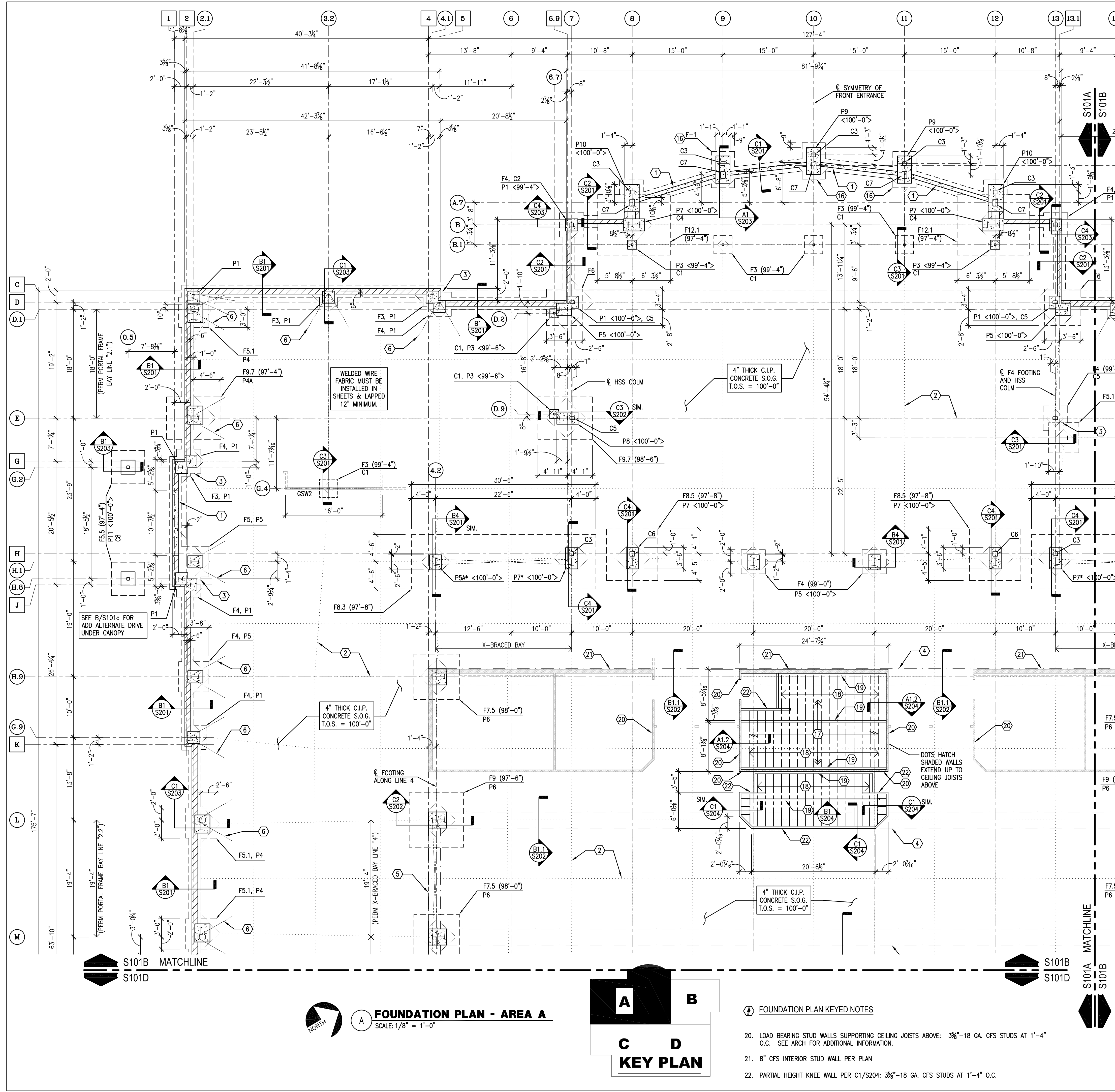
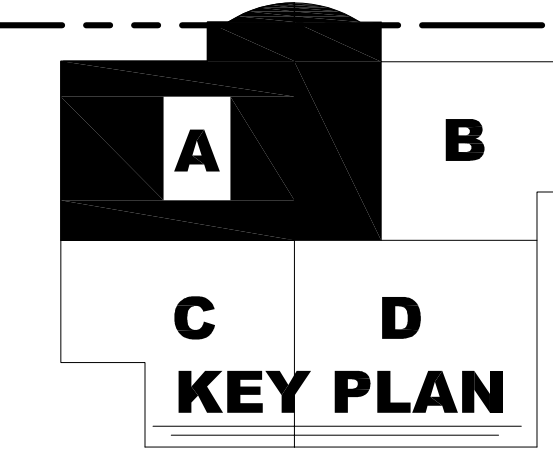


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- FOUNDATION PLAN NOTES:**
- SEE SHEET S001 FOR STRUCTURAL GENERAL NOTES AND LEGEND. SEE SHEET S101A FOR FOUNDATION PLAN KEYED NOTES, AND FOOTING, PIER, AND COLUMNS SCHEDULES.
 - REFERENCE ELEVATION 100'-0" = TOP OF SLAB ON GRADE. SEE CIVIL SITE PLAN FOR U.S.G.S ELEVATION.
 - FOOTINGS SHALL BE FOUNDED ON STABLE NATURAL SOILS OR NEW CONTROLLED FILLS PLACED DIRECTLY OVER STABLE NATURAL SOILS. AT THE ELEVATIONS SHOWN. WORK WITH A PRESUMED BEARING CAPACITY OF 1.5 KSF. A FINAL GEOTECHNICAL INVESTIGATION WHICH VERIFIES THE ABOVE ASSUMPTIONS MUST BE PROVIDED PRIOR TO CONSTRUCTION. THE REPORT MUST PROVIDE SITE PREPARATION REQUIREMENTS AND RECOMMENDATIONS INCLUDING THE BASE PREPARATION FOR THE SLAB ON GRADE.
 - ALL FOOTING EXCAVATIONS SHALL BE INSPECTED & APPROVED BY A QUALIFIED SOILS ENGINEER FOR THE BEARING CAPACITY INDICATED ABOVE PRIOR TO PLACING CONCRETE.
 - KEEP FOUNDATION EXCAVATIONS FREE OF WATER AT ALL TIMES. REPLACE SOFT OR WEAKENED SOIL WITH CLASS IV CONCRETE.
 - ALL EXTERIOR FOOTINGS SHALL BEAR MIN. 3'-0" BELOW ADJACENT GRADE. ADJUST BOTTOM OF FOOTING AS REQUIRED TO MAINTAIN FROST DEPTH.
 - ELEVATIONS SHOWN AT FOOTINGS ARE TOP OF FOOTING ELEVATION (T.O.F.).
 - ALL EXTERIOR TOP OF FOOTING (T.O.F.) = 97'-4", U.N.O.
 - TOP OF PIER ELEVATION <T.O.P.> FOR CONVENTIONAL STEEL COLUMNS = 99'-4", U.N.O. TOP OF PIER <T.O.P.> FOR P.E.M.B. COLUMNS = 100'-0", U.N.O.
 - FLOOR CONSTRUCTION = 4" THICK CONCRETE SLAB ON GRADE REINFORCED WITH WMF 6x6-W2.1xW2.1 AT MID-DEPTH, U.N.O., OVER 4 INCHES OF WELL GRADED CRUSHED STONE OR GRAVEL SUCH AS AASHTO #57. WELDED WIRE FABRIC IS PART OF THE STRUCTURAL LATERAL RESTRAINT SYSTEM AND MUST BE PROPERLY INSTALLED WITH 12" MINIMUM LAP SPLICES. SEE DETAIL A1/S201 FOR TYPICAL CONCRETE SLAB-ON-GRADE CONSTRUCTION.
 - TOP OF SLAB ELEVATION [T.O.S.] = 100'-0", U.N.O. COORDINATE WITH ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MINOR DEPRESSIONS AND SLOPES TO DRAIN NOT SHOWN ON FOUNDATION PLAN.
 - FOUNDATION WALL CONSTRUCTION: REINFORCED CMU WALLS WITH #5 VERTICAL BARS AT 48 INCHES O.C., U.N.O. SEE DETAIL A3/S201 FOR TYPICAL REINFORCED CMU WALL CONSTRUCTION.
 - ALL EXPANSION BOLTS, ADHESIVE ANCHORS, SIMPSON TITEN HD ANCHORS, ETC., MUST BE INSTALLED INTO 100% SOLID GROUTED CMU OR CAST-IN-PLACE CONCRETE WITH A MINIMUM OF 4" EDGE DISTANCE ALL AROUND.
 - ANCHOR BOLTS FOR P.E.M.B. COLUMNS SHALL HAVE A MINIMUM OF 2x BOLT PROJECTION ABOVE PIER. BOLT DIAMETER DESIGN FOR UPLIFT AND SHEAR SHALL BE PROVIDED BY THE P.E.M.B. MANUFACTURER. REQUIRED BOLT "EMBEDMENT" LENGTHS ARE LISTED BELOW (GRADE 36). TOTAL LENGTH OF BOLT = EMBEDMENT + PROJECTION.
 - RIGID FRAME COLUMN BASE PLATES BEARING ON CONCRETE PIERS OR ENDWALL COLUMNS THAT ACCEPT STEEL ROD X-BRACING OR BASE PLATES OF PORTAL FRAMES. STOP ANCHOR BOLTS 1" ABOVE FOOTING SURFACE. SEE PLAN FOR RESPECTIVE PIER AND FOOTING ELEVATIONS. MINIMUM ANCHOR BOLT EMBEDMENT IS PROVIDED BELOW.
 - 3/4" DIA. x 1'-11"
 - 1" DIA. x 1'-11"
 - 1-1/4" DIA. x 1'-11"
 - ENDWALL COLUMN BASE PLATES BEARING ON CONCRETE PIERS THAT DO NOT ACCEPT STEEL ROD X-BRACING OR INTERIOR COLUMNS BEARING ON ISOLATED SPREAD FOOTINGS.
 - 3/4" OR 5/8" DIA. x 1'-0"
 - FOR CONVENTIONALLY FRAMED STEEL COLUMNS, PROVIDE MIN. OF 4" OF ANCHOR BOLT PROJECTION ABOVE PIER OR FOOTING.
 - TRIANGULAR SYMBOL INDICATES FOOTING STEP PER A2/S201 AS NEEDED TO MAINTAIN LOCAL FROST DEPTH AS PROVIDED IN FOUNDATION PLAN GENERAL NOTE "G" ABOVE. FOOTING STEPS MAY OR MAY NOT BE REQUIRED, AND SHOULD BE COORDINATED WITH THE FINAL CIVIL/SITE GRADING PLANS.
 - ALL ANCHOR BOLTS SHALL TERMINATE WITH A HEAVY HEX NUT AT THE EMBEDDED END PER A4/S201. HOOKED ANCHOR BOLTS ARE NOT PERMITTED.
 - GRAY SHADED HATCHED WALLS WITH THE FOLLOWING NOTATION, "GSW1" OR "GSW2", IDENTIFIES LOAD-BEARING OR LATERAL RESISTING CFS STUD WALLS. REFER TO ROOF FRAMING PLAN AND DETAIL A4/S203 FOR MORE INFORMATION.
- FOUNDATION PLAN KEYED NOTES**
- OMIT TOP COURSE OF MASONRY AND POUR SLAB THRU AT DOOR OPENINGS, PER C1/S201 UNLESS ANOTHER DETAIL IS SPECIFIED.
 - SAW-CUT CONTROL JOINTS PER A1.1 OR A1.2/S201. LOCATE GENERALLY AT CENTERLINES OF COLUMNS, AND RE-ENTRANT CORNERS, AT A MAXIMUM SPACING OF 15'-0".
 - CAST BOTH SPREAD FOOTINGS TOGETHER AND OVERLAP REINFORCING.
 - 2'-8" WIDE x 12" THICK CONCRETE TIE-ROD ENCASUREMENT CAST INTEGRAL WITH SLAB ON GRADE WITH (3) #8 BARS AND (2) #7 BARS CONTINUOUS. REBAR COUPLERS ARE REQUIRED AT ALL SPLICES; DO NOT LAP SPLICE BARS. SEE B1/S202 FOR RELATED DETAILS.
 - 7" THICKENED SLAB x 2'-0" WIDE W/ (2) #6 TIE-RODS CONT. (LOCATED 4" BELOW TOP OF SLAB), SPANNING BETWEEN PIERS AT X-BRACED FRAMES ALONG LINES 4 & 16 ONLY. SEE B1/S202 FOR PLAN DETAIL.
 - #5 HAIR-PIN REBAR CAST INTO SLAB ON GRADE PER DETAIL C1/S202.
 - PLATFORM FLOOR CONSTRUCTION (LIMITS ARE DEFINED BY DOTS HATCH PATTERN): 3" CONCRETE OVER 2'-20 GA. COMPOSITE DECK W/ 6x6-W2.9xW2.9 WMF 2" BELOW TOP OF SLAB. OVERALL THICKNESS = 5". TOP OF SLAB = 102'-9 1/2". THE COMPOSITE FLOOR DECK HAS BEEN DESIGNED TO SUPPORT THE WEIGHT OF A 4,000# (TOTAL) SCISSORS LIFT WITH A WHEEL BASE AS FOLLOWS: 2'-6" x 6'-0". NOTIFY THE ENGINEER OF RECORD FOR SCISSORS LIFTS THAT VARY FROM THESE SPECIFICATIONS.
 - 8" CMU SUPPORT WALL WITH HORIZONTAL REINFORCING, NOT VERTICAL REINFORCING, BEARING ON TOP OF 6" THICK CONCRETE FLOOR SLAB PER NOTE 14 BELOW.
 - 8x16 CMU PIER EA. SIDE OF P.E.M.B. COLUMN TO SUPPORT ENDS OF DECK SUPPORT ANGLE PER KEYED NOTE 10 BELOW.
 - CONT. L5x 3x 3/8 DECK SUPPORT ANGLE AT EDGE OF PLATFORM.
 - (2) L3 1/2 x 4 x 3/8 STEEL LINTELS (LLV - LLBB). PROVIDE 6" BEARING LENGTH, E.E. AND MAINTAIN 2'-0" CLEAR VERTICAL OPENING HEIGHT BENEATH BOTTOM SIDE OF LINTELS.
 - TURN-DOWN SLAB AT EDGE OF BAPTISTERY PIT. VERTICAL FACE TERMINATES INTO MAT FOOTING BELOW.
 - 8" THICK MAT FOOTING W/ #5 BARS AT 12" O.C., E.W. AT MID-DEPTH (T.O.F. = 97'-10")
 - EDGE OF 6" THICK CONCRETE SLAB ON GRADE W/ W6x6-W2.9xW2.9 W.M.F. AT MID-DEPTH. SUPPORT ON CHAIRS DURING CONCRETE POUR. W.W.F. SHEETS MUST BE LAPPED A MINIMUM OF 12".
 - TOP OF PIER ELEVATION AT P3 IS AT A LOWER ELEVATION AS COMPARED TO THE OTHER PIERS WHICH ARE ALL CAST INTEGRALLY TOGETHER. ADJUST VERTICAL REINFORCING AND TIES AS REQ'D TO FIT ALL PIER REINFORCING TOGETHER. SEE C3/S202 FOR DETAILS.
 - FOOTING TYPE F-1, SEE FOOTING SCHEDULE FOR REINFORCING. SIZE OF FOOTING IS 9" BEYOND THE LIMITS OF THE CONCRETE PIER ON EACH SIDE.
 - PLATFORM FLOOR CONSTRUCTION: 1/2" PLYWOOD, OVER 1/2" SOUNDBOARD, OVER 3/4" FRT PLYWOOD FLOOR DECKING, OVER COLD-FORMED STEEL JOISTS. TOP OF PLATFORM = 101'-9".
 - 6'-16 GA. COLD-FORMED STEEL (CFS) JOISTS AT 1'-4" O.C.
 - LOAD BEARING STUD WALLS PER A1 & B1/S204: 3 1/2"-20 GA. CFS STUDS AT 1'-4" O.C.

FOUNDATION PLAN - AREA A
SCALE: 1/8" = 1'-0"



- FOUNDATION PLAN KEYED NOTES**
- LOAD BEARING STUD WALLS SUPPORTING CEILING JOISTS ABOVE: 3 1/2"-18 GA. CFS STUDS AT 1'-4" O.C. SEE ARCH FOR ADDITIONAL INFORMATION.
 - 8" CFS INTERIOR STUD WALL PER PLAN
 - PARTIAL HEIGHT KNEE WALL PER C1/S204: 3 1/2"-18 GA. CFS STUDS AT 1'-4" O.C.

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STATE OF MARYLAND
CHRISTOPHER P. SEKOL
PROFESSIONAL ENGINEER
No. 26219
LICENSE EXPIRES: 06-21-2025
DRAWING ISSUED: 11-17-2023

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NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
6300 WOODYARD ROAD
UPPER MARLBORO, MD 20772

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REVISIONS

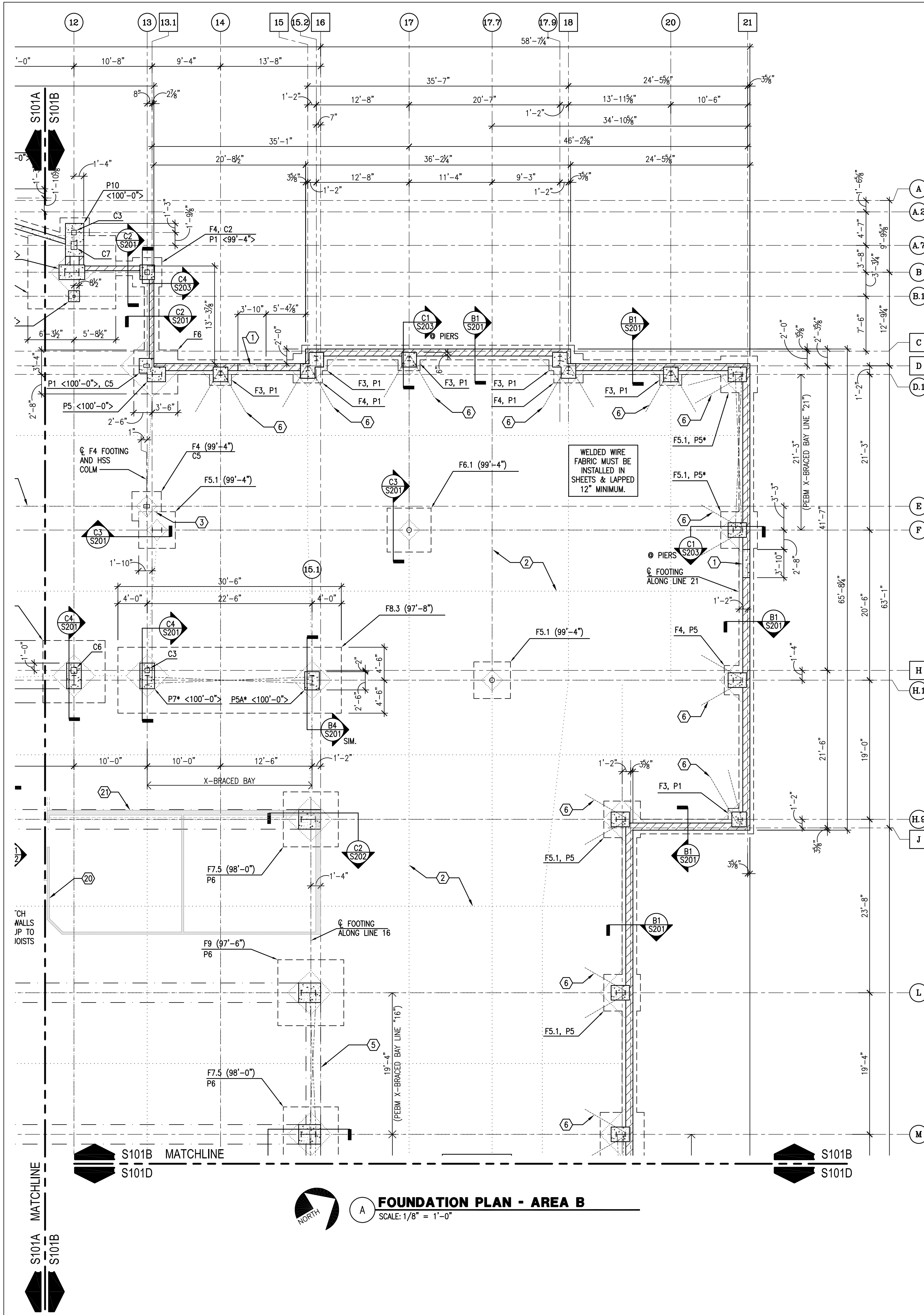
FOUNDATION PLAN

S101A

OF 18 SHEETS

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FOUNDATION PLAN - AREA B
SCALE: 1/8" = 1'-0"

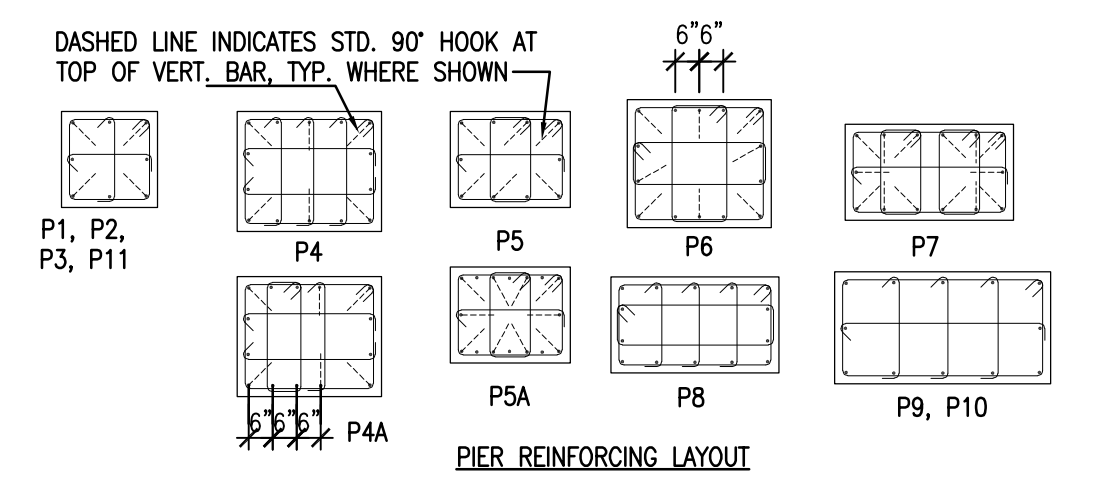
SPREAD FOOTING SCHEDULE

| MARK | SIZE | BOTTOM REINF. | TOP REINF. |
|-------|-----------------------------|---------------------|---------------------|
| F-1 | SEE PLAN | #5 @ 12" O.C., E.W. | |
| F3 | 3'-0" x 3'-0" x 12" THICK | (3) #5 E.W. | |
| F4 | 4'-0" x 4'-0" x 12" THICK | (4) #5 E.W. | |
| F5.1 | 5'-0" x 5'-0" x 14" THICK | (6) #5 E.W. | (6) #5 E.W. |
| F5.5 | 5'-6" x 5'-6" x 14" THICK | (6) #5 E.W. | (6) #5 E.W. |
| F6 | 6'-0" x 6'-0" x 14" THICK | (6) #5 E.W. | |
| F6.1 | 6'-0" x 6'-0" x 16" THICK | (8) #6 E.W. | (8) #5 E.W. |
| F7.5 | 7'-6" x 7'-6" x 18" THICK | (8) #6 E.W. | (8) #5 E.W. |
| F8.5 | 8'-6" x 8'-6" x 18" THICK | (9) #6 E.W. | (9) #6 E.W. |
| F8.30 | 8'-0" x 30'-6" x 20" THICK | #6 @ 10" O.C., E.W. | #6 @ 10" O.C., E.W. |
| F9.7 | 9'-0" x 7'-0" x 18" THICK | #6 @ 10" O.C., E.W. | #6 @ 10" O.C., E.W. |
| F9 | 9'-0" x 9'-0" x 20" THICK | (12) #6 E.W. | (12) #6 E.W. |
| F12.1 | 12'-0" x 10'-0" x 24" THICK | #6 @ 10" O.C., E.W. | #6 @ 10" O.C., E.W. |

PIER SCHEDULE

| MARK | SIZE | VERT. REINF. | TIES | REMARKS |
|------|---------------|--------------|---------------|------------------------|
| P1 | 2'-0" x 2'-0" | (4) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3 |
| P2 | 2'-0" x 2'-0" | (8) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4 |
| P3 | 1'-6" x 1'-6" | (4) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3 |
| P4 | 2'-6" x 3'-0" | (14) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5 |
| P4A | 2'-6" x 3'-0" | (14) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5, 6 |
| P5 | 2'-0" x 2'-6" | (10) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4 |
| P5A | 2'-0" x 2'-6" | (16) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5, 6 |
| P6 | 2'-8" x 3'-0" | (14) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5, 6 |
| P7 | 2'-0" x 3'-6" | (14) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5 |
| P8 | 2'-0" x 3'-6" | (14) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 5, 6 |
| P9 | 2'-2" x 4'-6" | (12) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3 |
| P10 | 2'-6" x 4'-6" | (12) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3 |
| P11 | 2'-4" x 2'-4" | (8) #7 | #4 @ 10" O.C. | NOTES 1, 2, 3, 4, 5 |

