AND CONFLICT RESOLVED BEFORE FINALIZING DESIGN.

I. PORTAL FRAMES SHALL BE ATTACHED DIRECTLY TO THE MAINFRAME AND HELD 6" A.F.F. J. NO FLANGE BRACING BELOW 15'-0".

THE LOCATION AND ORIENTATION OF THE METAL BUILDING COLUMNS AS SHOWN ON THIS PLAN HAVE BEEN CAREFULLY CONSIDERED AND CURRENTLY WORK WITH THE ARCHITECTURAL AND STRUCTURAL DESIGN. ANY CHANGES MAY RESULT IN A REDESIGN OF THE STRUCTURE AND A CHANGE IN LAYOUT OF THE ARCHITECTURE. THEREFORE, IF THE METAL BUILDING DESIGNER REQUIRES A CHANGE FROM THIS SET OF DRAWINGS, PLEASE CALL THE MCKNIGHT GROUP AT (614) 875-1689. FAILURE TO ADDRESS CHANGES EARLY IN THE PROCESS MAY RESULT IN REJECTION OF THE METAL BUILDING DESIGN.

NOTE: PRE-ENGINEERED METAL BUILDING WAS DESIGNED PER CECO METAL BUILDING MANUFACTURER'S SPECIFICATIONS. SHOULD ANOTHER MANUFACTURER BE SELECTED FOR THIS PROJECT, IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR (GC) TO MAKE ALL NECESSARY ADJUSTMENTS IN THE FIELD TO ACCOMODATE ANY/ALL DIFFERENCES. ANY ADDITIONAL DRAWINGS THAT ARE REQUESTED BY THE GC TO AID IN THIS PROCESS SHALL COME AT THE EXPENSE OF THE GC, PAID TO THE ARCHITECT.

PEMB DESIGN LOADS -

| 1. | ROOF LIVE LOADS a. MINIMUM ROOF LIVE LOAD (W/ REDUCTIONS) = 20PSF (reducible) b. DESIGN ROOF LIVE LOAD = 30 PSF |
|----|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 2. | ROOF SNOW DESIGN PARAMETERS a. GROUND SNOW LOAD Pg = 35 PSF b. FLAT-ROOF SNOW LOAD Pf = 27 PSF c. LOW SLOPE ROOF LOAD Pm = 22 PSF d. SNOW LOAD IMPORTANCE FACTOR IS = 1.1 e. SNOW EXPOSURE FACTOR, Ce = 1.0 f. THERMAL FACTOR, Ct (FOR MAIN ROOF) = 1.0 g. THERMAL FACTOR, Ct (FOR CANOPIES) = 1.2 h. ALL APPLICABLE EFFECTS DUE TO SNOW DRIFTING |
| 3. | WIND LOAD PARAMETER (ASCE 7-16) a. RISK CATEGORY III b. BASIC WIND SPEED (V) = 120 MPH c. BASIC WIND SPEED ASD (V-ASD) = 93 MPH d. WIND EXPOSURE CATEGORY = C e. INTERNAL PRESSURE COEFFICIENT, (GCpi) = +/-0.18 |
| 4. | SEISMIC DESIGN PARAMETERS a. SEISMIC RISK CATEGORY = III b. SITE CLASS = D c. MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION AT 0.2 SECOND PERIOD, Ss = 0.140 d. MAXIMUM CONSIDERED EARTHQUAKE GROUND MOTION AT 1.0 SECOND PERIOD, S1 = 0.052 e. SEISMIC LOAD IMPORTANCE FACTOR, Ie = 1.25 f. SEISMIC DESIGN CATEGORY = B g. ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE h. SEISMIC-FORCE-RESISTING SYSTEM R = 3 (NO SPECIAL SEISMIC DETAILING REQUIRED) i. DESIGN BASE SHEAR = 25 KIPS |
| 5. | COLLATERAL DEAD LOAD (SPRINKLERS, DUCTS, LIGHTS, CEILINGS) 8 PSF |
| 6. | DEFLECTION LIMITATIONS: a. FRAME DEFLECTION b. GIRT DEFLECTION (WIND) EIFS AND STONE = L/360 c. ROOF DEFLECTION i. DL = L/180 ii. DL + LL = L/240 |