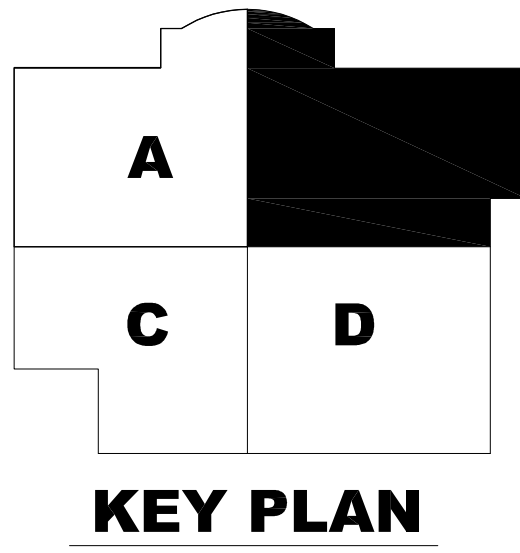
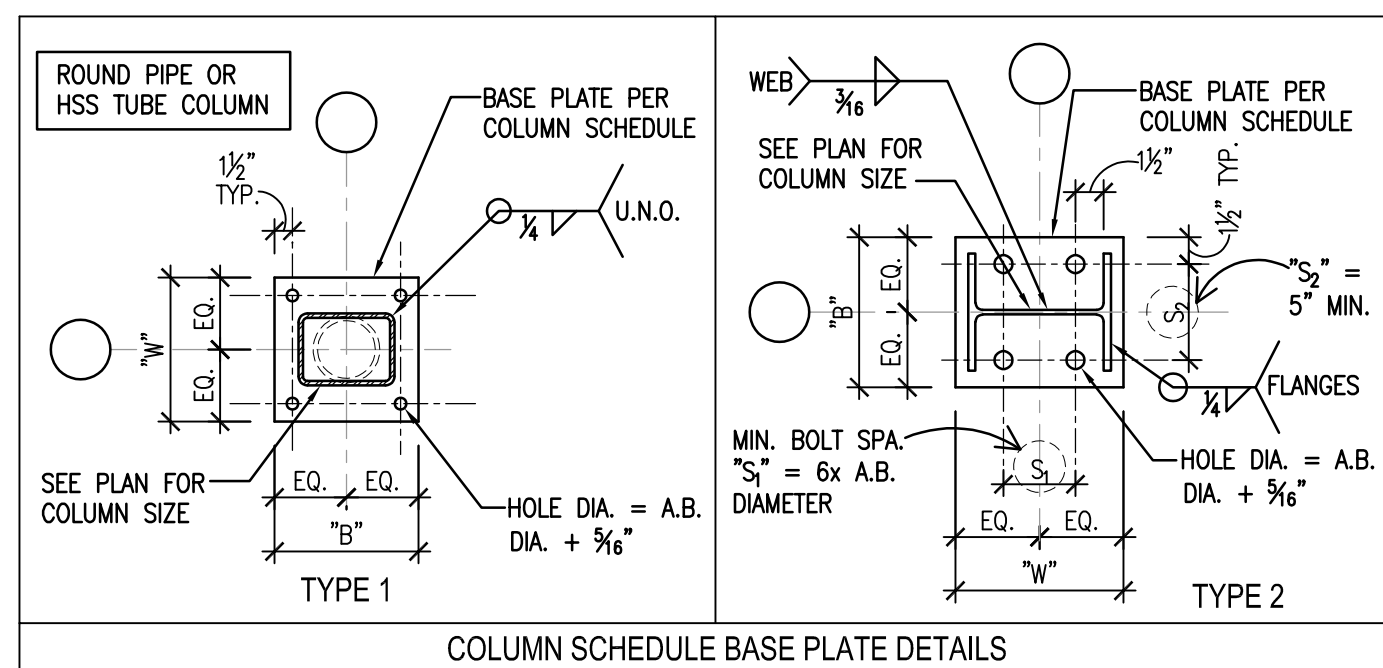
- PIER SCHEDULE NOTES:**
- SEE A4/S201 FOR TYPICAL ANCHOR BOLT DETAIL.
 - SEE DETAIL A1/S202 FOR TYPICAL PIER CONSTRUCTION.
 - SEE "PIER REINFORCING LAYOUT" BELOW FOR CONFIGURATION OF PIER REINFORCING.
 - WHERE PIERS ARE LABELED ON THE PLANS WITH AN ASTERISK (P2*), THE PIER VERTICAL CORNER BARS (AND SELECT MIDDLE BARS ON CERTAIN PIERS) SHALL TERMINATE W/ 90° HOOK AT TOP OF PIER, SHOWN AS A DASHED LINE ON "PIER REINFORCING LAYOUT" BELOW.
 - PROVIDE DOUBLE TIES AT TOP OF PIER.
 - PROVIDE TIES AT 4" O.C. IN THE TOP 1'-6" OF THE PIER HEIGHT.
 - ALL PIER VERTICAL BARS SHALL TERMINATE IN FOOTING WITH A STANDARD 90° HOOK WITH 3" CLEAR CONCRETE COVER BELOW THE HOOK TO THE BOTTOM OF FOOTING.



COLUMN SCHEDULE

| MARK | SIZE | BASE PLATE SIZE (T x B x W) | BASE PLATE TYPE | ANCHOR BOLTS |
|------|---------------|-----------------------------|-----------------|------------------------|
| C1 | HSS 4x4x1/4 | R 3/4 x 10" x 0'-10" | TYPE 1 | (4) 3/4" φ + 8" EMBED |
| C2 | HSS 8x8x3/8 | R 3/4 x 1'-2" x 1'-2" | TYPE 1 | (4) 3/4" φ + 8" EMBED |
| C3 | HSS 8x8x3/8 | R 3/4 x 1'-2" x 1'-0" | TYPE 1 | (4) 3/4" φ + 12" EMBED |
| C4 | W24x104 | R 1 1/4 x 1'-4" x 2'-8 1/2" | C4/S202 | (4) 1" φ + 31" EMBED |
| C5 | HSS 10x10x3/8 | R 1 x 1'-0" x 1'-0" | TYPE 1 | (4) 3/4" φ + 12" EMBED |
| C6 | HSS 10x10x3/8 | R 1 x 1'-4" x 1'-0" | TYPE 1 | (4) 3/4" φ + 12" EMBED |
| C7 | HSS 10x14x3/8 | R 1 x 1'-4" x 1'-4" | TYPE 1 | (4) 3/4" φ + 12" EMBED |
| C8 | HSS 6x6x3/8 | R 1 x 1'-0" x 1'-0" | A3/S204 | (8) 3/4" φ + 18" EMBED |

- COLUMN SCHEDULE NOTES:**
- SEE A4/S201 FOR TYPICAL ANCHOR BOLT DETAIL.
 - UNLESS NOTE OTHERWISE, ANCHOR BOLTS SHALL BE ASTM F1554, GR 36 (Fy=36 KSI)
 - ANCHOR BOLTS MARKED WITH AN ASTERISK SHALL BE ASTM F1554, GR 50 (Fy=55 KSI)



KEY PLAN

- FOUNDATION PLAN KEYED NOTES**
- OMIT TOP COURSE OF MASONRY AND POUR SLAB THRU AT DOOR OPENINGS, PER C1/S201 UNLESS ANOTHER DETAIL IS SPECIFIED.
 - SAW-CUT CONTROL JOINTS PER A1.1 OR A1.2/S201. LOCATE GENERALLY AT CENTERLINES OF COLUMNS, AND RE-ENTRANT CORNERS, AT A MAXIMUM SPACING OF 15'-0".
 - CAST BOTH SPREAD FOOTINGS TOGETHER AND OVERLAP REINFORCING.
 - 2'-8" WIDE x 12" THICK CONCRETE TIE-ROD ENCASUREMENT CAST INTEGRAL WITH SLAB ON GRADE WITH (3) #8 BARS AND (2) #7 BARS CONTINUOUS. REBAR COUPLERS ARE REQUIRED AT ALL SPLICES; DO NOT LAP SPLICE BARS. SEE B1/S202 FOR RELATED DETAILS.
 - 7" THICKENED SLAB x 2'-0" WIDE W/ (2) #6 TIE-RODS CONT. (LOCATED 4" BELOW TOP OF SLAB), SPANNING BETWEEN PIERS AT X-BRACED FRAMES ALONG LINES 4 & 16 ONLY. SEE B1/S202 FOR PLAN DETAIL.
 - #5 HAIR-PIN REBAR CAST INTO SLAB ON GRADE PER DETAIL C1/S202.
 - PLATFORM FLOOR CONSTRUCTION (LIMITS ARE DEFINED BY DOTS HATCH PATTERN): 3" CONCRETE OVER 2'-20 GA. COMPOSITE DECK W/ 6x6-W2.9xW2.9 W.W.F. 2" BELOW TOP OF SLAB. OVERALL THICKNESS = 5". TOP OF SLAB = 102'-9 1/2". THE COMPOSITE FLOOR DECK HAS BEEN DESIGNED TO SUPPORT THE WEIGHT OF A 4,000# (TOTAL) SCISSORS LIFT WITH A WHEEL BASE AS FOLLOWS: 2'-6" x 6'-0". NOTIFY THE ENGINEER OF RECORD FOR SCISSORS LIFTS THAT VARY FROM THESE SPECIFICATIONS.
 - 8" CMU SUPPORT WALL WITH HORIZONTAL REINFORCING, NOT VERTICAL REINFORCING, BEARING ON TOP OF 6" THICK CONCRETE FLOOR SLAB PER NOTE 14 BELOW.
 - 8x16 CMU PIER EA. SIDE OF P.E.M.B. COLUMN TO SUPPORT ENDS OF DECK SUPPORT ANGLE PER KEYED NOTE 10 BELOW.
 - CONC. L5x 3/8x 3/8 DECK SUPPORT ANGLE AT EDGE OF PLATFORM.
 - (2) L3 1/2 x 4 x 3/8 STEEL LINTELS (LLV - LLBB). PROVIDE 6" BEARING LENGTH, E.E. AND MAINTAIN 2'-0" CLEAR VERTICAL OPENING HEIGHT BENEATH BOTTOM SIDE OF LINTELS.
 - TURN-DOWN SLAB AT EDGE OF BAPTISTERY PIT. VERTICAL FACE TERMINATES INTO MAT FOOTING BELOW.
 - 8" THICK MAT FOOTING W/ #5 BARS AT 12" O.C., E.W. AT MID-DEPTH (T.O.F. = 97'-10")
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 - FOOTING TYPE F-1, SEE FOOTING SCHEDULE FOR REINFORCING. SIZE OF FOOTING IS 9" BEYOND THE LIMITS OF THE CONCRETE PIER ON EACH SIDE.
 - PLATFORM FLOOR CONSTRUCTION: 3/4" PLYWOOD, OVER 3/4" SOUNDBOARD, OVER 3/4" FRT PLYWOOD FLOOR DECKING, OVER COLD-FORMED STEEL JOISTS. TOP OF PLATFORM = 101'-9".
 - 6'-16 GA. COLD-FORMED STEEL (CFS) JOISTS AT 1'-4" O.C.
 - LOAD BEARING STUD WALLS PER A1 & B1/S204: 3/8"-20 GA. CFS STUDS AT 1'-4" O.C.
 - LOAD BEARING STUD WALLS SUPPORTING CEILING JOISTS ABOVE: 3/8"-18 GA. CFS STUDS AT 1'-4" O.C. SEE ARCH FOR ADDITIONAL INFORMATION.
 - 8" CFS INTERIOR STUD WALL PER PLAN
 - PARTIAL HEIGHT KNEE WALL PER C1/S204: 3/8"-18 GA. CFS STUDS AT 1'-4" O.C.

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NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
 6300 WOODYARD ROAD
 UPPER MARLBORO, MD 20772

DRAWING DATE
 Bid Set 17 NOV 2023

REVISIONS

FOUNDATION PLAN
S101B
 OF 18 SHEETS
216118

S101A MATCHLINE
S101C

S101A
S101C

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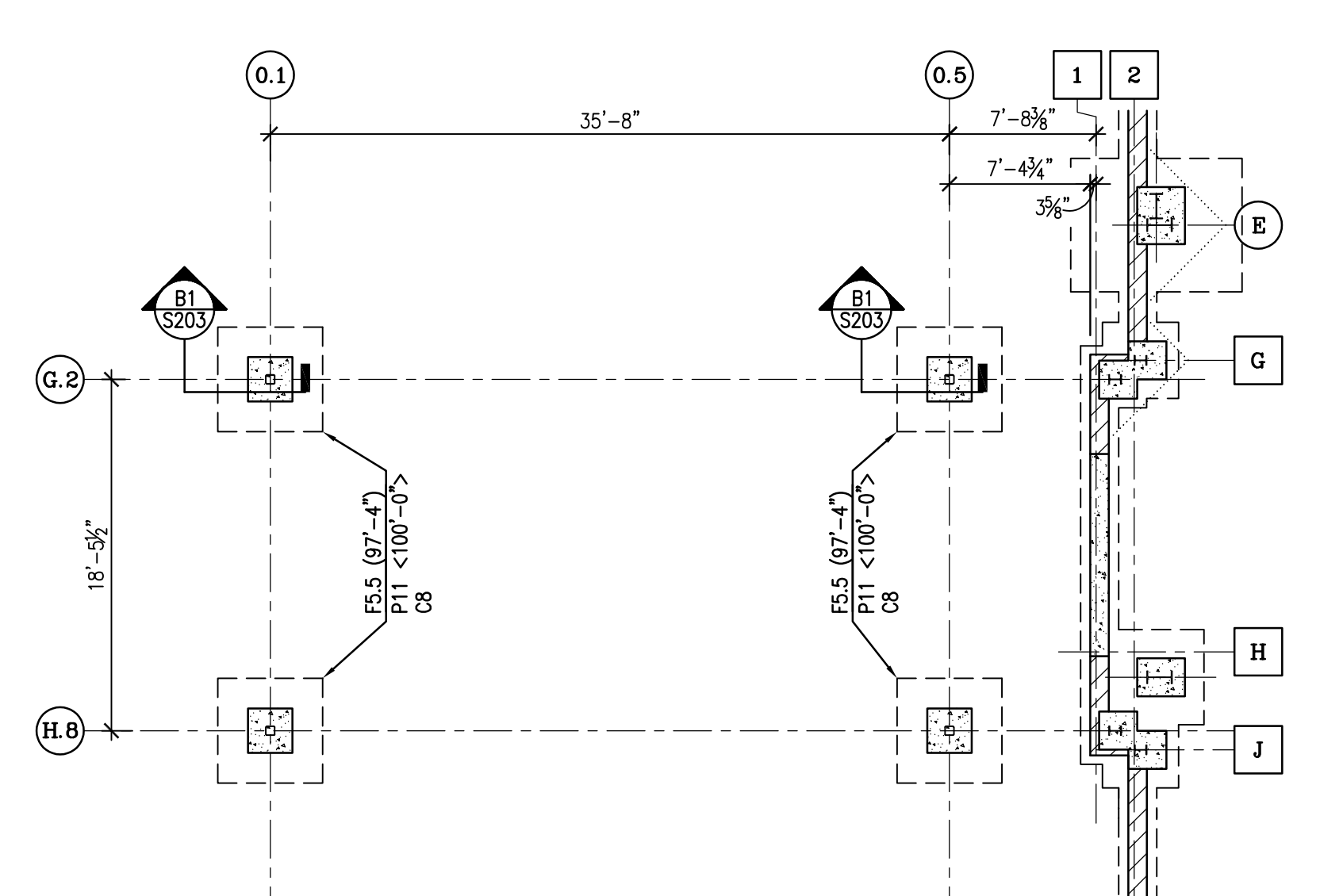
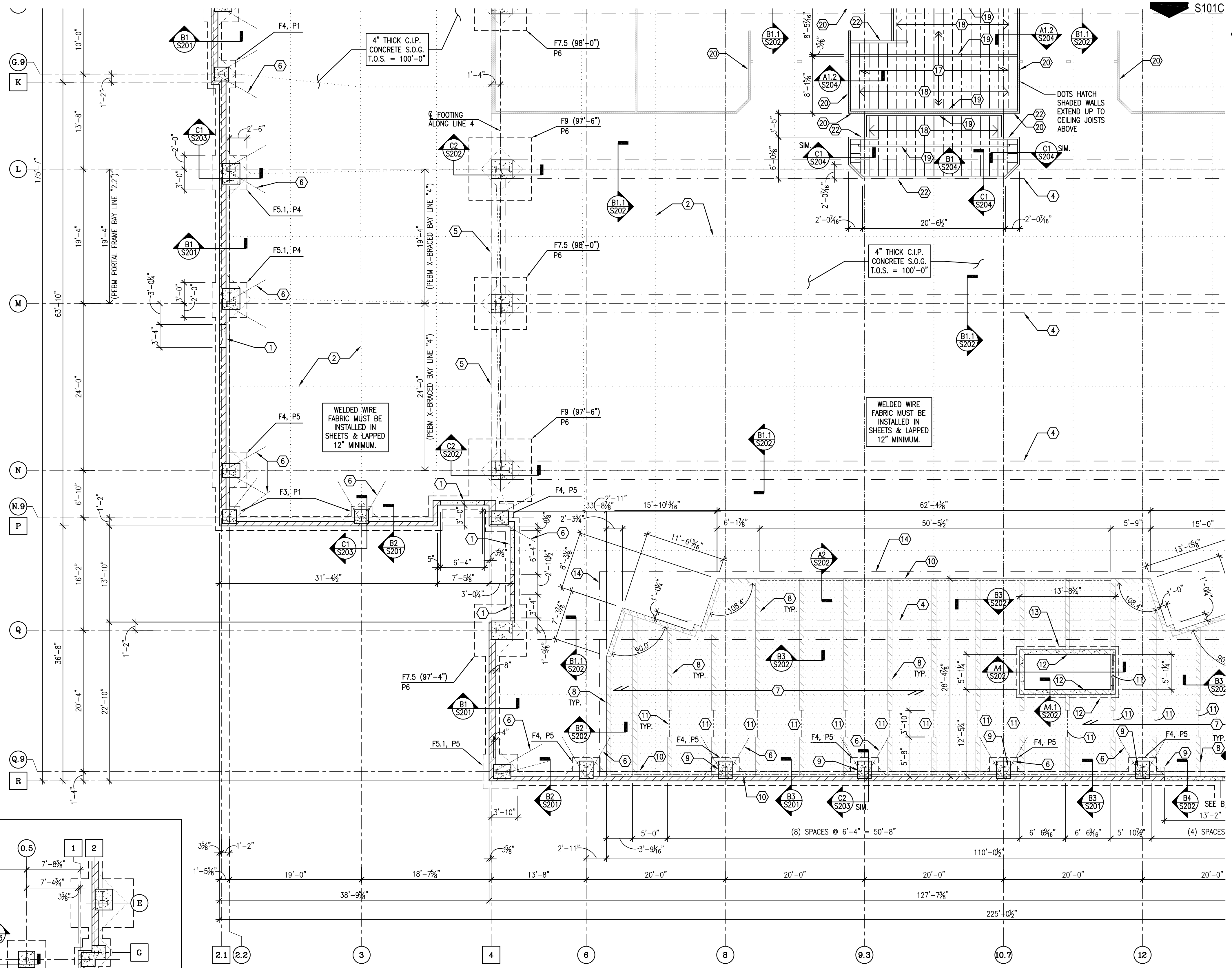
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REVISIONS

FOUNDATION PLAN

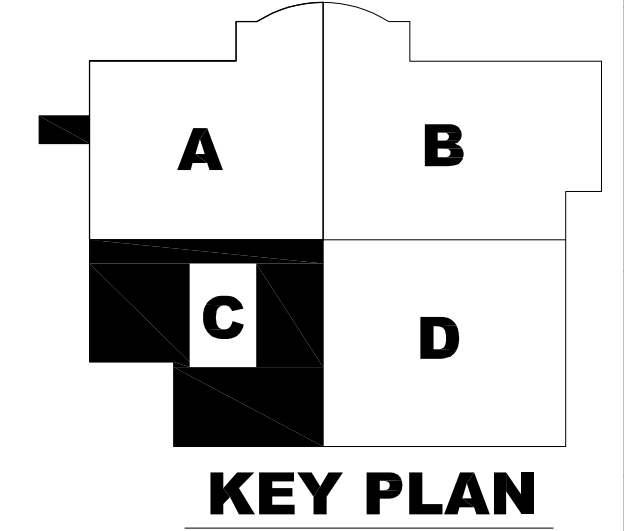
S101C
OF 18 SHEETS

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A FOUNDATION PLAN - AREA C
SCALE: 1/8" = 1'-0"
FOUNDATION PLAN NOTES:
A. SEE SHEET S101A FOR FOUNDATION PLAN NOTES AND FOUNDATION KEYED NOTES.

B ADD ALTERNATE ENTRANCE CANOPY FOUNDATION PLAN
SCALE: 1/8" = 1'-0"



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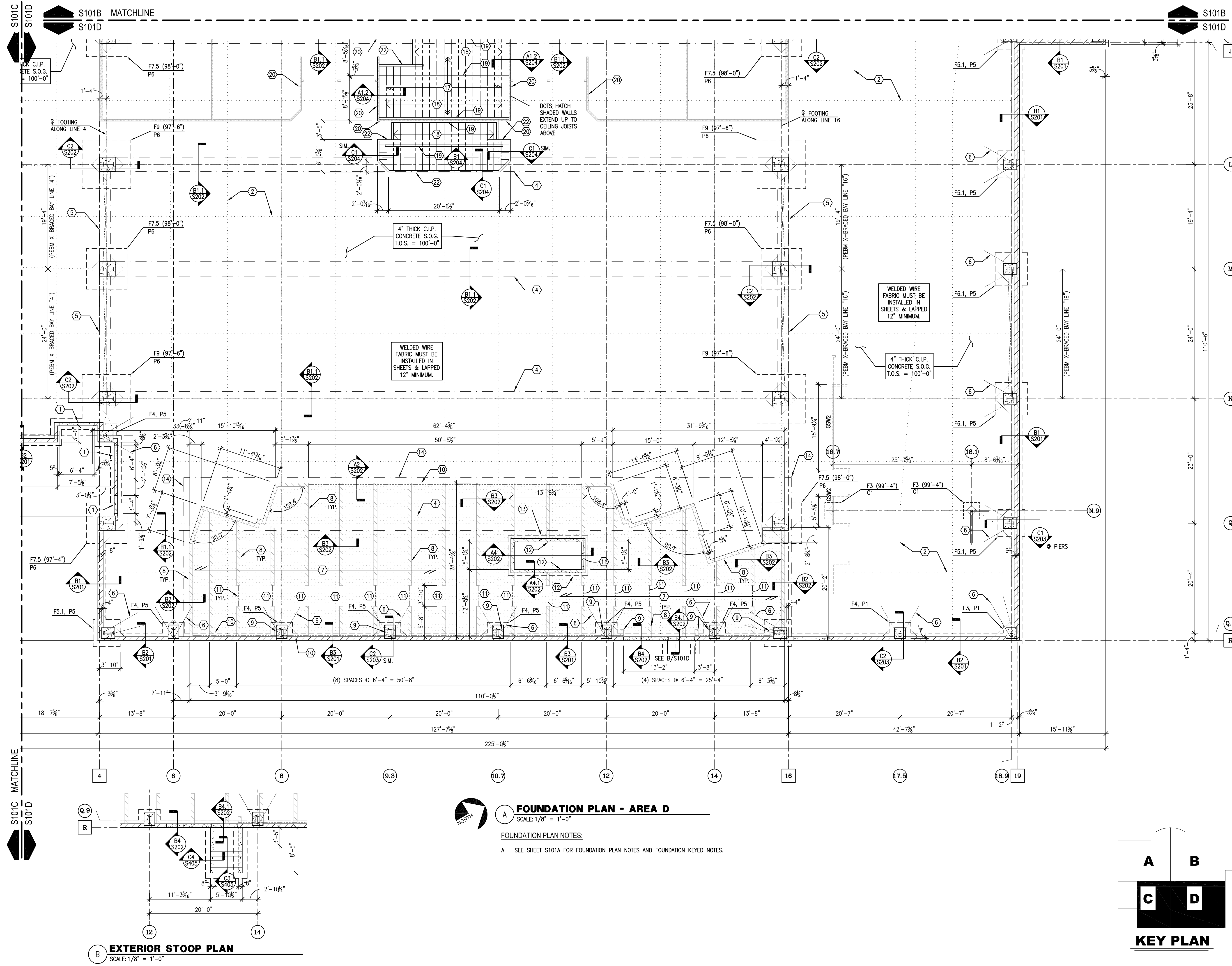
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 6300 WOODYARD ROAD
 UPPER MARLBORO, MD 20772

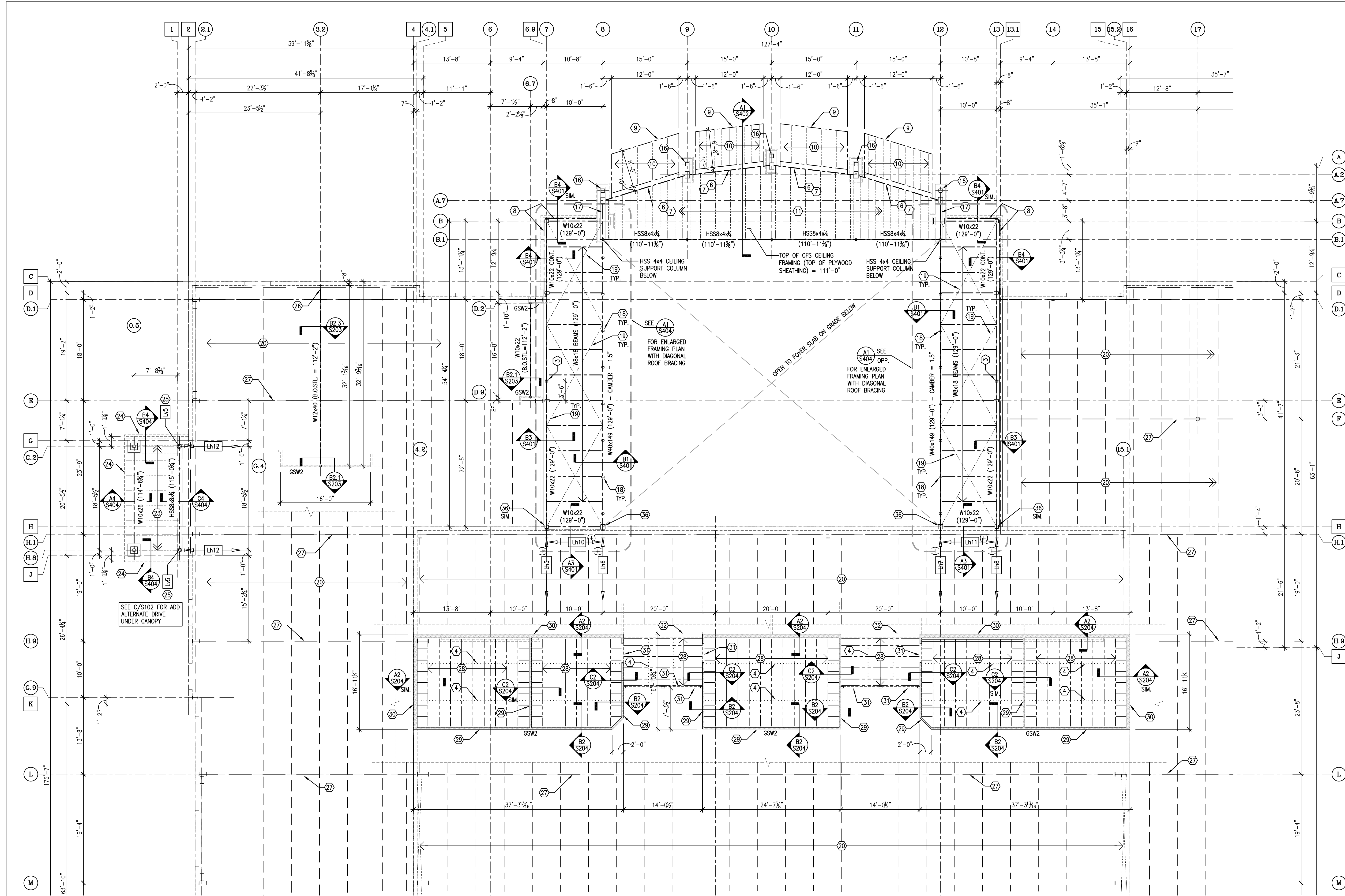
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REVISIONS

FOUNDATION PLAN
S101D
 OF 18 SHEETS
 216118



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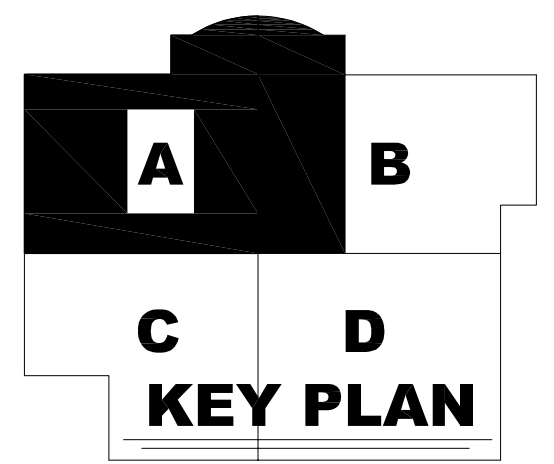


MID-HEIGHT ROOFS OVER FOYER AND AREA A + B

SCALE: 1/8" = 1'-0"

ROOF FRAMING PLAN GENERAL NOTES

- A. SEE SHEET S102 FOR ROOF FRAMING PLAN GENERALS NOTES AND ROOF FRAMING PLAN KEYED NOTES.



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Architect is not responsible for any dimensions scaled from drawings. Dimensions noted take precedence.

NEW BUILDING FOR:
GALILEE BAPTIST CHURCH

6300 WOODYARD ROAD
UPPER MARLBORO, MD 20772

| DRAWING | DATE |
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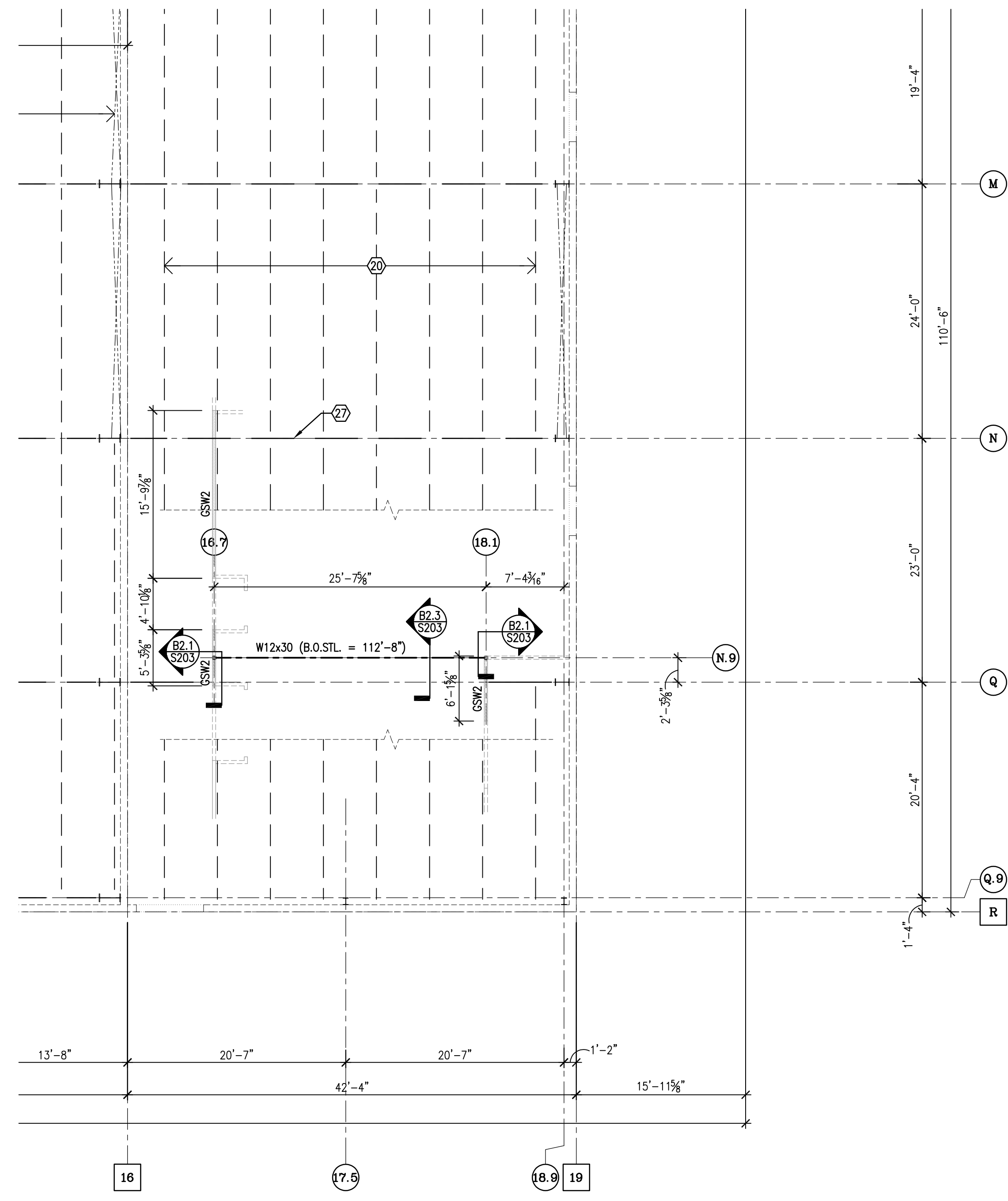
REVISIONS

ROOF FRAMING PLAN - A

S102A

OF 18 SHEETS

216118

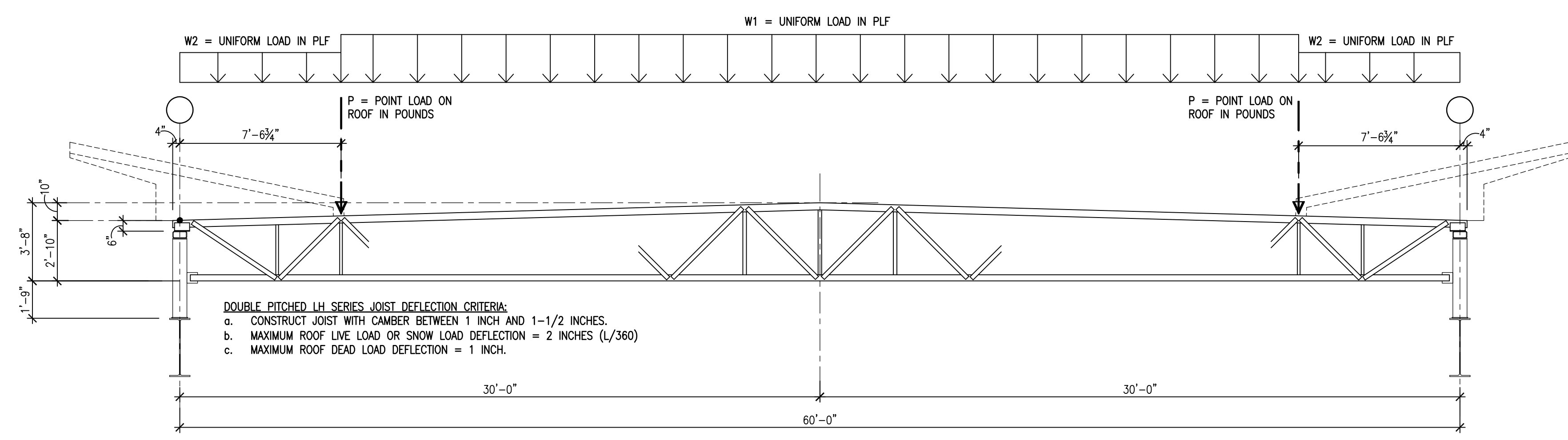


PEMB ROOF - OPERABLE PARTITION FRAMING
SCALE: 1/8" = 1'-0"

| MARK | DEAD | ROOF LIVE | SNOW | WIND | ELEVATION APPLIED TO PEMB COLUMN | NOTES/COMMENTS |
|------|--------|-----------|---------|--------|----------------------------------|----------------|
| Lv1 | +9.0 K | +5.0 K | +9.5 K | -7.0 K | 133'-8" A.F.F. | |
| Lv2 | +9.0 K | +7.1 K | +14.0 K | -9.0 K | 134'-2" A.F.F. | |
| Lv3 | +9.0 K | +7.1 K | +14.0 K | -9.0 K | 134'-2" A.F.F. | |
| Lv4 | +9.0 K | +5.0 K | +9.5 K | -7.0 K | 133'-8" A.F.F. | |
| Lv5 | +2.0 K | +2.5 K | +5.0 K | -4.5 K | 115'-2" A.F.F. | |

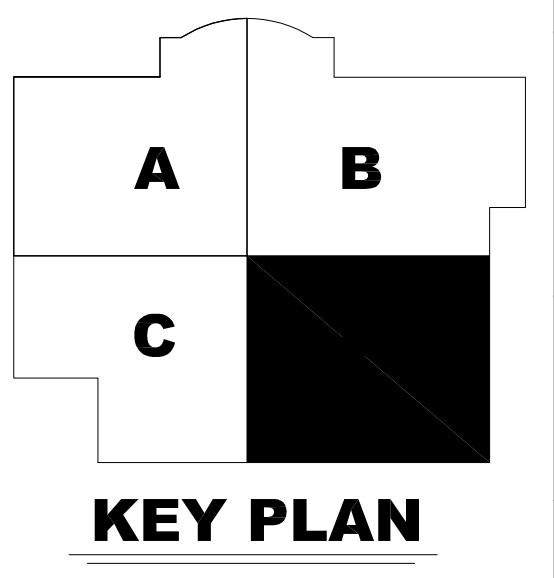
| LOAD CASE | MARK | WIND | EARTH QUAKE | COLUMN LINE | ELEVATION APPLIED TO PEMB COLUMN | NOTES/COMMENTS |
|-------------|------|----------|-------------|-------------|----------------------------------|----------------|
| LOAD CASE 1 | Lh1 | +/- 2.5K | +/- 1.4K | (8) | 133'-8" A.F.F. | |
| | Lh2 | +/- 3.8K | +/- 2.4K | (9) | 134'-2" A.F.F. | |
| | Lh3 | +/- 3.8K | +/- 2.4K | (10) | 134'-2" A.F.F. | |
| | Lh4 | +/- 2.5K | +/- 1.4K | (12) | 133'-8" A.F.F. | |
| | Lh5 | +/- 3.3K | +/- 1.0K | (7) | 129'-0" A.F.F. | |
| | Lh6 | +/- 3.0K | +/- 1.0K | (8) | 129'-0" A.F.F. | |
| | Lh7 | +/- 3.0K | +/- 1.0K | (12) | 129'-0" A.F.F. | |
| | Lh8 | +/- 3.3K | +/- 1.0K | (10) | 129'-0" A.F.F. | |
| | Lh9 | NA | NA | (H) | 134'-2" A.F.F. | |
| | Lh10 | - 7.5K | NA | (H) | 129'-0" A.F.F. | |
| | Lh11 | + 7.5K | NA | (H) | 129'-0" A.F.F. | |
| LOAD CASE 2 | Lh1 | + 8.0K | NA | (8) | 133'-8" A.F.F. | |
| | Lh2 | + 3.3K | NA | (9) | 134'-2" A.F.F. | |
| | Lh3 | + 3.3K | NA | (10) | 134'-2" A.F.F. | |
| | Lh4 | + 1.9K | NA | (12) | 133'-8" A.F.F. | |
| | Lh5 | + 1.5K | NA | (7) | 129'-0" A.F.F. | |
| | Lh6 | - 1.8K | NA | (8) | 129'-0" A.F.F. | |
| | Lh7 | + 0.9K | NA | (12) | 129'-0" A.F.F. | |
| | Lh8 | - 2.1K | NA | (10) | 129'-0" A.F.F. | |
| | Lh9 | + 10.0K | + 3.6K | (H) | 134'-2" A.F.F. | |
| | Lh10 | + 5.9K | + 2.0K | (H) | 129'-0" A.F.F. | |
| | Lh11 | + 2.6K | + 2.0K | (H) | 129'-0" A.F.F. | |
| LOAD CASE 3 | Lh1 | + 1.9K | NA | (8) | 133'-8" A.F.F. | |
| | Lh2 | + 3.3K | NA | (9) | 134'-2" A.F.F. | |
| | Lh3 | + 3.3K | NA | (10) | 134'-2" A.F.F. | |
| | Lh4 | + 8.0K | NA | (12) | 133'-8" A.F.F. | |
| | Lh5 | - 2.1K | NA | (7) | 129'-0" A.F.F. | |
| | Lh6 | + 0.9K | NA | (8) | 129'-0" A.F.F. | |
| | Lh7 | - 1.8K | NA | (12) | 129'-0" A.F.F. | |
| | Lh8 | + 1.5K | NA | (10) | 129'-0" A.F.F. | |
| | Lh9 | - 10.0K | - 3.6K | (H) | 134'-2" A.F.F. | |
| | Lh10 | - 5.9K | - 2.0K | (H) | 129'-0" A.F.F. | |
| | Lh11 | - 2.6K | - 2.0K | (H) | 129'-0" A.F.F. | |
| | Lh12 | - 1.0K | - 0.5K | (2) | 115'-0" A.F.F. | |

- PEMB COLUMN LOAD SCHEDULE NOTES:**
- WIND AND EARTHQUAKE LATERAL LOADS PROVIDED IN THIS TABLE REPRESENT ULTIMATE LEVEL LOADS PER ASCE 7-16. MULTIPLY THESE LOADS BY THE APPROPRIATE FACTORS FOR LOAD COMBINATIONS: 0.6 FOR WIND OR 0.7 FOR EARTHQUAKE PER ASCE 7-16.
 - DIRECTION OF LATERAL LOADS ARE INDICATED BY ARROWS ATTACHED TO THE PLAN MARK/NOTE. FORCES NOTED AS (+/-) ACT IN BOTH DIRECTIONS OR ARE REVERSIBLE. THE POSITIVE DIRECTION, IF INDICATED ON THE PLAN, IS INDICATED THUS: (+).
 - "BUILDING" NOTATION WHEN PROVIDED AS A WIND LOAD INDICATES THAT THE P.E.M.B. FRAME SHOULD BE DESIGNED FOR THE FULL LEeward OR WINDWARD PRESSURE. THUS, THE CONVENTIONAL STEEL BUILDING DOES NOT SHED OR REDUCE THE WIND LOAD TO THE P.E.M.B. STRUCTURAL FRAME.
 - GRAVITY LOADS SHOWN WITH A NEGATIVE SIGN (-) INDICATE UPWARD ACTING LOADS (WIND UPLIFT, ETC).



LH JOIST PROFILE AND LOADING DIAGRAM
SCALE: 1/4" = 1'-0"

| MARK | DEAD | | ROOF LIVE | | | SNOW | | | WIND UPLIFT | | | |
|--------|---------|---------|-----------|---------|--------|------|---------|-------|-------------|---------|-------|------|
| | W1 | W2 | W1 | W2 | P(lb) | W1 | W2 | P(lb) | W1 | W2 | P(lb) | |
| LH-SJ1 | 163 plf | 163 plf | 320 | 195 plf | 65 plf | 780 | 176 plf | 0 plf | 710 | -40 plf | 0 plf | -160 |
| LH-SJ2 | 163 plf | 163 plf | 320 | 195 plf | 65 plf | 780 | 240 plf | 0 plf | 960 | -40 plf | 0 plf | -160 |
| LH-SJ3 | 163 plf | 163 plf | 320 | 195 plf | 65 plf | 780 | 440 plf | 0 plf | 1640 | -40 plf | 0 plf | -160 |



FOUNDATION PLAN
S102D
OF 18 SHEETS
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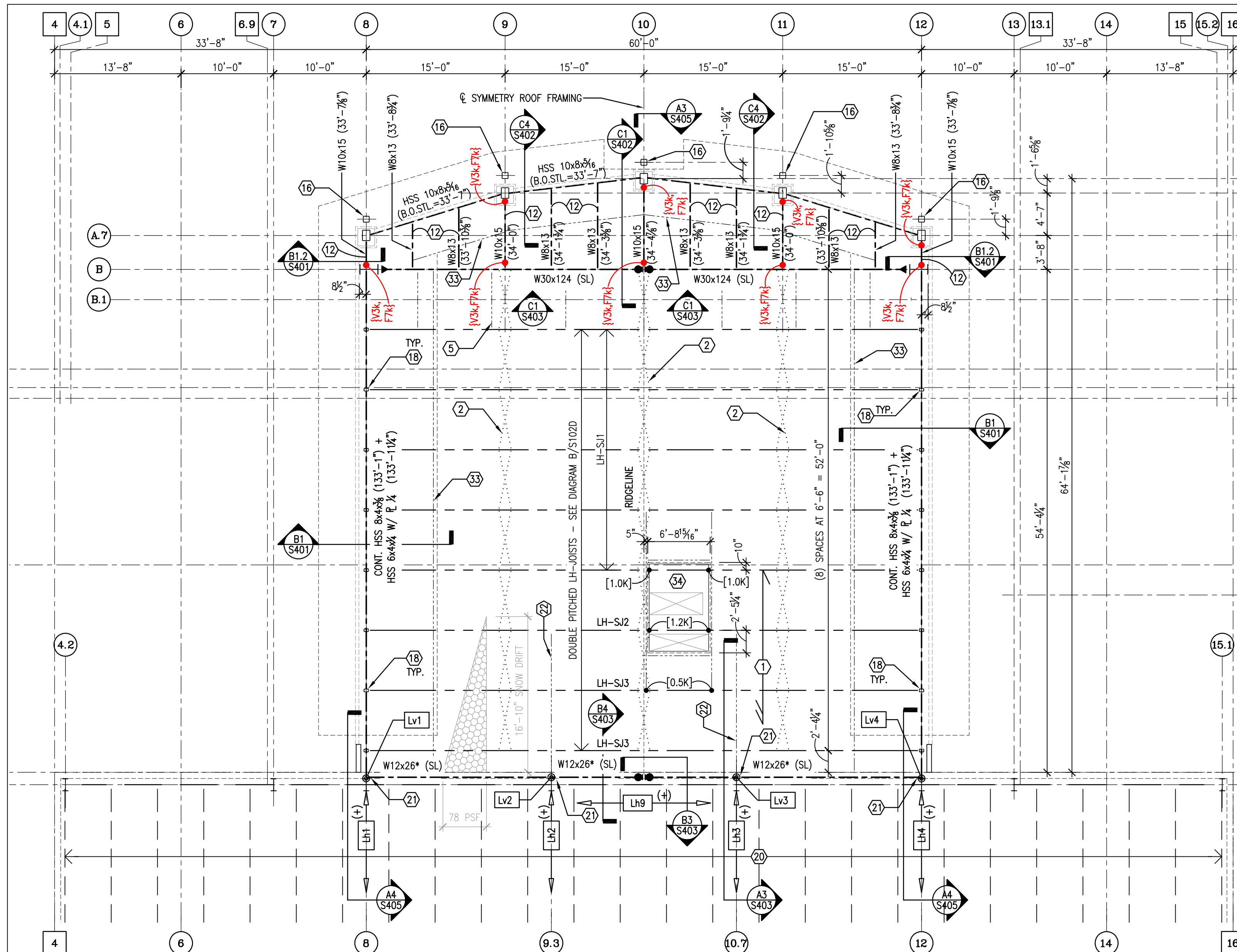
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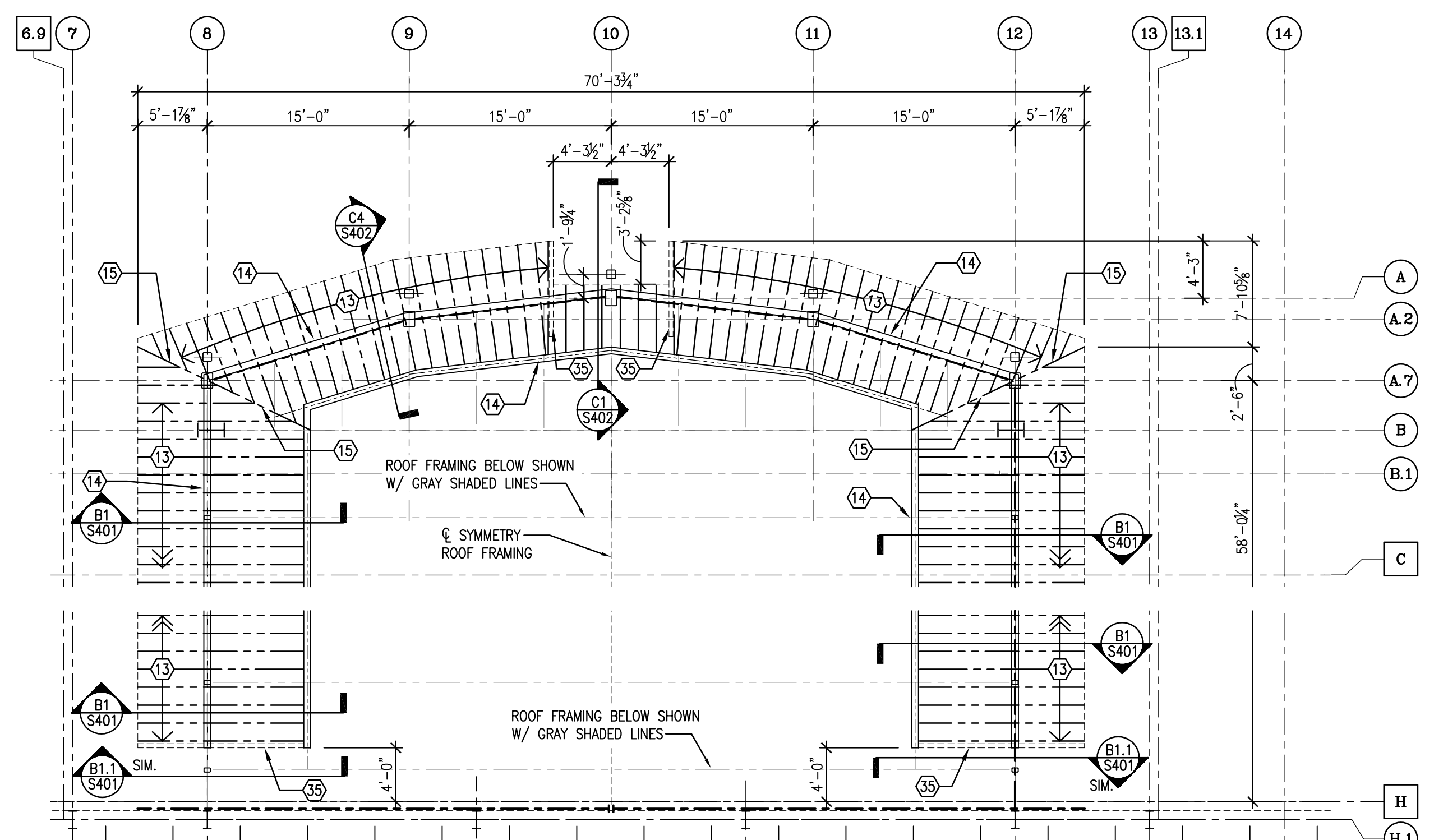
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A HIGH ROOF FRAMING OVER FOYER
SCALE: 1/8" = 1'-0"



B HIGH ROOF PARAPET FRAMING PLAN
SCALE: 1/8" = 1'-0"

ROOF FRAMING PLAN GENERAL NOTES

- A. SEE SHEET S001, "STRUCTURAL GENERAL NOTES" FOR DESIGN ROOF LIVE LOADS, SNOW LOADS, AND LEGEND. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF "ARCHITECTURALLY EXPOSED STRUCTURAL STEEL" OR AESS. SEE SECTION 051200 FOR SPECIFICATIONS ON AESS.
- B. SEE SHEET S101B FOR COLUMN SCHEDULE. COLUMNS ARE NOTED ON THE FOLLOWING FOUNDATION PLANS, S101A, S101B, S101C, AND S101D. ALL COLUMNS ARE TO BE SHIPPED AS ONE SINGLE PIECE WITHOUT SPLICES.
- C. ROOF CONSTRUCTION: 1 1/2" - 20 GAGE, ACOUSTICAL (NON-CELLULAR), WIDE RIB, GALVANIZED STEEL DECK OVER STEEL BAR JOISTS AND BEAMS, U.N.O. EXAMPLE PRODUCT INCLUDES WULFURMET 1.5-B OR EQUAL. USE STANDARD 1.5B DECK OVER ENTRANCE CANOPY AREAS. SEE PLAN FOR LOCATIONS OF HEAVIER GAGE DECK UNDER SNOW DRIFT LOADS.
- D. TOP OF STEEL ELEVATION (T.O.STL.) = 129'-0", U.N.O. ELEVATIONS PROVIDED IN PARENTHESES ADJACENT TO BEAM SIZES, THUS (XXX'-XX") INDICATE RELATIVE HEIGHT ABOVE THE SLAB ON GRADE REFERENCE ELEVATION OF 100'-0". DUE TO LIMITED SPACE FOR TEXT ON THE PLANS, SOME ELEVATIONS ARE EXPRESSED WITHOUT THE LEADING 100'S PLACE; THUS (129'-0") WOULD BE EXPRESSED AS (29'-0").
- E. ELEVATIONS SHOWN INDICATE THE FOLLOWING:
AT STEEL JOISTS: JOIST BEARING EL.
AT STEEL BEAMS: TOP OF BEAM EL.
- F. SLOPING BEAMS ARE INDICATED THUS: (SL). TOP OF STEEL ELEVATIONS ARE PROVIDED IN PARENTHESES AT EACH END OF SLOPING BEAMS. TOP OF STEEL OF BEAMS LABELED WITH AN ASTERISK "*" SHALL MATCH SLOPING TOP PLANE OF ADJACENT STEEL JOISTS.
- G. WORK-POINTS AT SLOPING MEMBERS ARE AT CENTERLINES OF COLUMNS, OR SUPPORTING BEAMS AND AT INSIDE FACE OF WALLS, U.N.O.
- H. SIMPLE SHEAR CONNECTIONS AT THE ENDS OF BEAMS SHALL BE DESIGNED FOR HALF OF THE FULL UNIFORM LOAD CAPACITY OF THE MEMBER AS PUBLISHED BY AISC, EXCEPT WHERE END REACTIONS ARE PROVIDED (IN KIPS) AT THE END OF A BEAM AS RED COLORED TEXT CONTAINED IN CURLY BRACKETS, THUS: {V10K, F10K}. LOADS SHOWN NEXT TO THE BEAM SIZE SHALL BE APPLIED TO EACH END. THE NOTATION "V" INDICATES A VERTICAL SHEAR FORCE AND "F" INDICATES A HORIZONTAL REVERSIBLE AXIAL FORCE ALONG THE AXIS OF THE MEMBER (AXIAL LOADS ARE APPLICABLE ONLY IF LISTED). FORCES ARE EXPRESSED AT SERVICE (ASD) LEVEL AND ALL LOAD COMBINATION FACTORS HAVE BEEN APPLIED.
- I. -I- SYMBOL SHOWN ON PLAN INDICATES BEAM TO COLUMN MOMENT RESISTING CONNECTION PER DETAIL B2/S403.
- J. -I- SYMBOL SHOWN ON PLAN INDICATES A BEAM TO BEAM, MOMENT RESISTING, SPLICE CONNECTION PER DETAIL A1/S403 (LINE B) OR B3/S403 (LINE H).
- K. -I- SYMBOL SHOWN ON PLAN INDICATES A BEAM TO BEAM, MOMENT RESISTING, SPLICE CONNECTION PER DETAIL B4/S204.
- L. LH-SERIES JOISTS SHALL HAVE 6 INCH DEEP BEARING SEATS, U.N.O.
- M. JOISTS MARKED AS "SJ" SHALL BE DESIGNED FOR SNOW DRIFTING OR OTHER "NON-UNIFORM" LOADING CONDITIONS AS LISTED IN THE "SJ - JOIST LOADING SCHEDULE" ON SHEET S101D.
- N. JOISTS SHALL BE DESIGNED FOR THE ADDITIONAL WEIGHT OF ROOF TOP HVAC UNITS AS PROVIDED ON THE PLANS. SEE DETAILS A1/S405 AND B1/S405 FOR TYPICAL JOIST REINFORCING REQUIRED.
- O. {XX.X} NUMBERS IN CURVED BRACKETS REPRESENT DESIGN REACTIONS AS ENDS OF BEAMS THAT ARE LESS THAN HALF THE PUBLISHED UNIFORM LOAD CAPACITY OF THE BEAM.
- R. PROVIDE L 5 x 3/2 x 3/8 GALVANIZED BRICK LITELS WHERE REQUIRED OVER EXTERIOR DOORS AND WINDOWS, U.N.O. PROVIDE A MIN. OF 6" BEARING LENGTH, E.E.
- S. EXTERIOR COLD-FORMED STEEL (CFS) STUD WALL CONSTRUCTION: 6"-16 GA. CFS STUDS (1/2" FLANGE) AT 16" O.C., U.N.O.
- T. ALL COLD-FORMED STEEL STUD WALLS SHALL HAVE CONT. HORIZONTAL ROWS OF 1/2"-16 GA. CFS U-CHANNEL BRIDGING AT 4'-0" O.C. AND 3/8" SHEATHING SHALL BE APPLIED TO BOTH SIDES OF THE WALL WHERE POSSIBLE. FASTENER SPACING ALONG EACH STUD AND AT BOUNDARY EDGES SHALL NOT EXCEED 12" O.C.
- V. GRAY SHADDED HATCHED WALLS WITH THE FOLLOWING NOTATION, "CSW1" OR "CSW2", IDENTIFIES LOAD-BEARING OR LATERAL RESISTING CFS STUD WALLS. REFER TO ROOF FRAMING PLAN AND DETAIL A4/S203 FOR MORE INFORMATION.
- W. [LVL] SQUARE BOX KEYED NOTES INDICATE LOADS TRANSFERRED TO THE P.E.M.B. COLUMN BY CONVENTIONAL STEEL BEAMS (FRAMING NOT SUPPLIED BY THE P.E.M.B.M.). SEE P.E.M.B. COLUMN LOAD SCHEDULE ON SHEET S102D.

ROOF FRAMING PLAN KEYED NOTES:

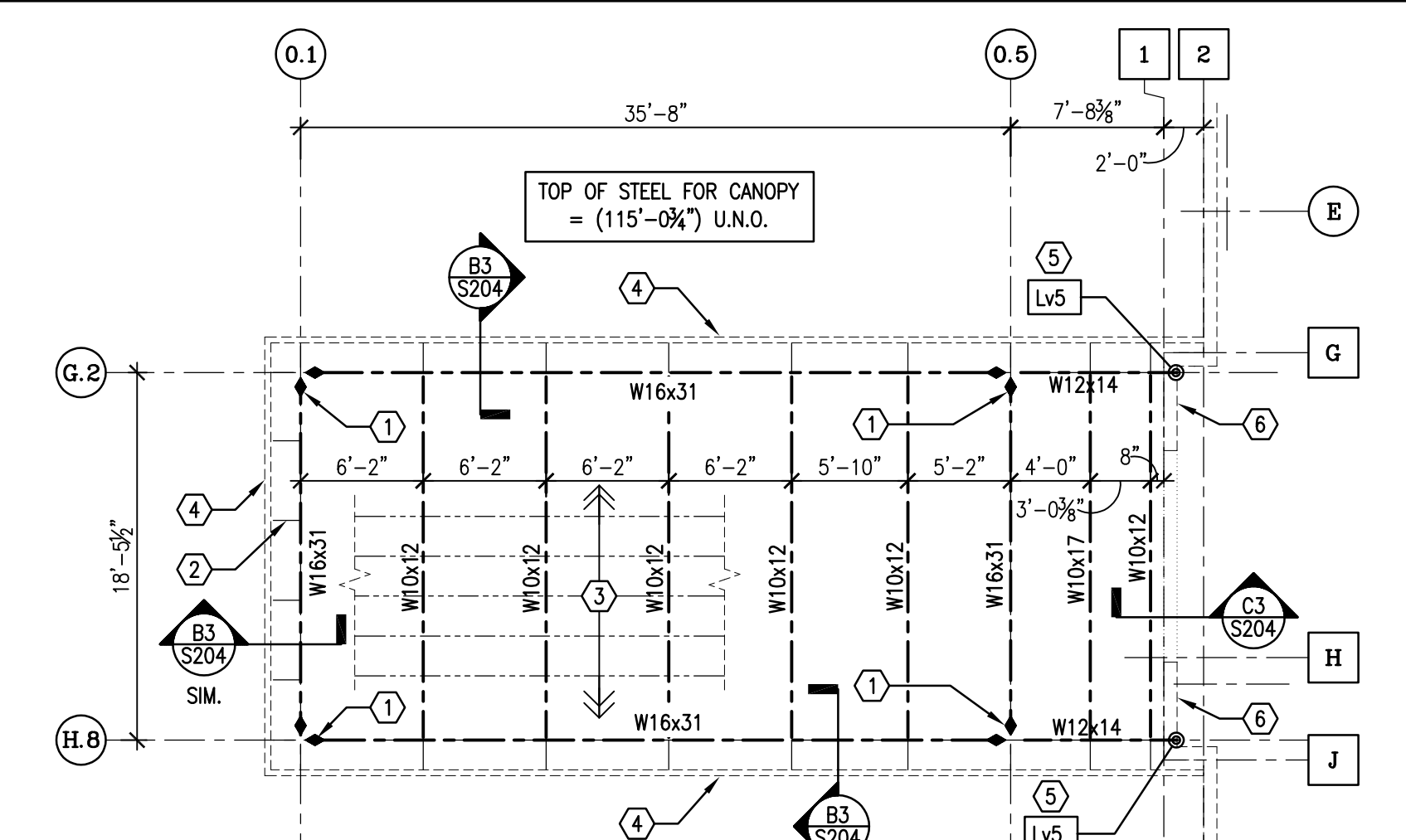
- 4. CONT. ROWS OF X-BRIDGING BETWEEN CFS JOISTS PER B3/S203.
- 5. L3x3x1/4 DIAGONAL BRACING ANGLES EXTENDING FROM BOTTOM FLANGE OF MOMENT FRAME GIRDER UPWARD TO TOP CHORD PANEL POINT OF LH-SERIES JOIST. EA. END OF BRACE ATTACHED BY FIELD WELDING. MAINTAIN SLOPE OF BRACE AT 45° (OR GREATER) W/ THE VERTICAL PLANE.
- 6. HSS 8x8x3/8 HEADER BEAM AT NEW ENTRANCE CANOPY ELEVATION (T.O.STL. = 110'-10").
- 7. HSS 8x8x3/8 HORIZONTAL GIRT SUPPORTING WINDOWS (T.O.STL. = 124'-4 1/2"), 6" TALL.
- 8. HSS 6x6x3/8 HORIZONTAL GIRT SUPPORTING WINDOWS AT THE FOLLOWING ELEVATIONS, (T.O.STL. = 110'-10" AND 124'-4 1/2").
- 9. C8x11.5 FRAME AROUND NEW CANOPY (T.O.STL. = 110'-10"). FULLY WELD AND MITER OUTER CORNERS OF FRAME. ALL FRAME MEMBERS TO BE GALVANIZED.
- 10. CANOPY INFILL JOISTS: 6"-16 GA. CFS JOISTS AT 1'-4" O.C. (T.O.JOIST = 110'-8").
- 11. INTERIOR ENTRY VESTIBULE CEILING JOISTS: 8"-18 GA. CFS JOISTS AT 1'-4" O.C. (T.O.JOIST = 110'-10"). PROVIDE ROWS OF X-BRIDGING AT 6'-0" O.C. PER DETAIL B3/S203.
- 12. ROTATE VERTICAL AXIS OF BEAM SO THAT TOP FLANGE IS ALIGNED WITH BOTTOM SURFACE OF ROOF DECK.
- 13. OVERHANG FRAMING, CFS JOISTS AT 1'-4" O.C. (1.625" FLANGES): TOP SURFACE, 6"-16 GA.; BOTTOM SURFACE, 3/8"-16 GA.
- 14. OVERHANG FRAMING SUPPORT WALLS: 6"-16 GA. CFS STUDS AT 1'-4" O.C.
- 15. (2) 8"-16 GA. STUD/JOISTS BACK TO BACK (1.625" FLANGES). FASTEN WEBS BACK TO BACK W/ #10 TEK SCREWS AT 12" O.C., STAGGERED T. & B., INTERSECTING AND SUPPORTED CFS JOIST FRAMING SHALL BE ATTACHED W/ CLARK-DIETRICH S445 CLIPS.
- 16. SEE C4/S403 FOR DIMENSIONS TO HSS 8x8 COLUMNS FROM HSS 10x14 COLUMNS. SEE ARCH DRAWINGS FOR DIMENSIONS OF COLUMN ENCLOSURES.
- 17. W8x18 AT (E) ELEVATIONS: 1 = (129'-0"), 2 = (110'-10"). DESIGN END OF BEAM CONNECTIONS FOR 4 KIPS OF REVERSIBLE AXIAL LOAD DUE TO WIND.
- 18. HSS 8x4x3/8 STUB COLUMN SUPPORTING LH JOIST ABOVE, CENTERED ON JOIST SEAT.
- 19. (2) L3 x 3/4 x 3/8 DIAGONAL BRACE (LLV) W/ 3/8" GAP. PROVIDE 1/2" DIA. STITCH FASTENERS AND 3x3x3/8 PLATE WASHERS AT MID-LENGTH BUT NOT GREATER THAN 4'-0" O.C.
- 20. CFS "Z" PURLINS AT 5'-0" O.C. +/- OR PER PEMBM DESIGN.
- 21. WELD W12x26 BEAM DIRECTLY TO FACE OF P.E.M.B. COLUMN FLANGE PER DETAIL A4/S403.
- 22. (2) L3x3x3/8 COLLECTOR BRACED WELDED TO P.E.M.B. COLUMN PER DETAIL A3/S403.
- 23. SIDE ENTRANCE VESTIBULE ROOF: 1/2" - 20 GA. CFS ROOF DECK OVER 6"-16 GA. CFS JOISTS AT 1'-4" O.C. (1.625" FLANGES) + 3/8"-18 GA. CEILING JOISTS BELOW AT 1'-4" O.C.
- 24. 3/8"-16 GA. CFS STUD PARAPET AROUND LOWER CANOPY ROOF.
- 25. PROVIDE CONNECTION OF TUBE BEAM TO P.E.M.B. COLUMN PER DETAIL A4/S401.
- 26. CONNECT CONVENTIONAL STEEL BEAM TO P.E.M.B. COLUMN PER DETAIL C3/S203.
- 27. P.E.M.B. MAIN FRAME GIRDER BEAM.
- 28. CEILING SUPPORT FRAMING: 6"-16 GA. CFS STUDS AT 2'-0" O.C. W/ 3/8" GYPSUM SHEATHING CAP. FASTEN SHEATHING W/ #6 BUGLE HEAD TEK SCREWS AT 8" O.C. ALONG ALL SUPPORTS.
- 29. CFS STUD WALL: 3/8" CFS STUDS (1.625" FLANGES) AT 1'-4" O.C.
- 30. CFS STUD WALL: SEE ARCH FOR STUD SIZE, SPACING, AND GAGE. NOTE: GAGE SHALL NOT BE LESS THAN 18 GAGE.
- 31. (2) 6"-18 GA. CFS STUD HEADER (1.625" FLANGES) PER DETAIL B4/S203.
- 32. (2) 8"-14 GA. CFS STUDS HEADER (2" FLANGES) SIMILAR TO DETAIL B4/S203. PROVIDE AT LEAST (1) JACK STUD.
- 33. CONT. C6x10.5, LEGS POINTING DOWN.
- 34. ROOF TOP MECHANICAL UNIT SUPPORT PER DETAILS A1/S405 & B1/S405. POINT LOAD LOCATIONS FOR DESIGN OF JOISTS ARE SHOWN W/ A SOLID DOT AND THE ASSOCIATED LOAD IN POUNDS IS PROVIDED ADJACENT TO EACH DOT.
- 35. CFS STUD WALL AT SIDE OF FRAMING: 3/8"-16 GA. CFS STUDS AT 16" O.C. FASTEN TO SIDE OF ROOF JOISTS PER DETAIL C1/S402.
- 36. SEE A4/S405 FOR GIRDER BEAM SUPPORT DETAIL. SIM CONDITION NOTED.

ROOF FRAMING PLAN KEYED NOTES:

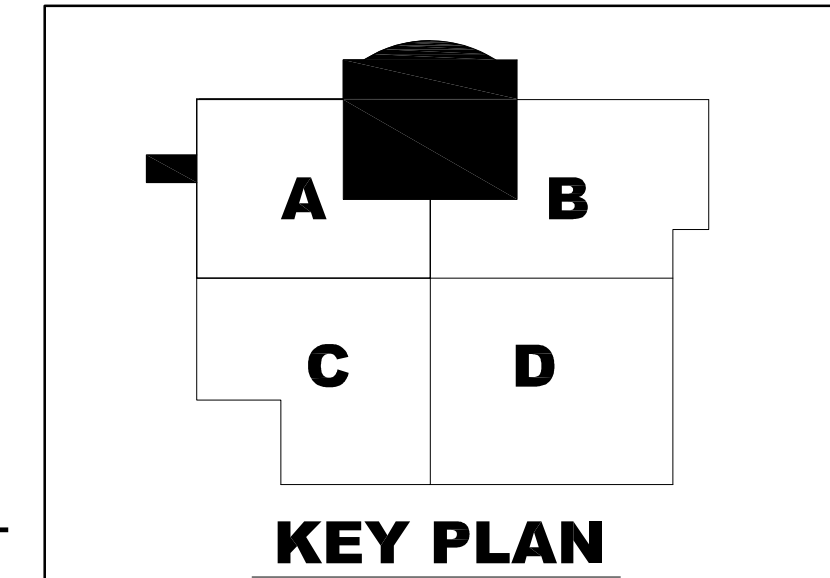
- 1. 1/2"-18 GA. CFS ROOF DECK IN THESE JOIST BAYS.
- 2. ROWS OF BOLTED DIAGONAL CROSS BRIDGING PER SJ REQUIREMENTS.
- 3. EXTEND BEAM OVER TOP OF COLUMN TO FORM A CANTILEVER AND, WHERE SHOWN ON THE PLANS, PROVIDE A BEAM TO BEAM SPLICE PER DETAIL A1/S402.

ADD ALTERNATE ENTRANCE CANOPY KEYED NOTES

- 1. MOMENT RESISTING (*) CONNECTION IN TWO DIRECTIONS PER DETAIL B4/S204.
- 2. L3x2x1/4 OUTRIGGERS AT 4'-0" O.C.
- 3. 6"-18 GA. CFS CEILING JOISTS AT 2'-0" O.C.
- 4. CFS STUD PARAPET: 3/8" - 18 GA. STUDS AT 1'-4" O.C.
- 5. CONNECT CONVENTIONAL STEEL BEAM TO FLANGE OF P.E.M.B. COLUMN W/ FIELD WELDED SINGLE SHEAR PLATE PER DETAIL C3/S203.
- 6. GIRT WALL BY P.E.M.B.M.



C ADD ALTERNATE ENTRANCE CANOPY ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"



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STATE OF MARYLAND
 CHRISTOPHER P. SEKOL
 PROFESSIONAL ENGINEER
 No. 26219

LICENSE EXPIRES: 06-21-2025
 DRAWING ISSUED: 11-17-2023

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NEW BUILDING FOR:

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| DRAWING | DATE |
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| <input checked="" type="checkbox"/> Bid Set | 17 NOV 2023 |

REVISIONS

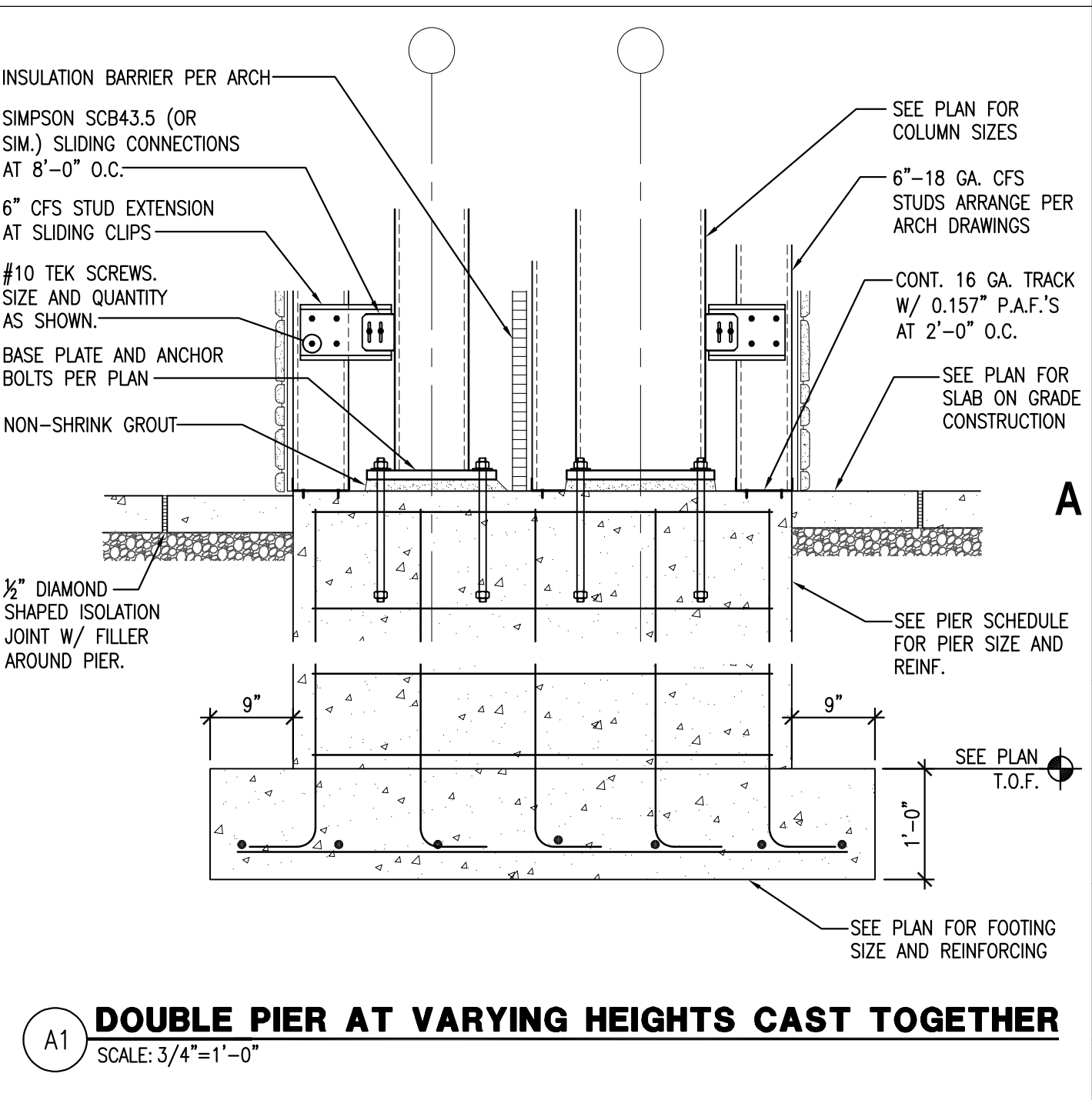
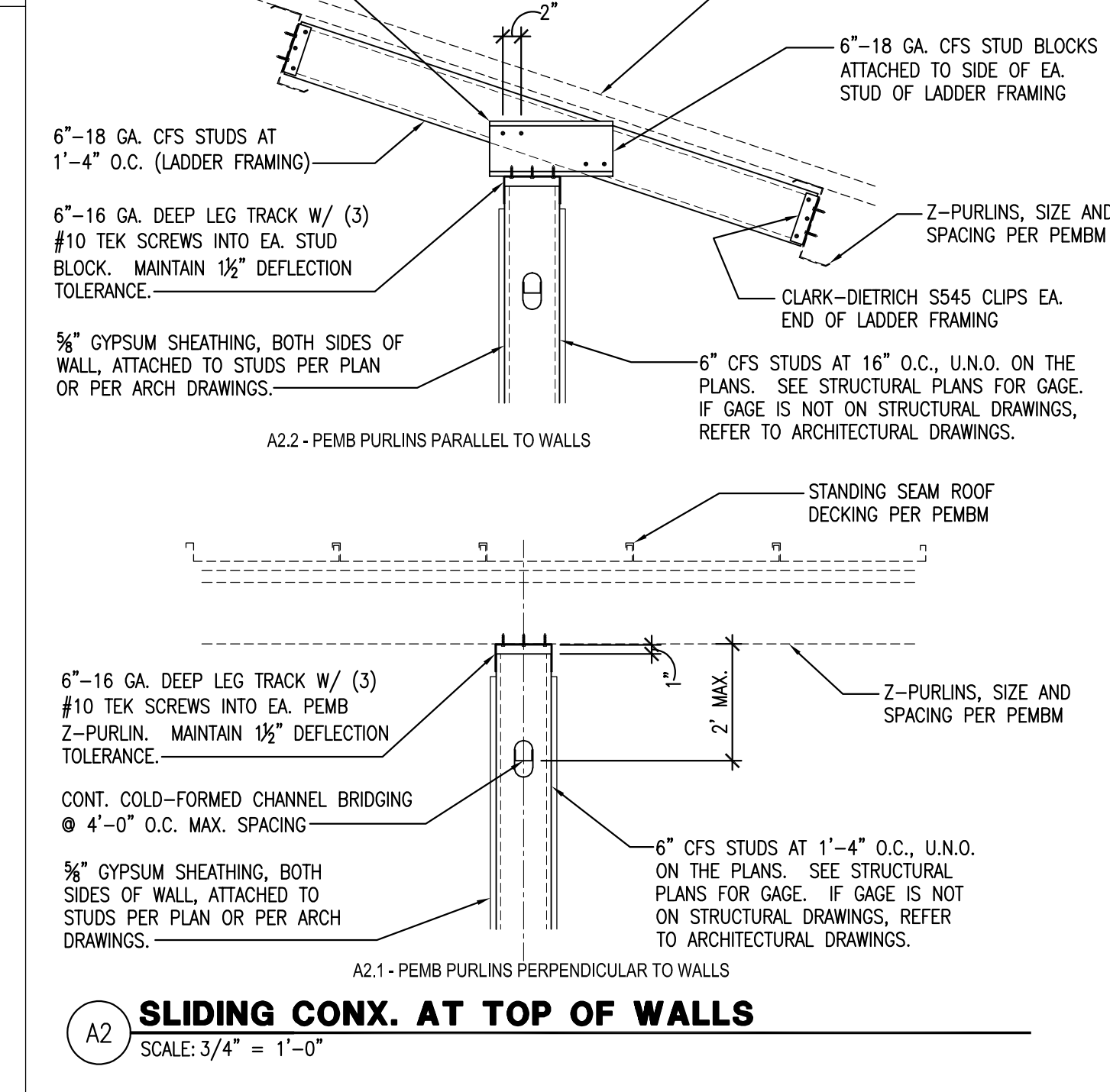
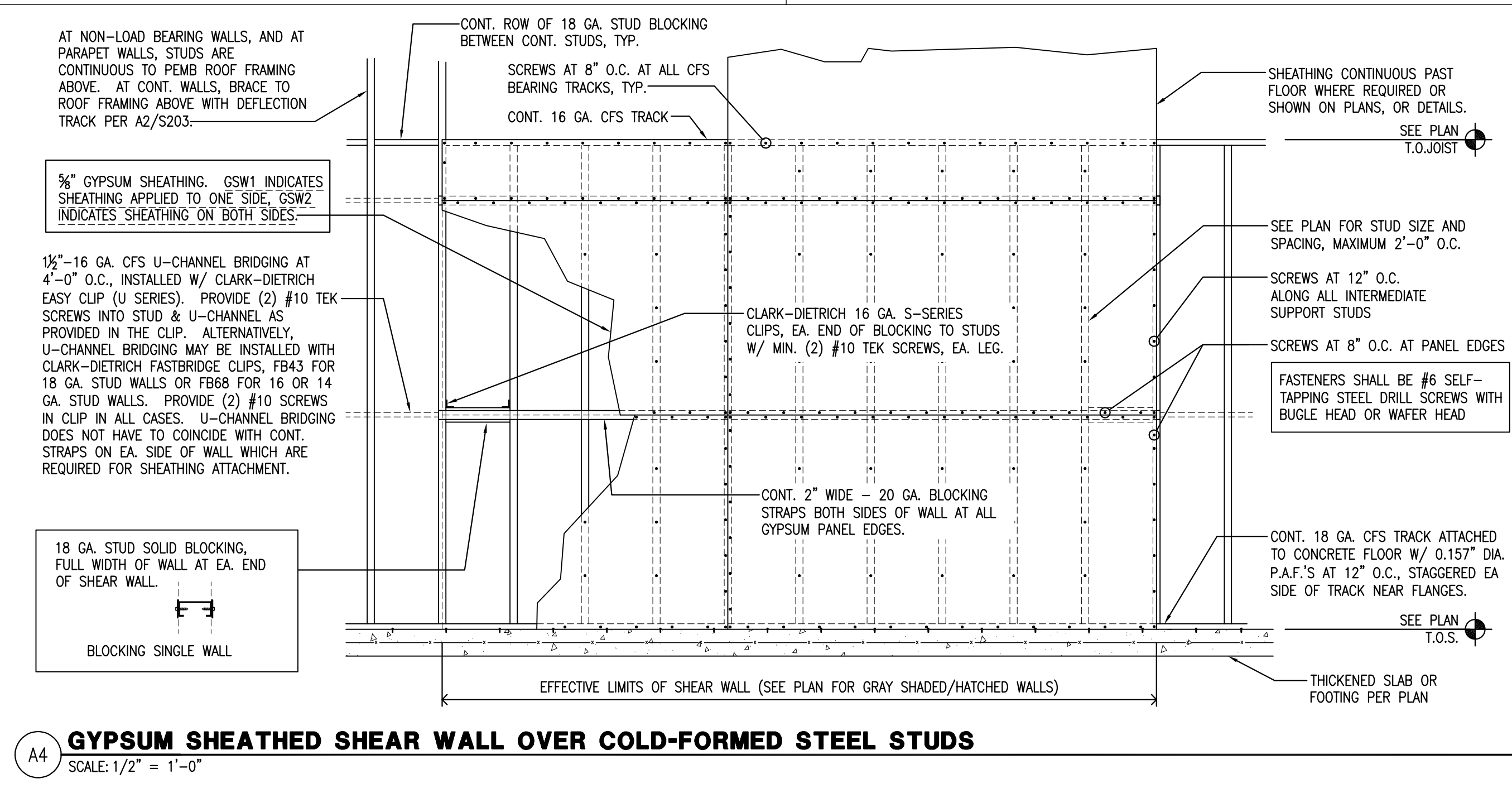
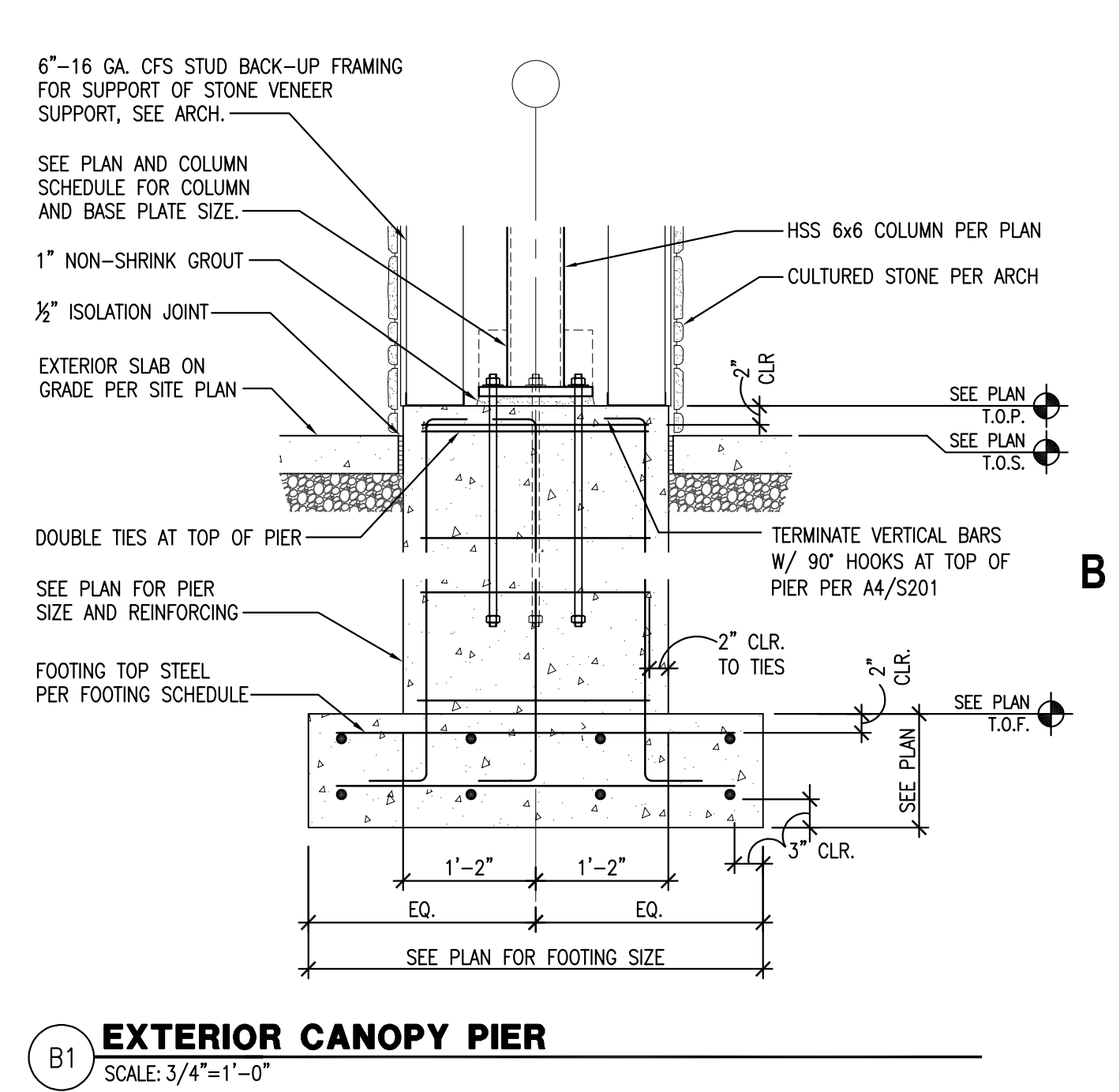
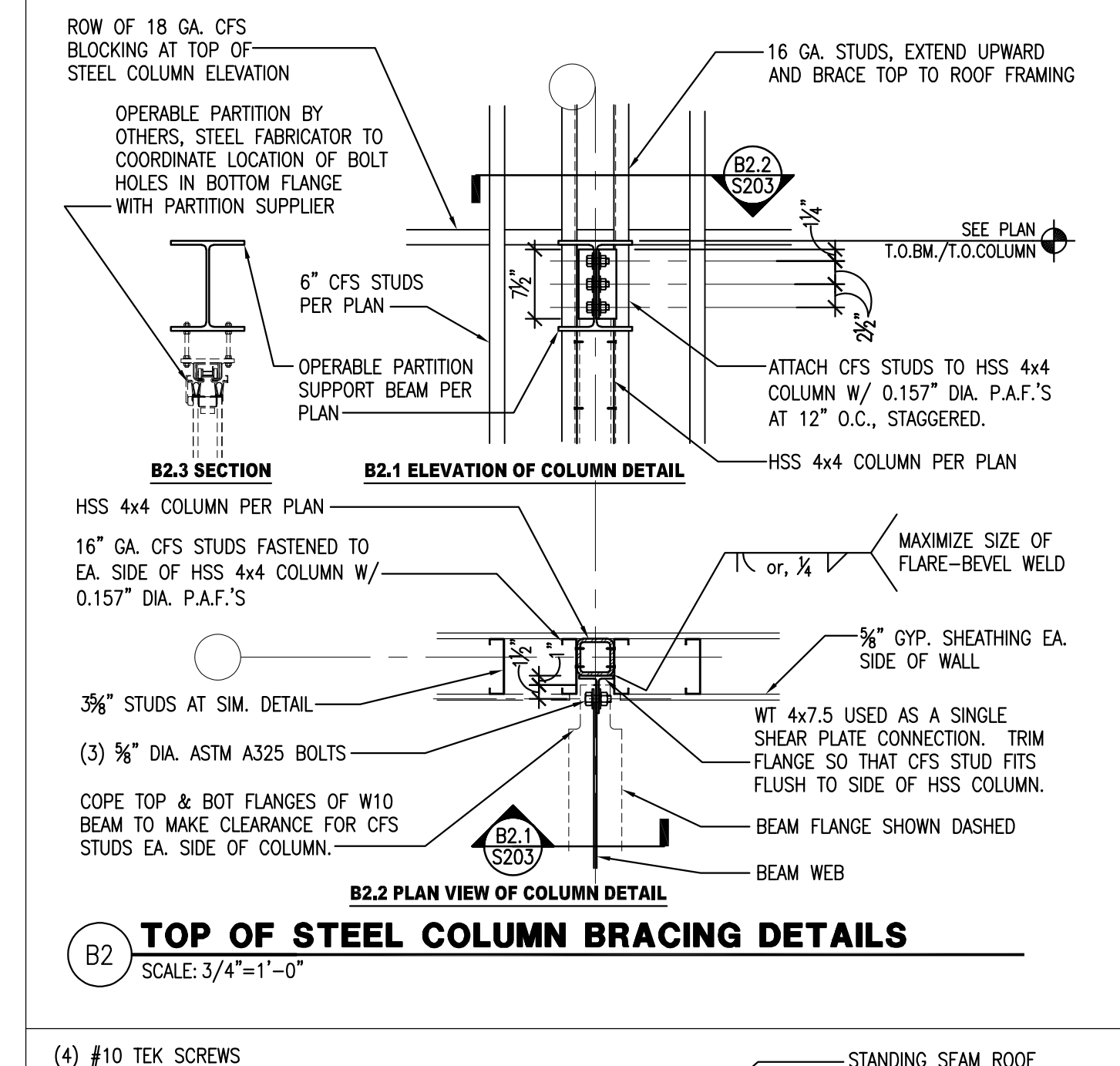
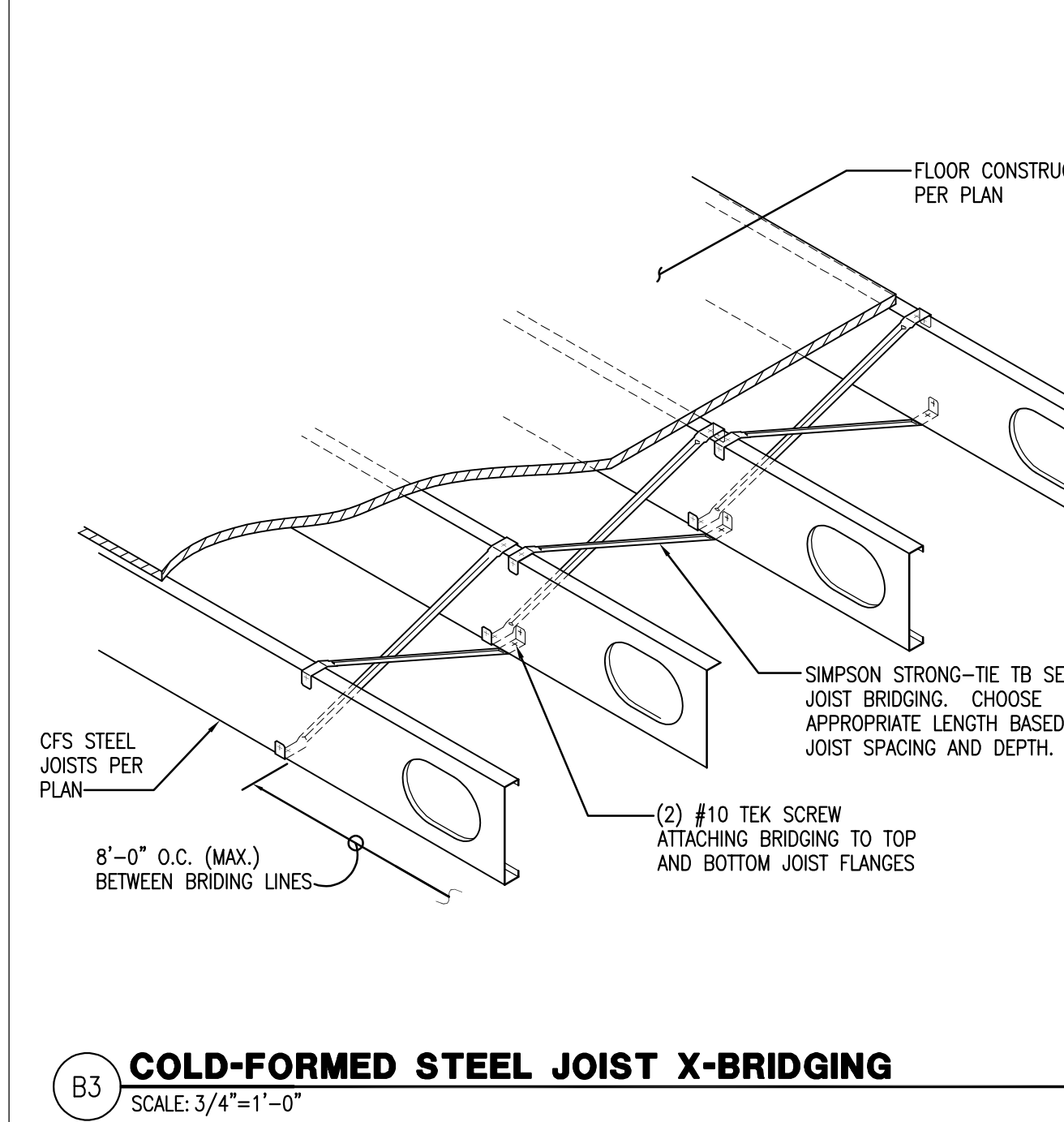
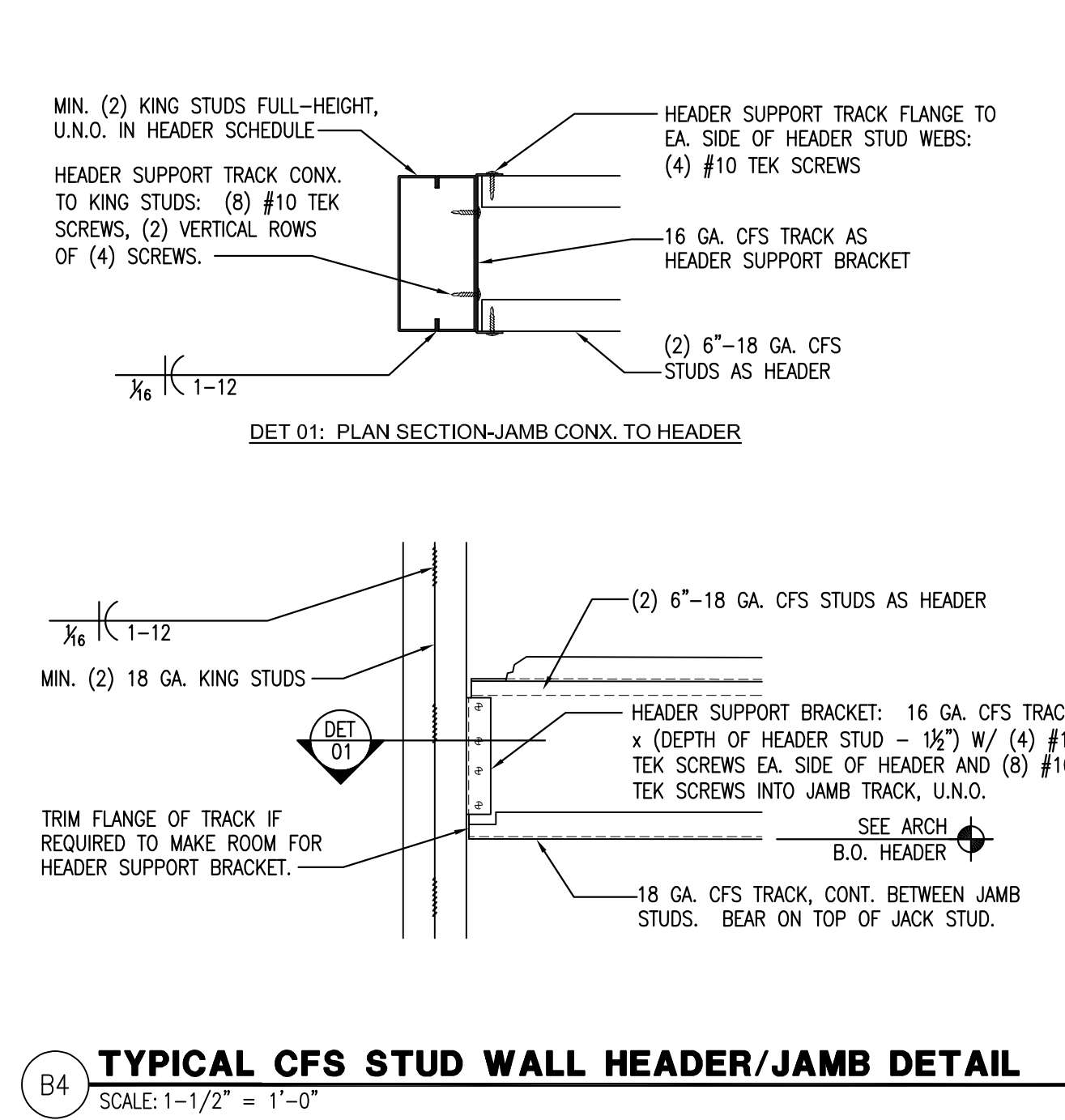
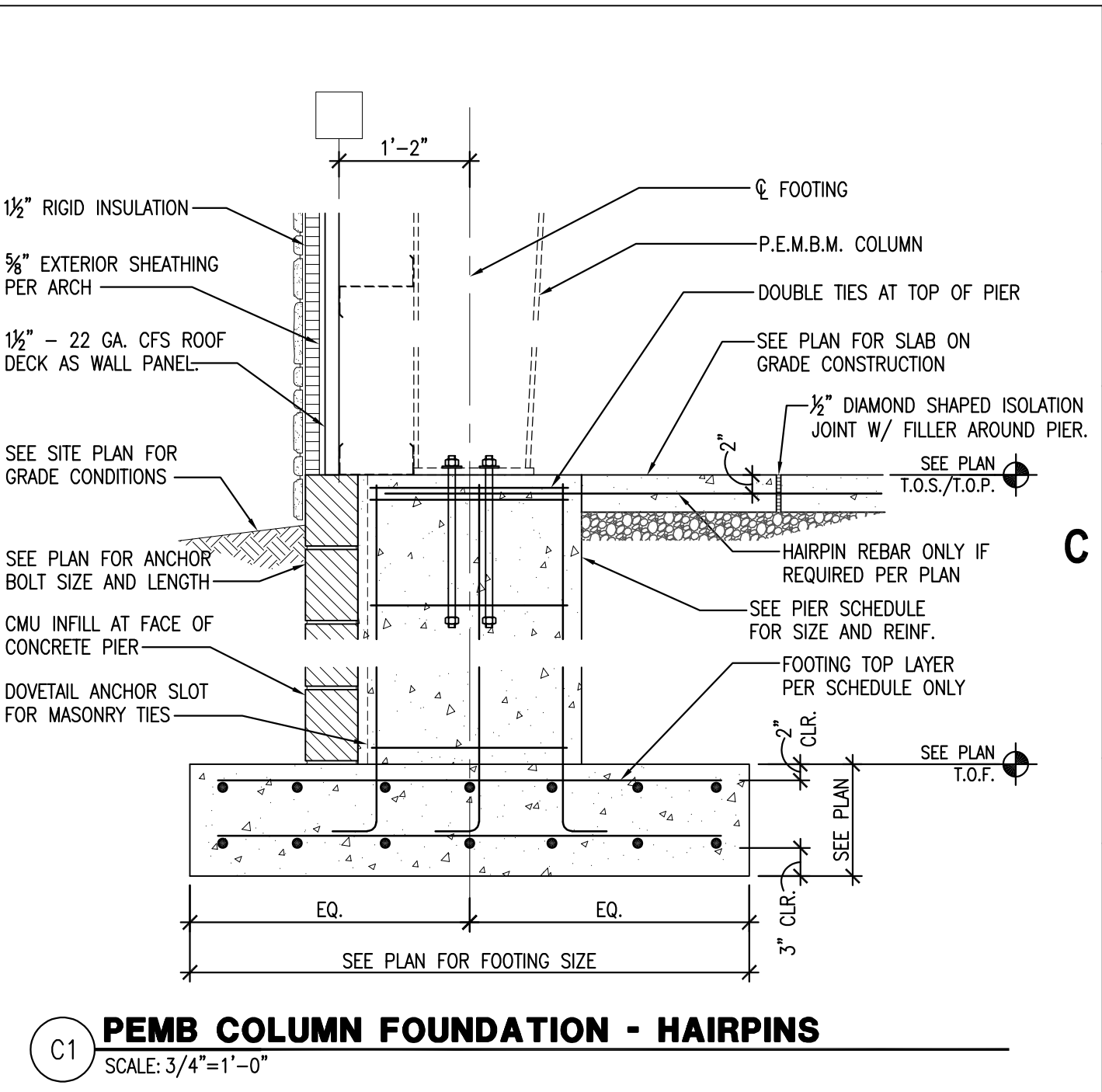
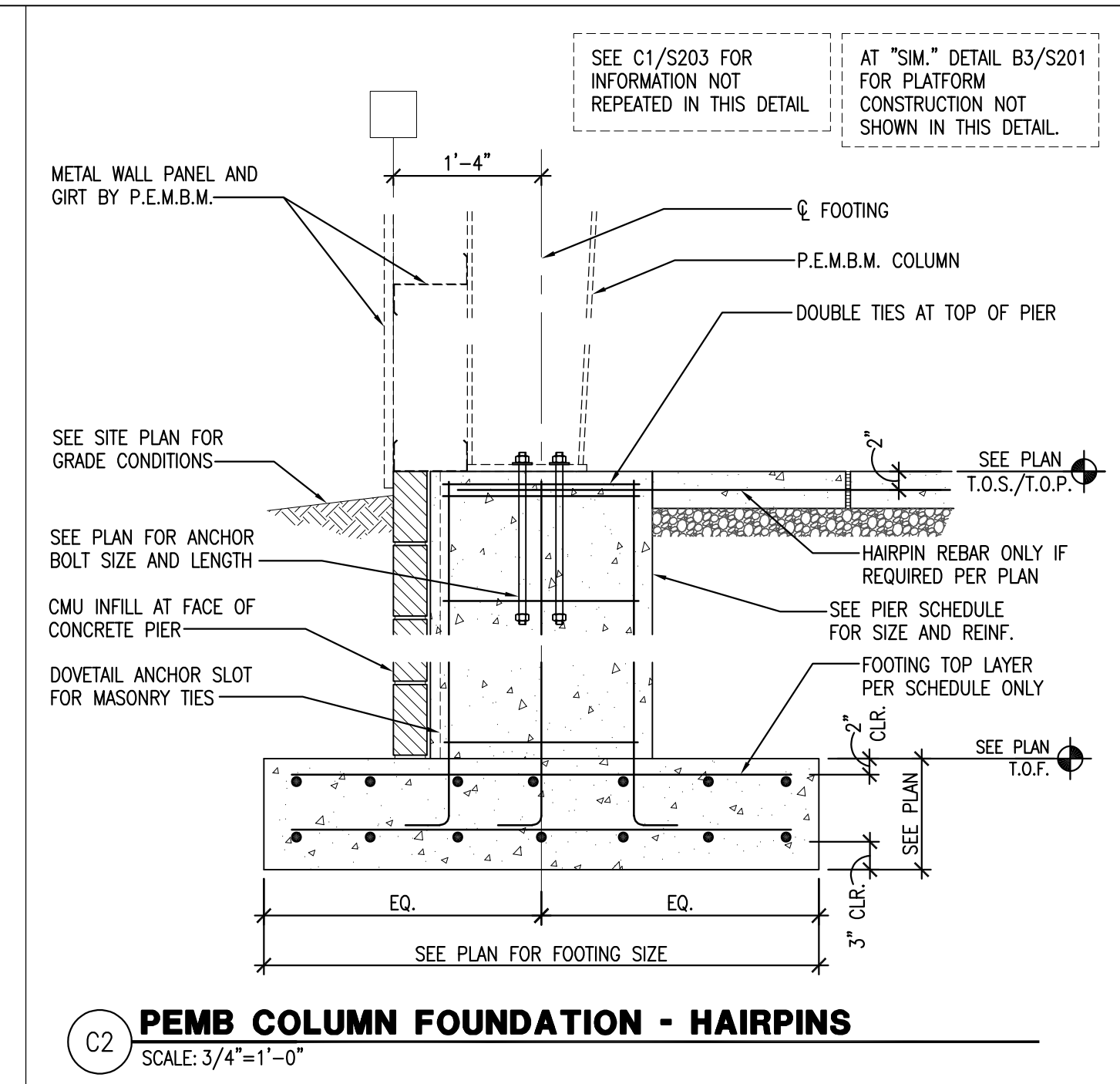
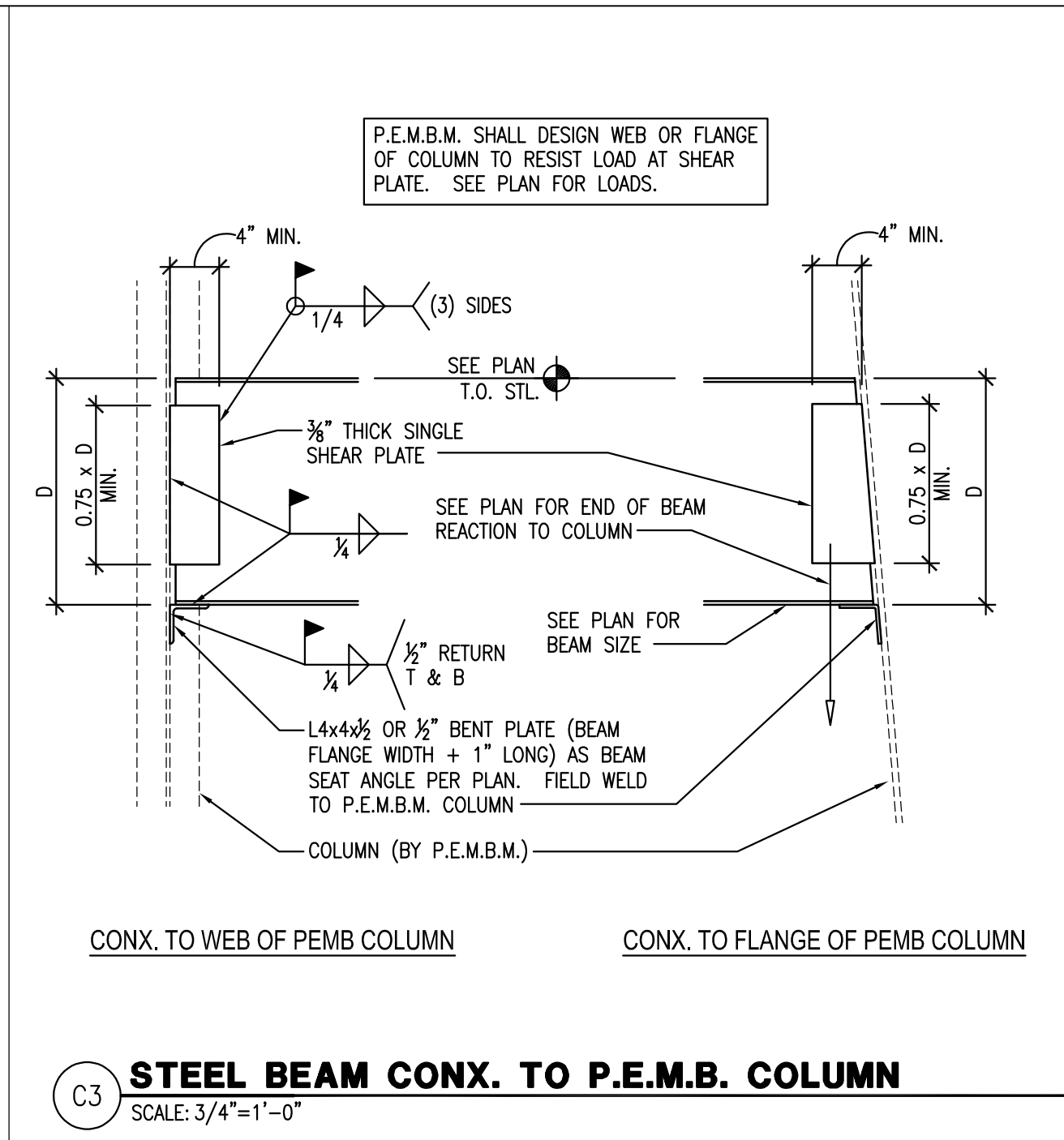
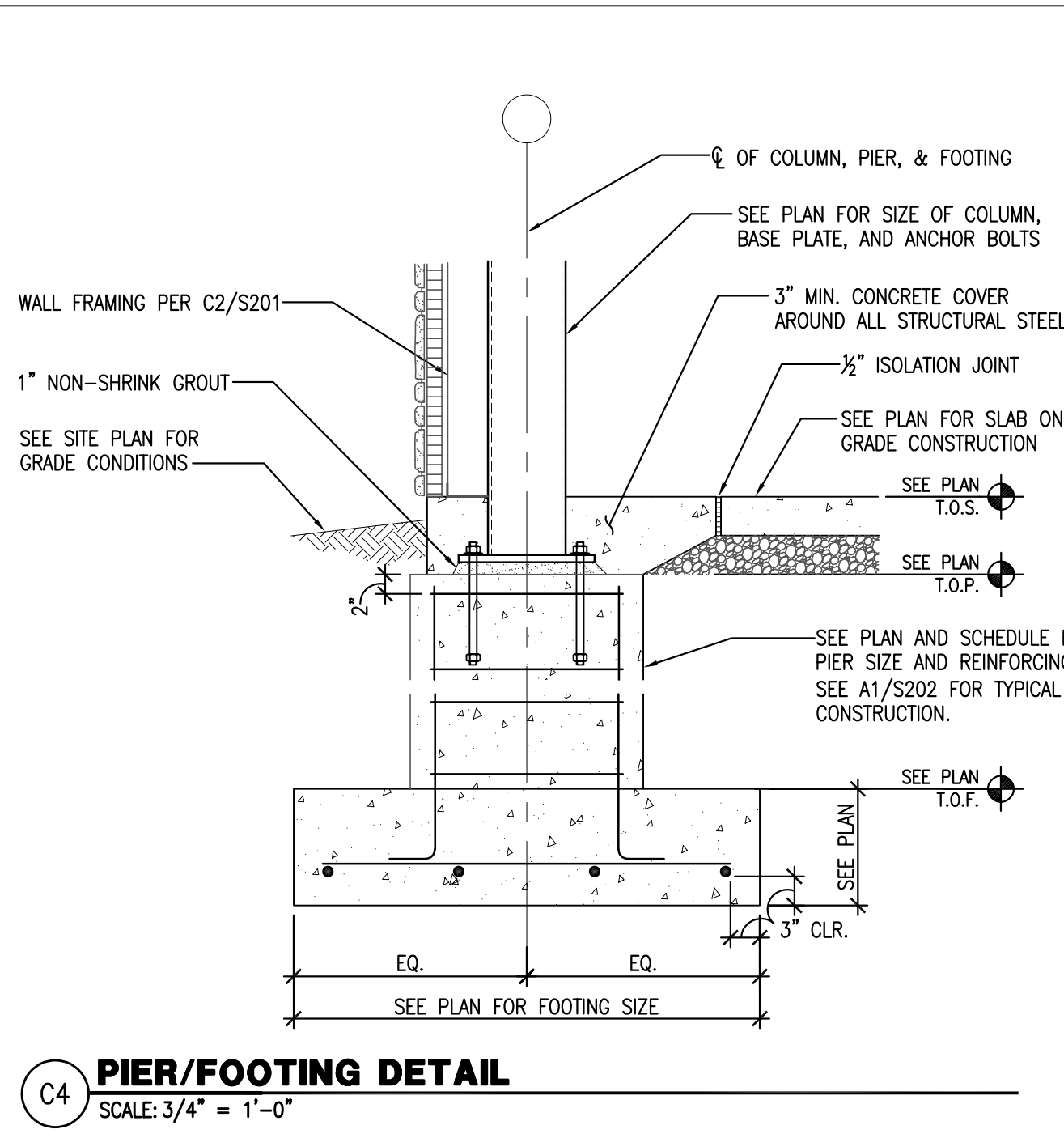
FOYER ROOF FRAMING PLAN

S102
OF 18 SHEETS

216118



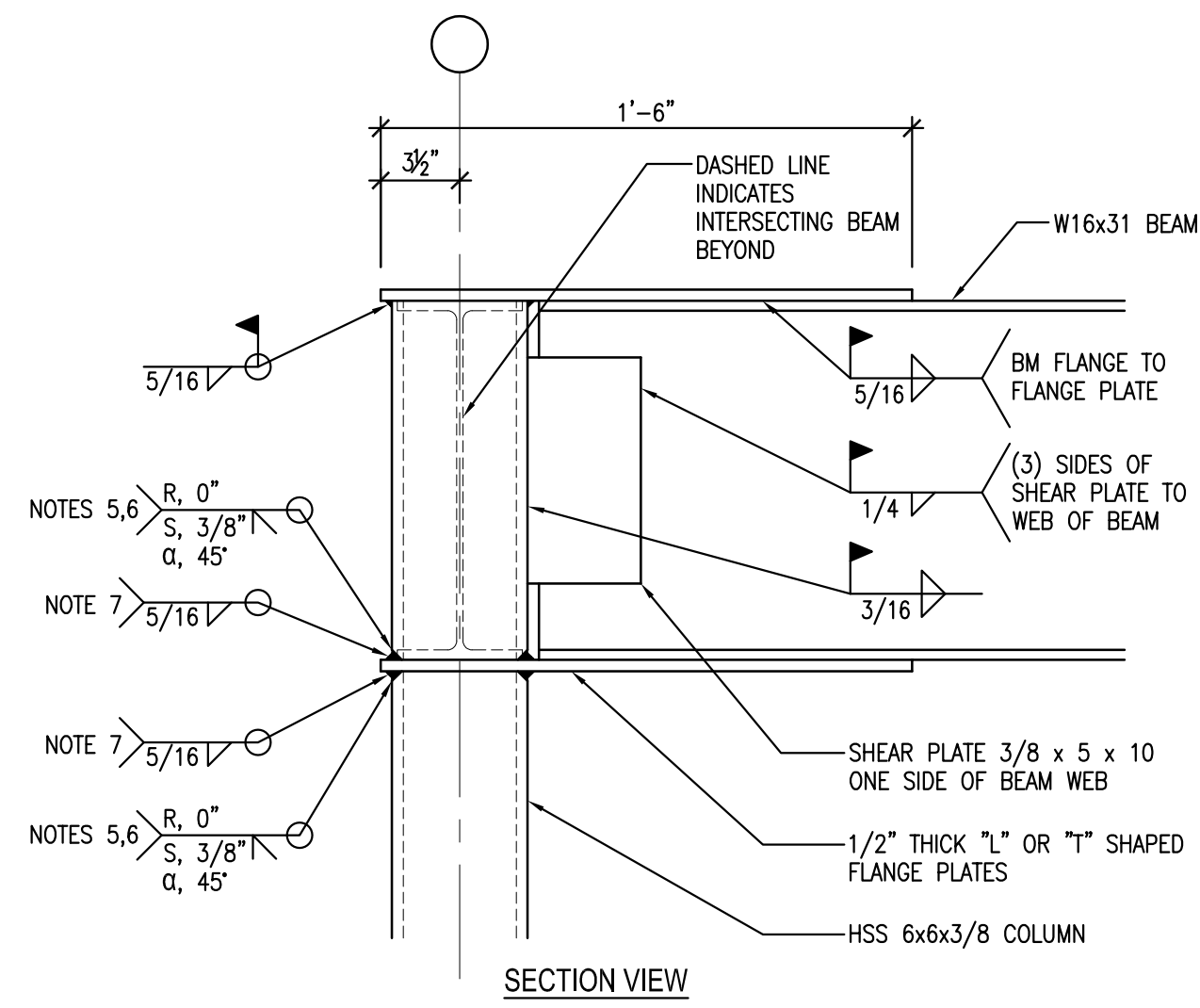
NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
 6300 WOODYARD ROAD
 UPPER MARLBORO, MD 20772



| DRAWING | DATE |
|---|-------------|
| <input checked="" type="checkbox"/> Bid Set | 17 NOV 2023 |

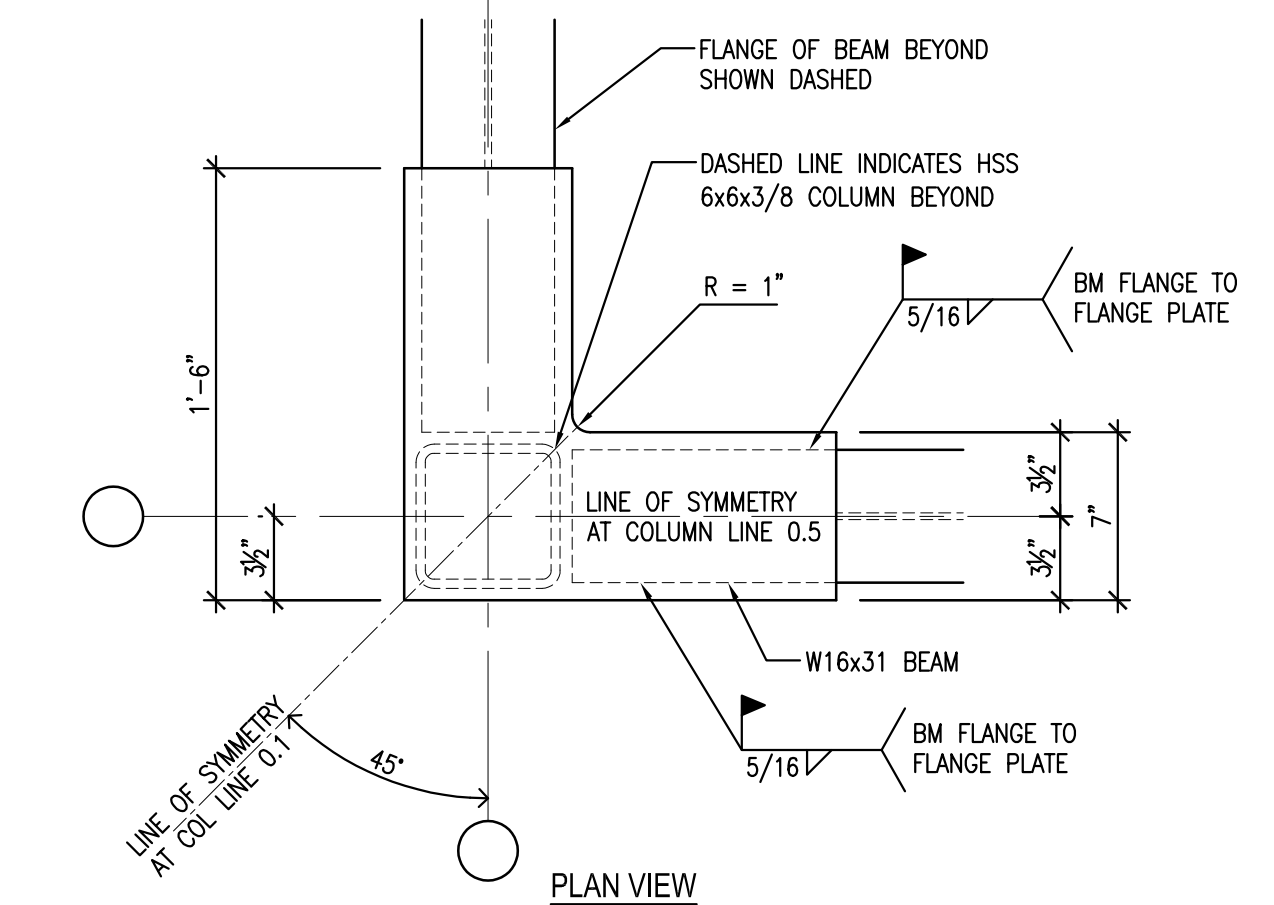
REVISIONS

Plot Date: 11/9/2023 2:04:58 AM s401-galilee baptist roof details.dwg Chris Sekol

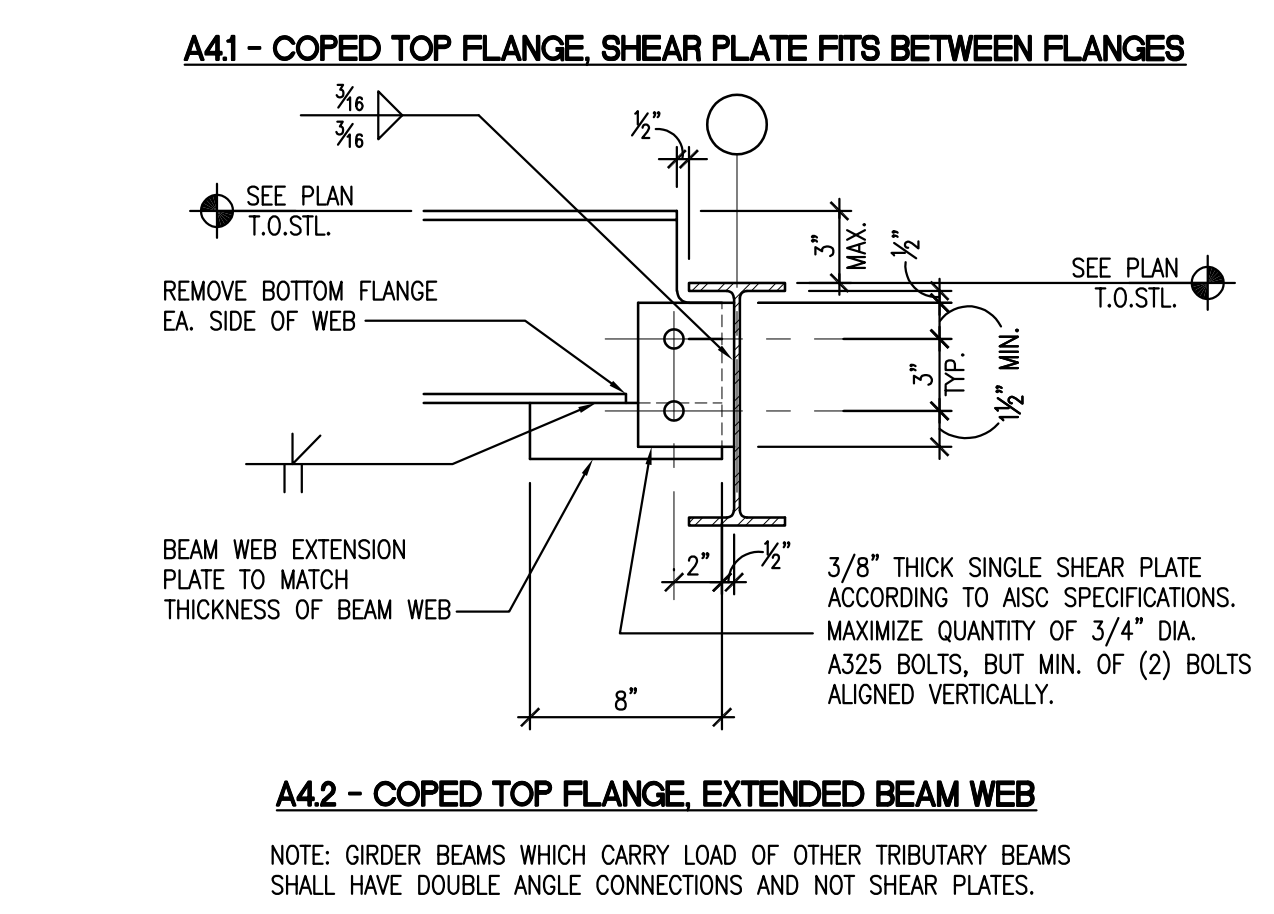
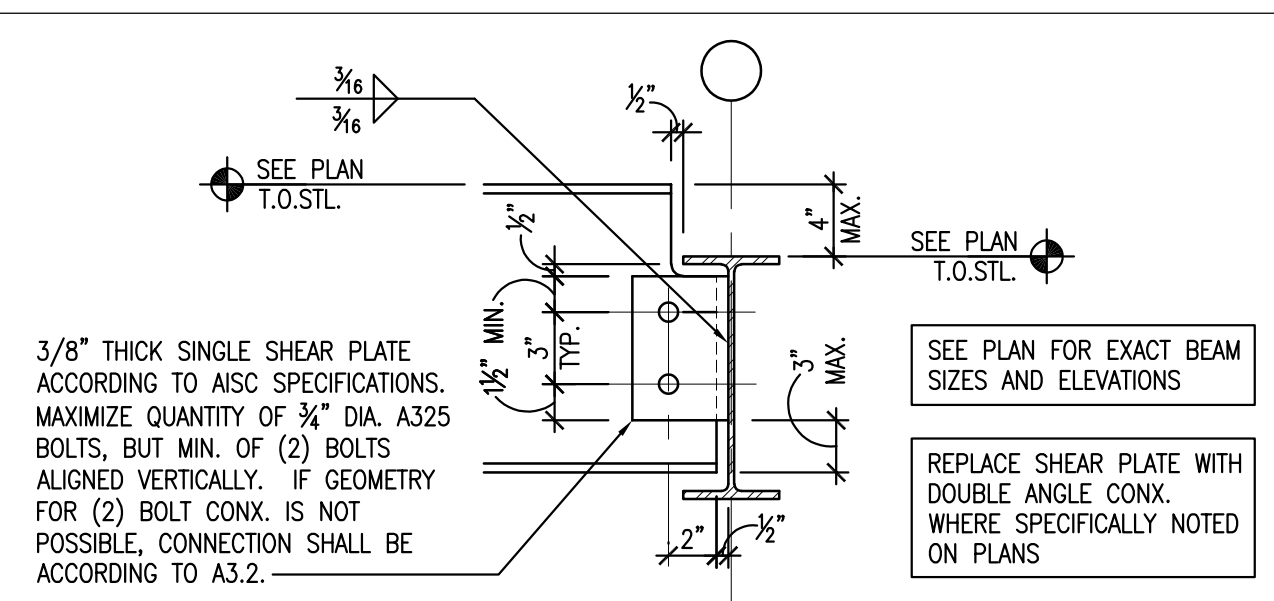


NOTES ON WELDING REQUIREMENTS

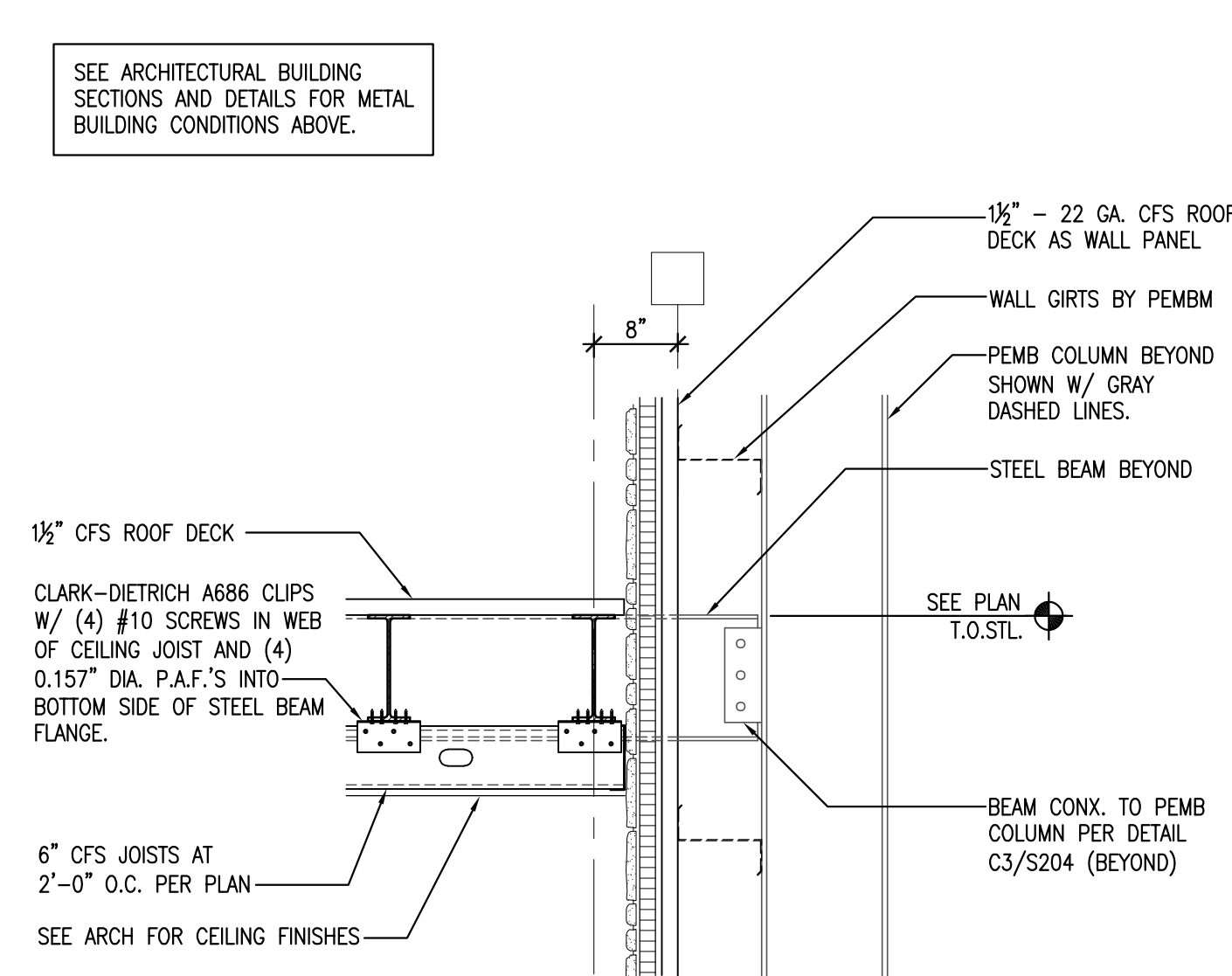
1. PREPARE THE TUBE WALLS WITH A 45° BEVEL WITH GROOVE RADIUS TO MAXIMIZE EFFECTIVE THROAT THICKNESS.
2. FIT-UP THE FLANGE-PLATE AND TUBE WITH A MINIMUM ROOT OPENING.
3. PRE-HEAT THE SPECIMENS AS REQUIRED BY AWS SPECIFICATIONS.
4. PREPARE SURFACES FOR WELDING AS REQUIRED BY AWS SPECIFICATIONS.
5. PLACE TEMPORARY TACK WELDS TO MAINTAIN ALIGNMENT OF SPECIMENS DURING PENETRATION WELDING. GRIND AWAY ALL TACK WELDS DURING THE PENETRATION WELDING PROCESS. DO NOT FILL OVER TACK WELDS.
6. PLACE PARTIAL PENETRATION WELDS ON TUBE WALLS (DEMAND CRITICAL).
7. GRIND SMOOTH PARTIAL PENETRATION WELD BEFORE PLACING THE 5/16" REINFORCING FILLET WELDS.



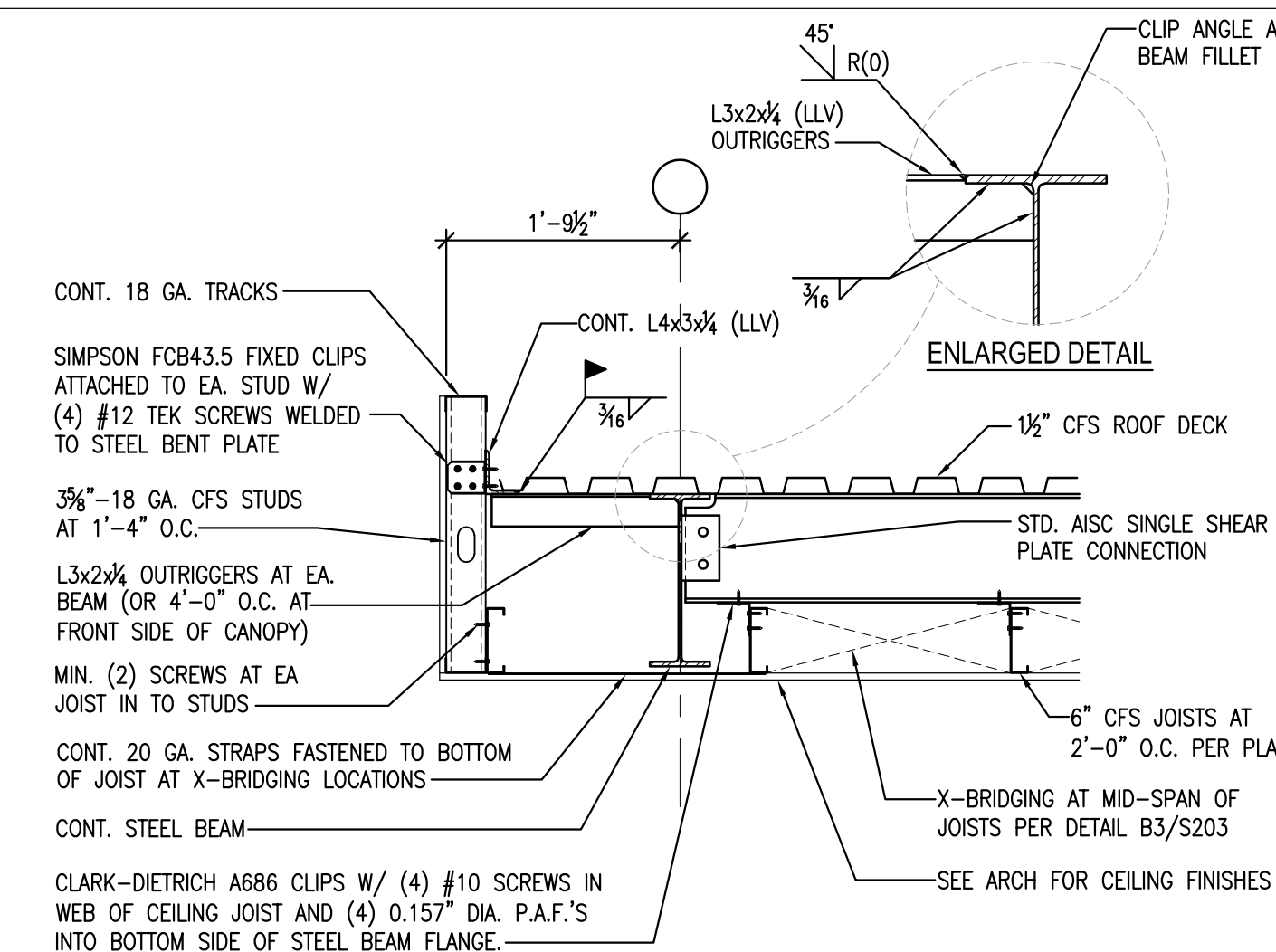
B4 BEAM/COLUMN MOMENT CONNECTION
SCALE: 1 1/2" = 1'-0"



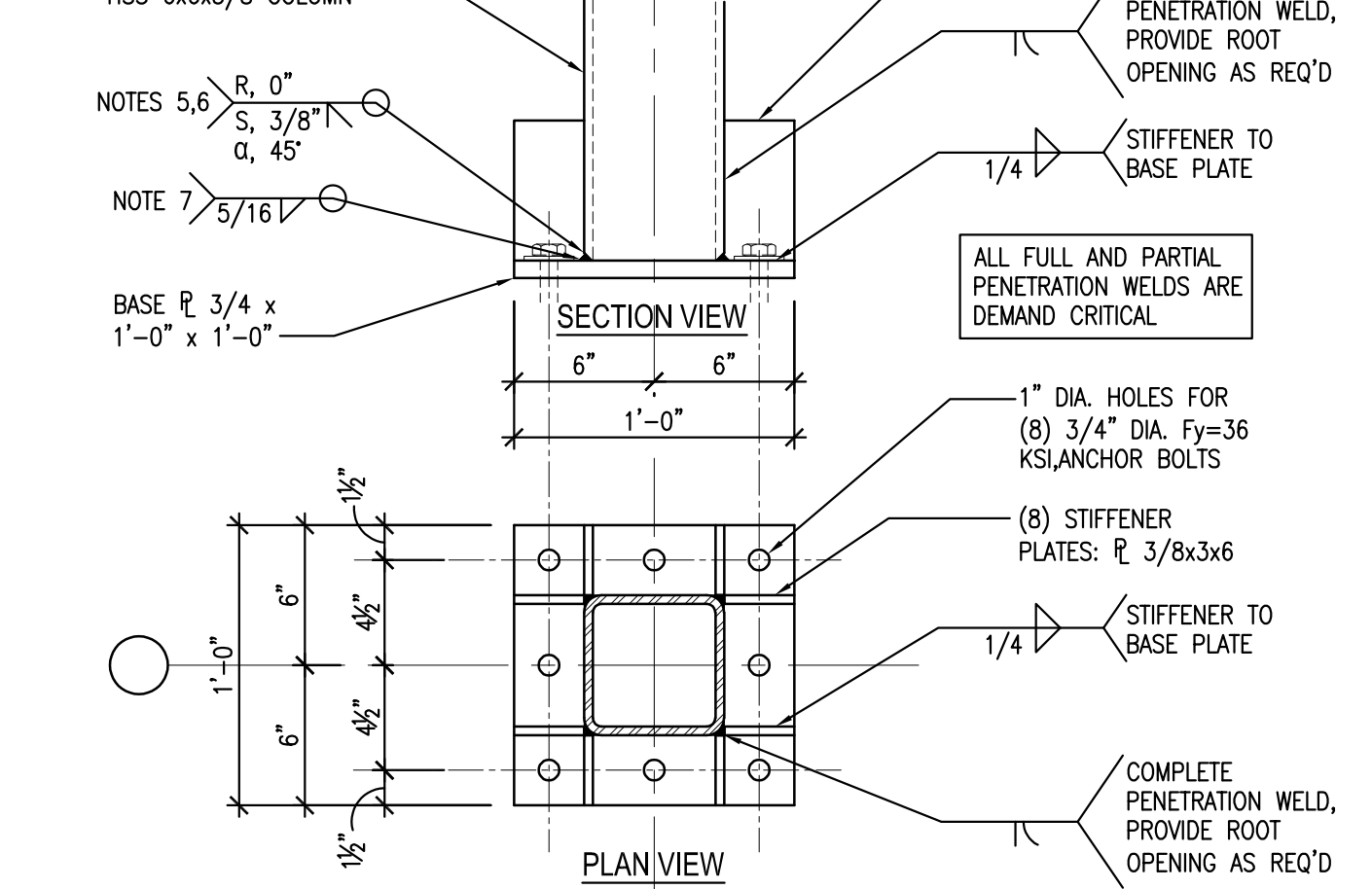
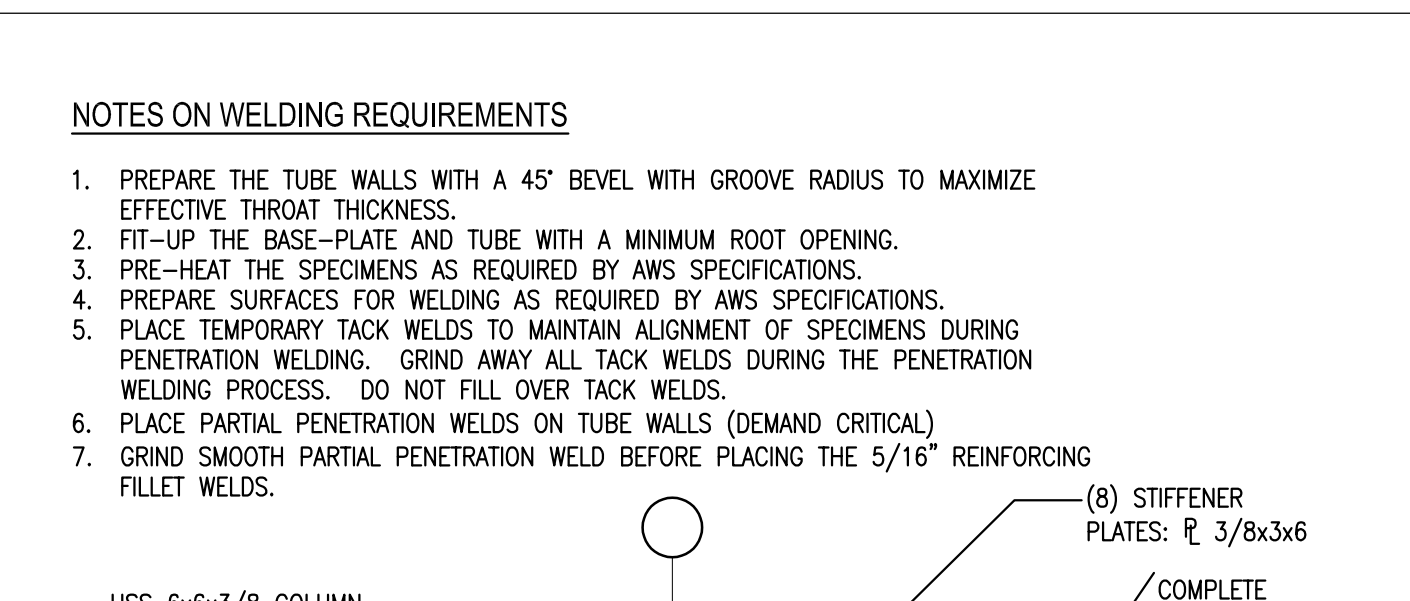
A4 TYPICAL COPED BEAM TO BEAM CONX'S
SCALE: 1-1/2" = 1'-0"



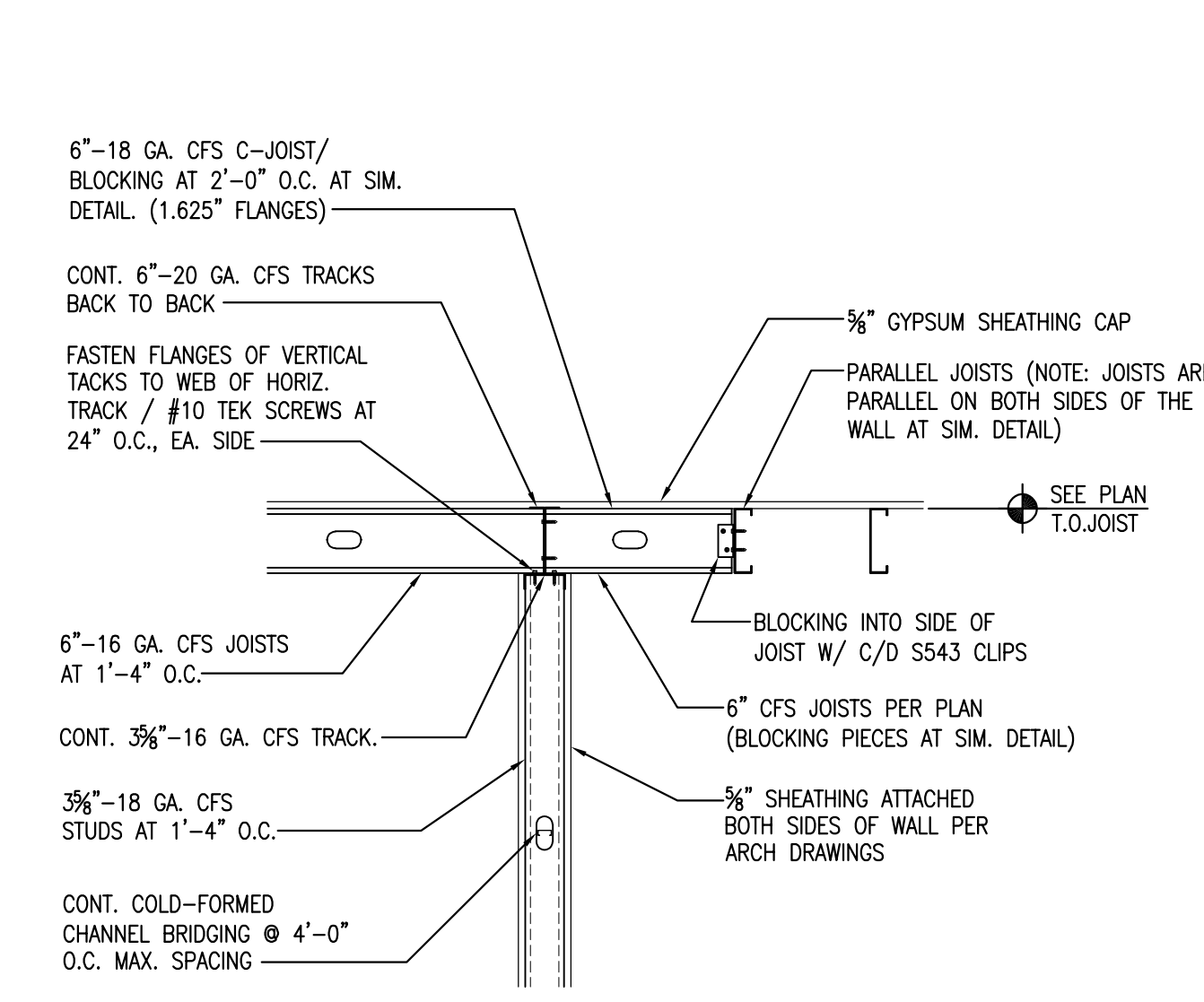
C3 ALT ADD: SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"



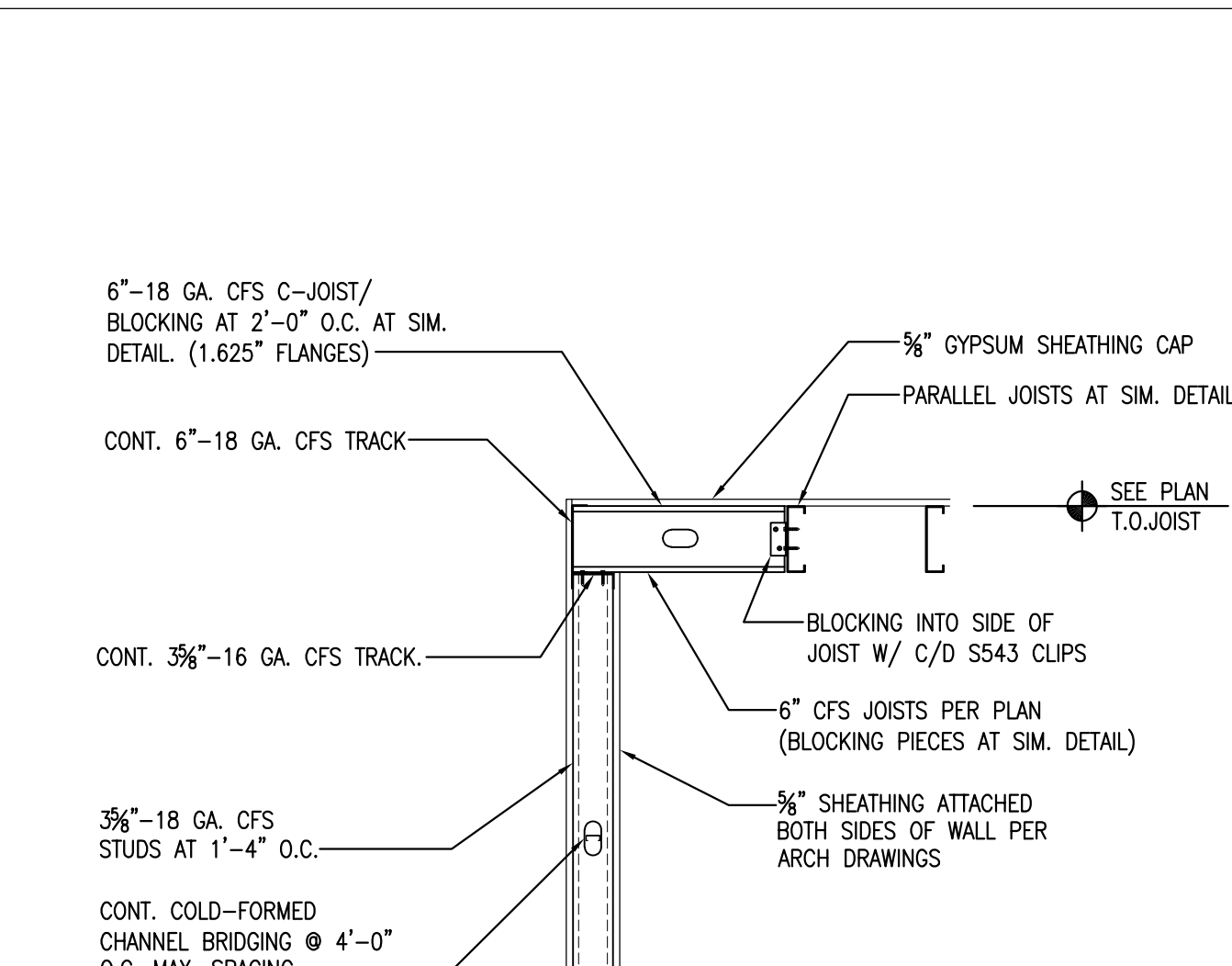
B3 ALT ADD: SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"



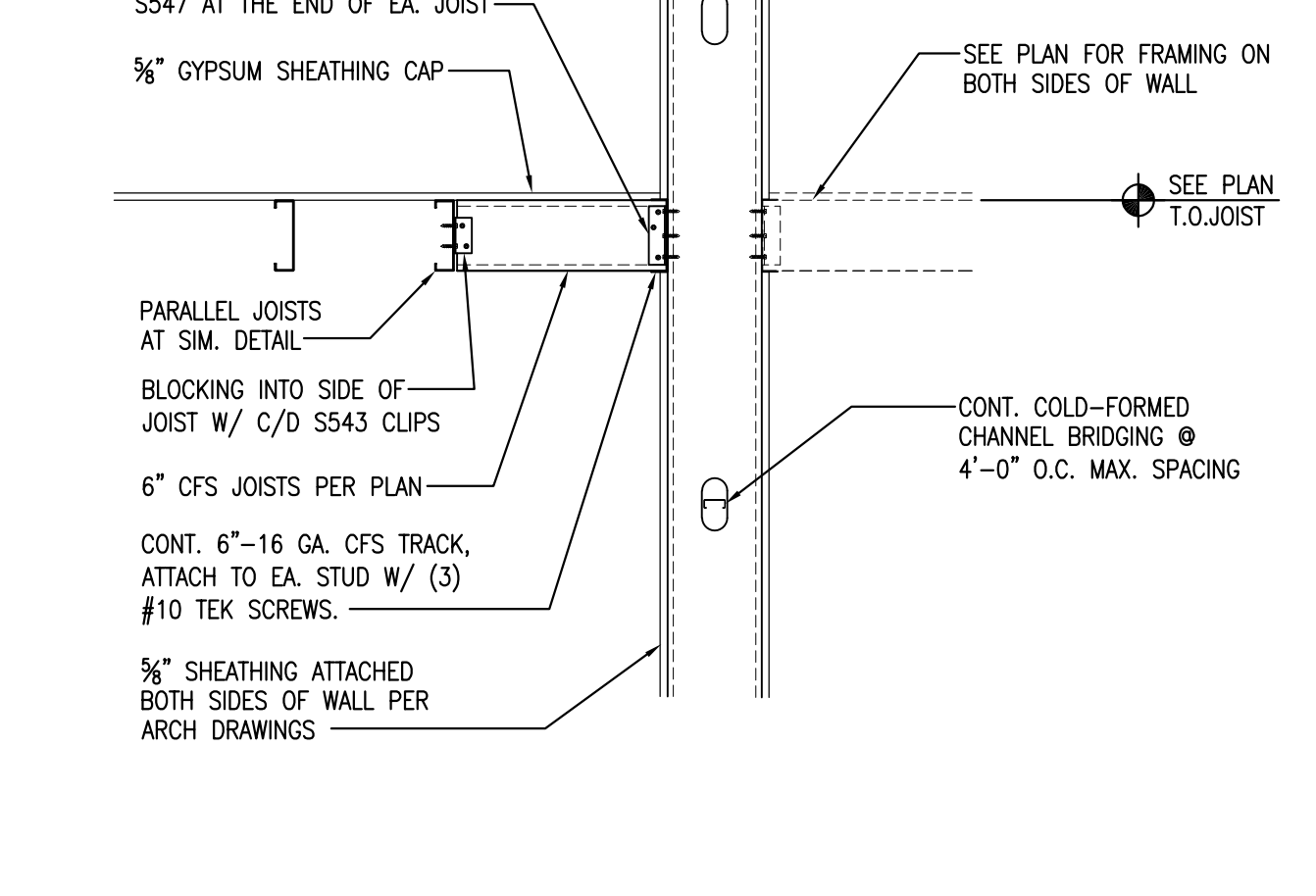
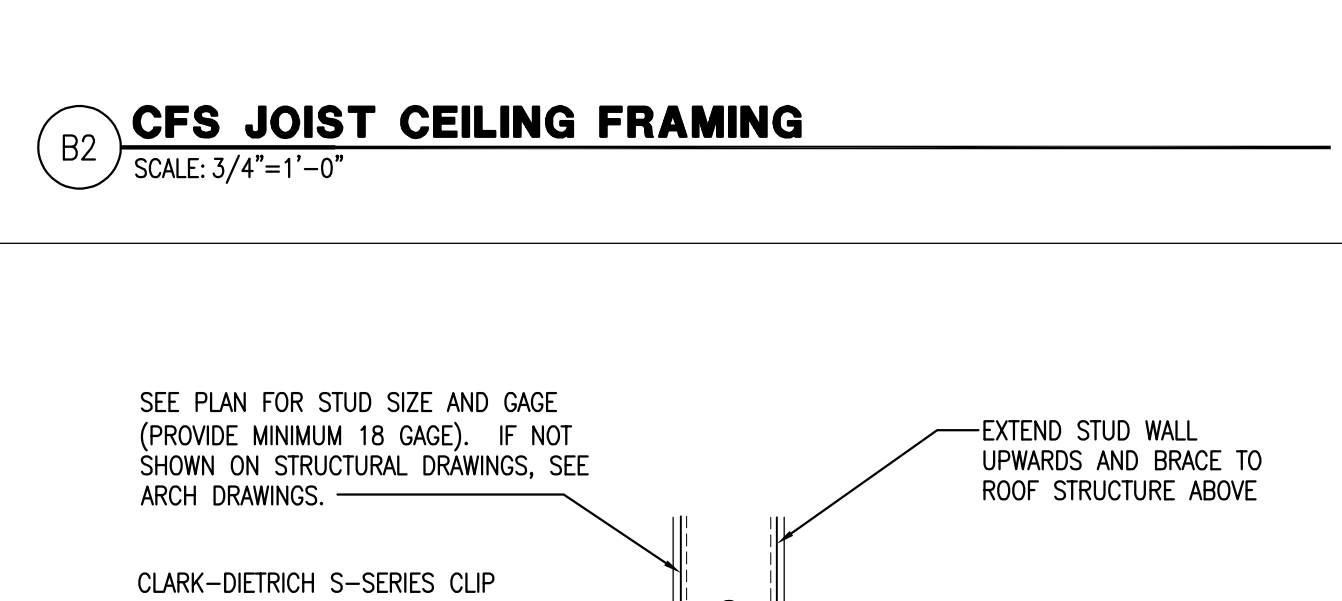
A3 MOMENT BASE PLATE
SCALE: 1 1/2" = 1'-0"



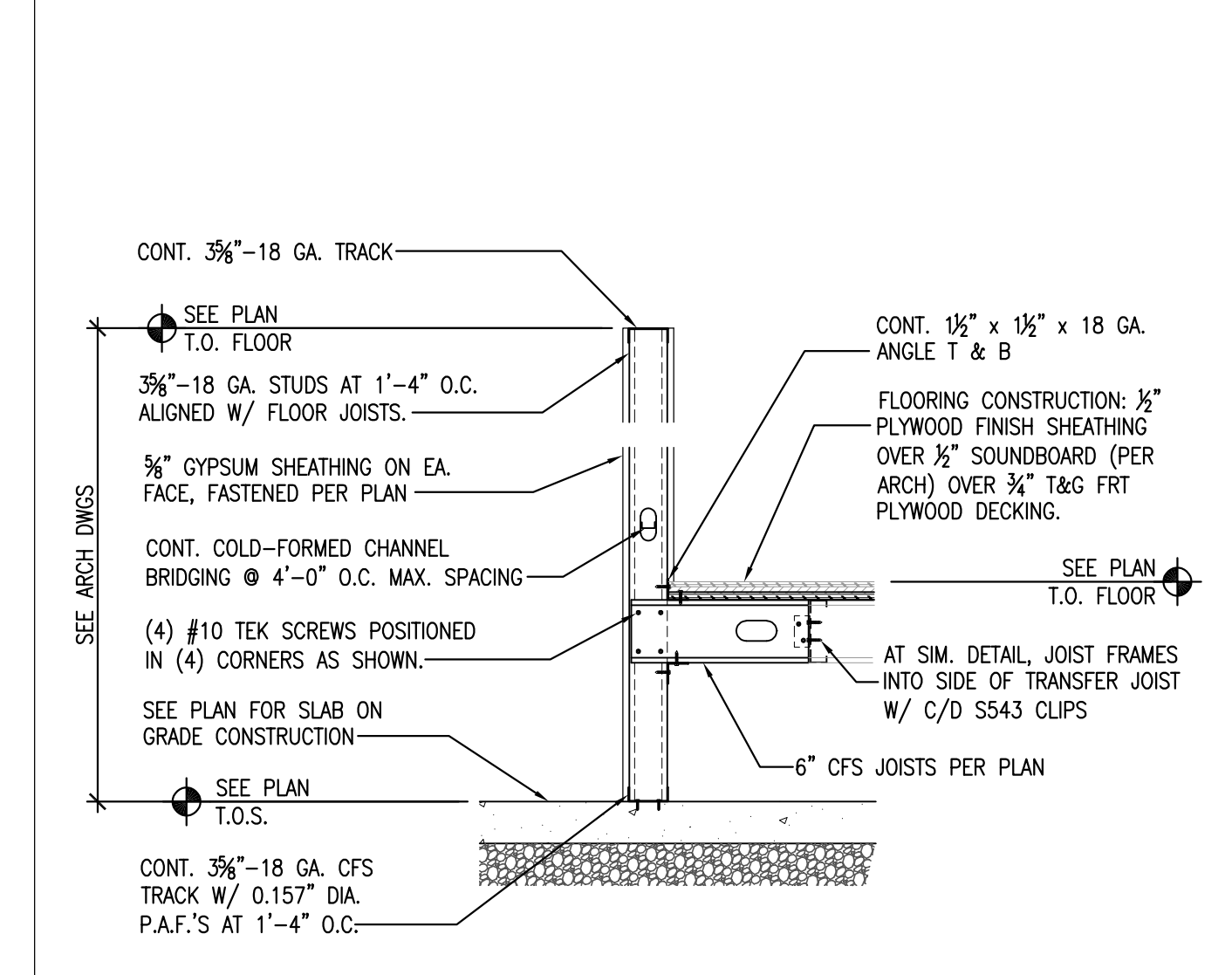
C2 CFS JOIST CEILING FRAMING
SCALE: 3/4" = 1'-0"



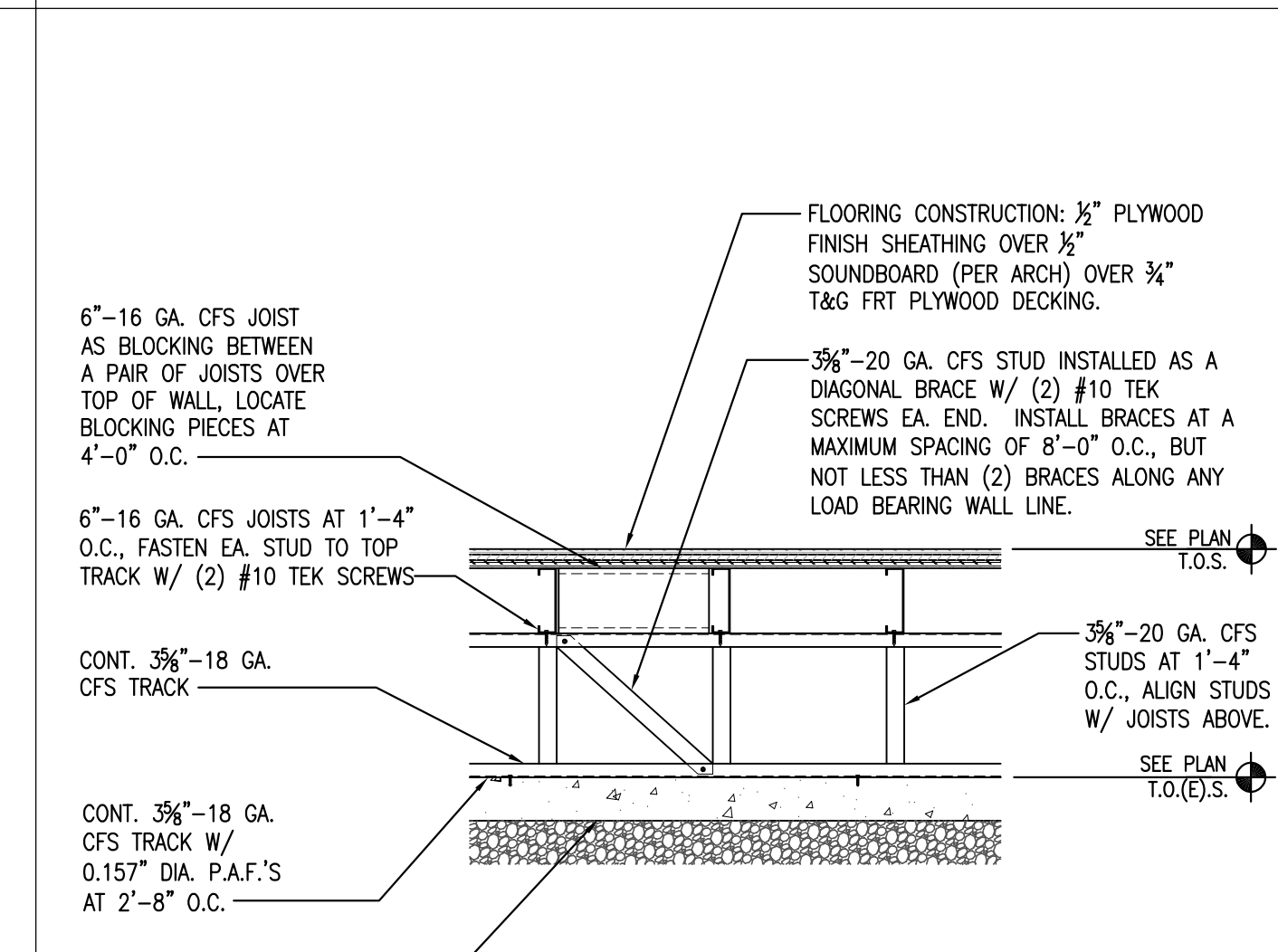
B2 CFS JOIST CEILING FRAMING
SCALE: 3/4" = 1'-0"



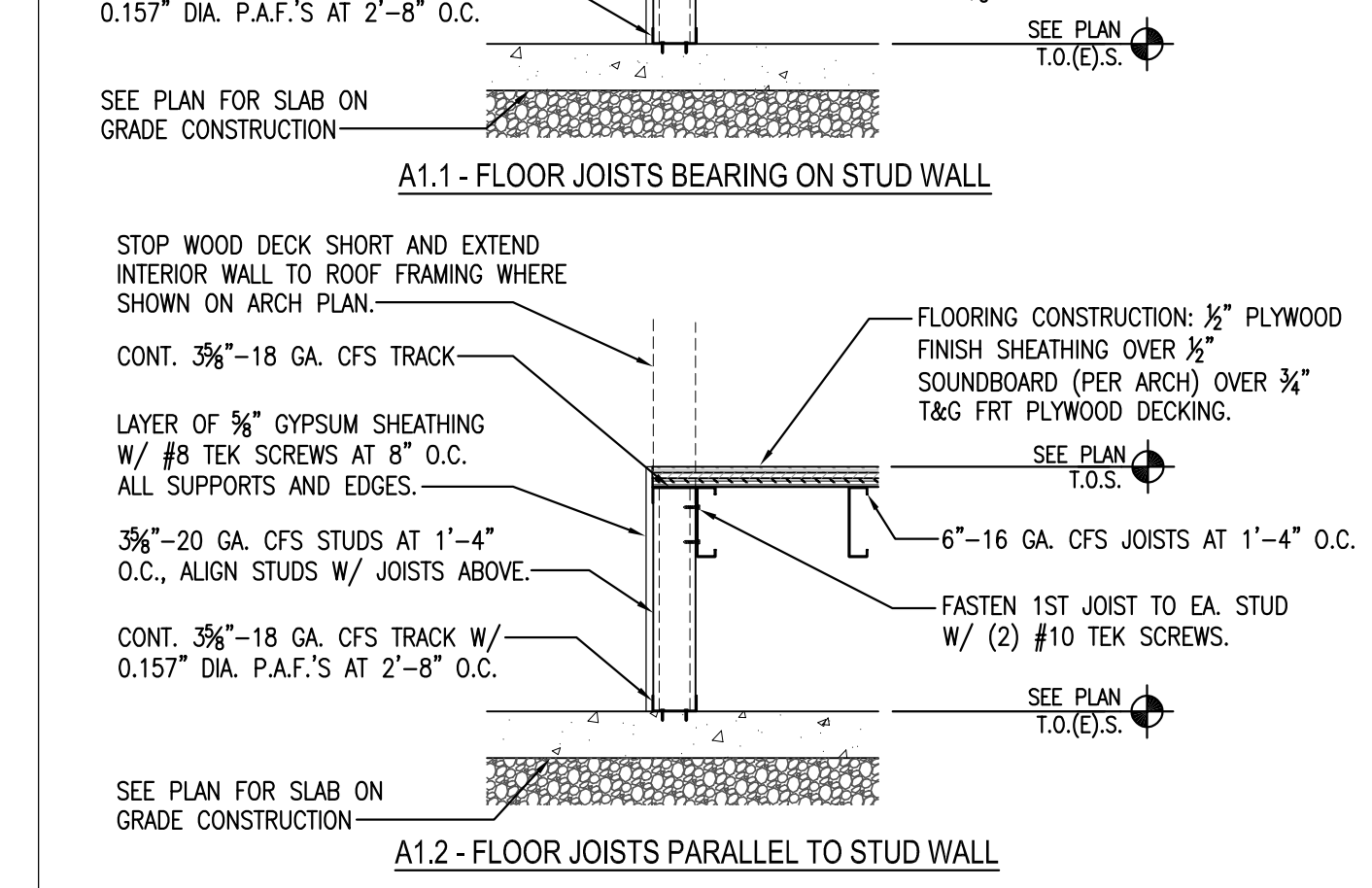
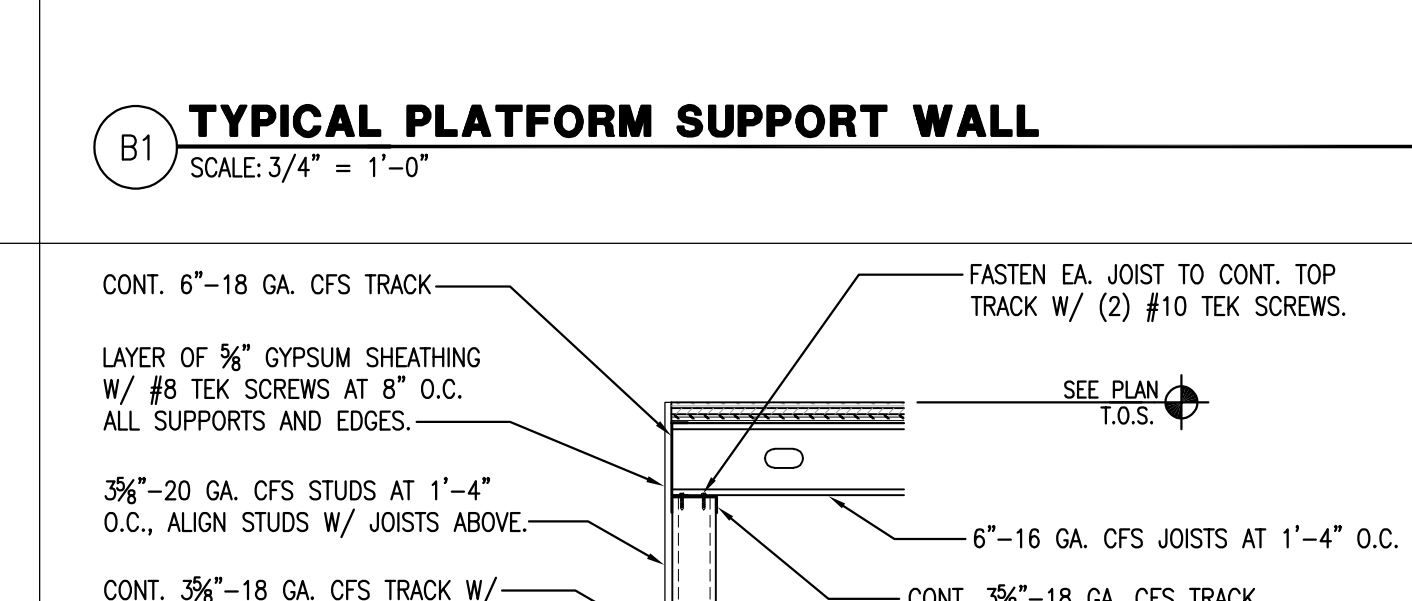
A2 CFS JOIST CEILING FRAMING
SCALE: 3/4" = 1'-0"



C1 PLATFORM FLOOR FRAMING
SCALE: 3/4" = 1'-0"



B1 TYPICAL PLATFORM SUPPORT WALL
SCALE: 3/4" = 1'-0"



A1 TYPICAL EDGE PLATFORM CONSTRUCTION
SCALE: 3/4" = 1'-0"

The McKnight Group
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Grove City, Ohio 43123
Phone: (614) 873-1655
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STATE OF MARYLAND
CHRISTOPHER P. SEKOL
PROFESSIONAL ENGINEER
No. 26219

LICENSE EXPIRES: 06-21-2025
DRAWING ISSUED: 11-17-2023

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NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
6300 WOODYARD ROAD
UPPER MARLBORO, MD 20772

DRAWING DATE
17 NOV 2023

REVISIONS

FOUNDATION DETAILS

S204
OF 18 SHEETS

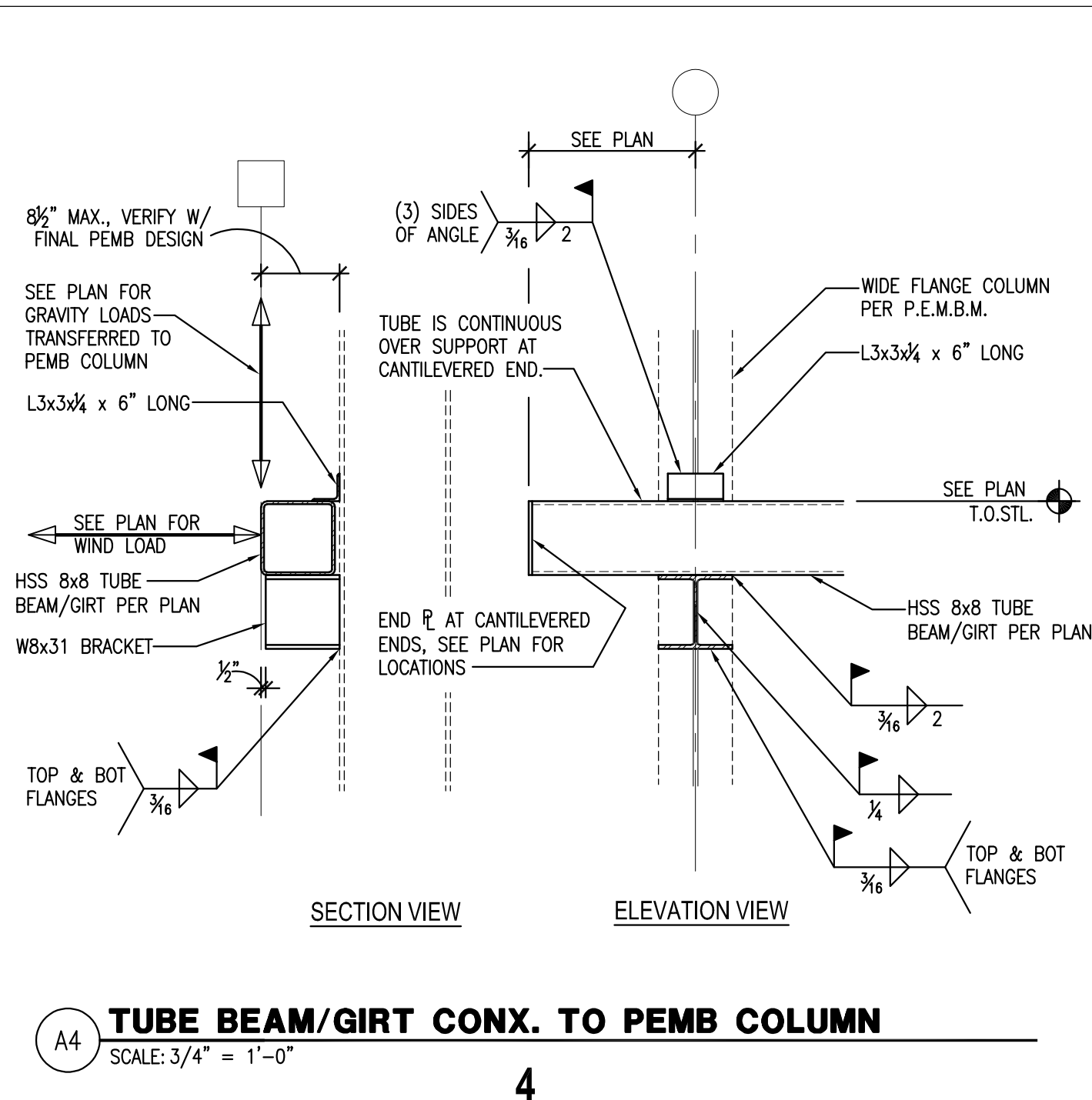
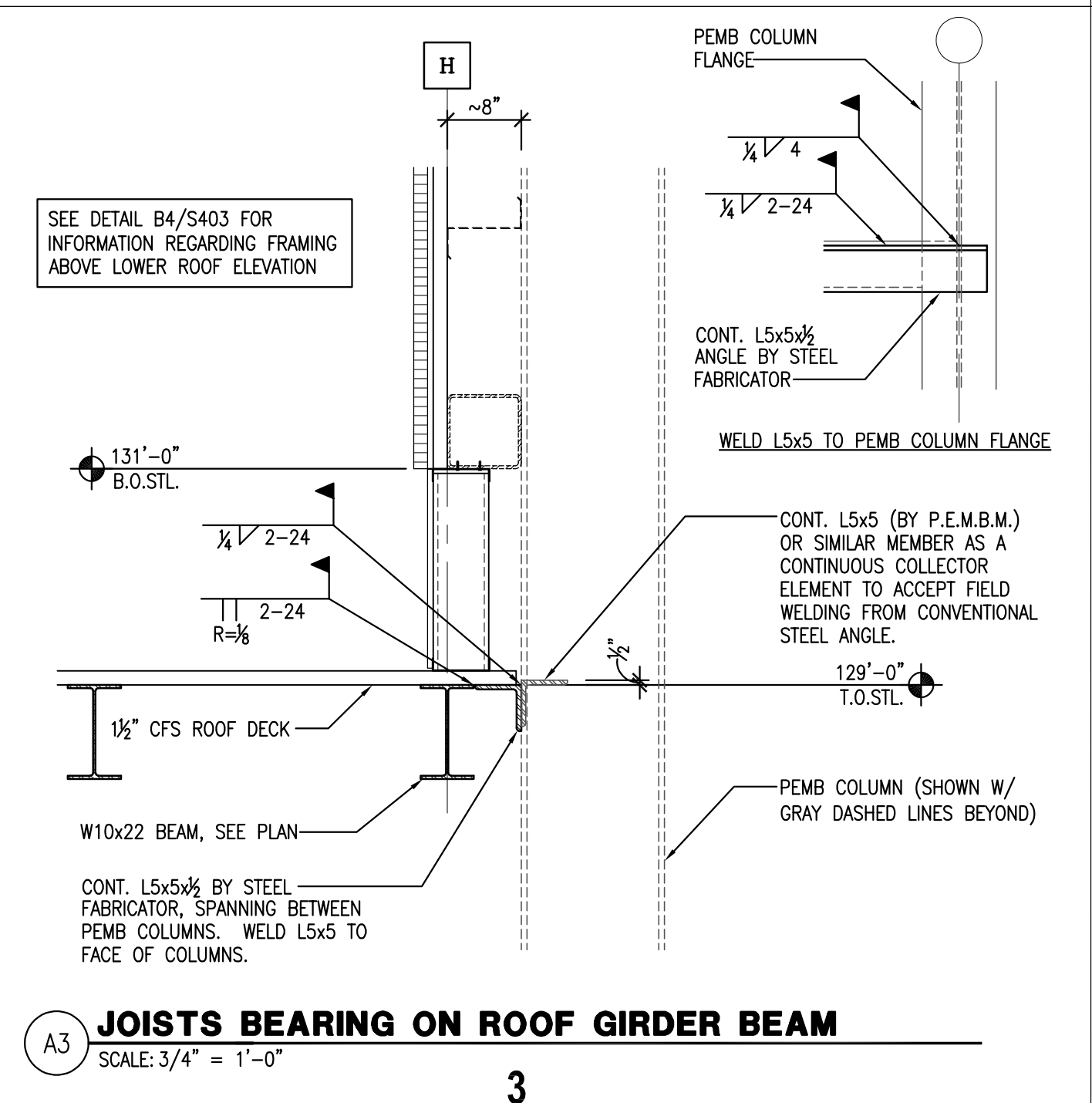
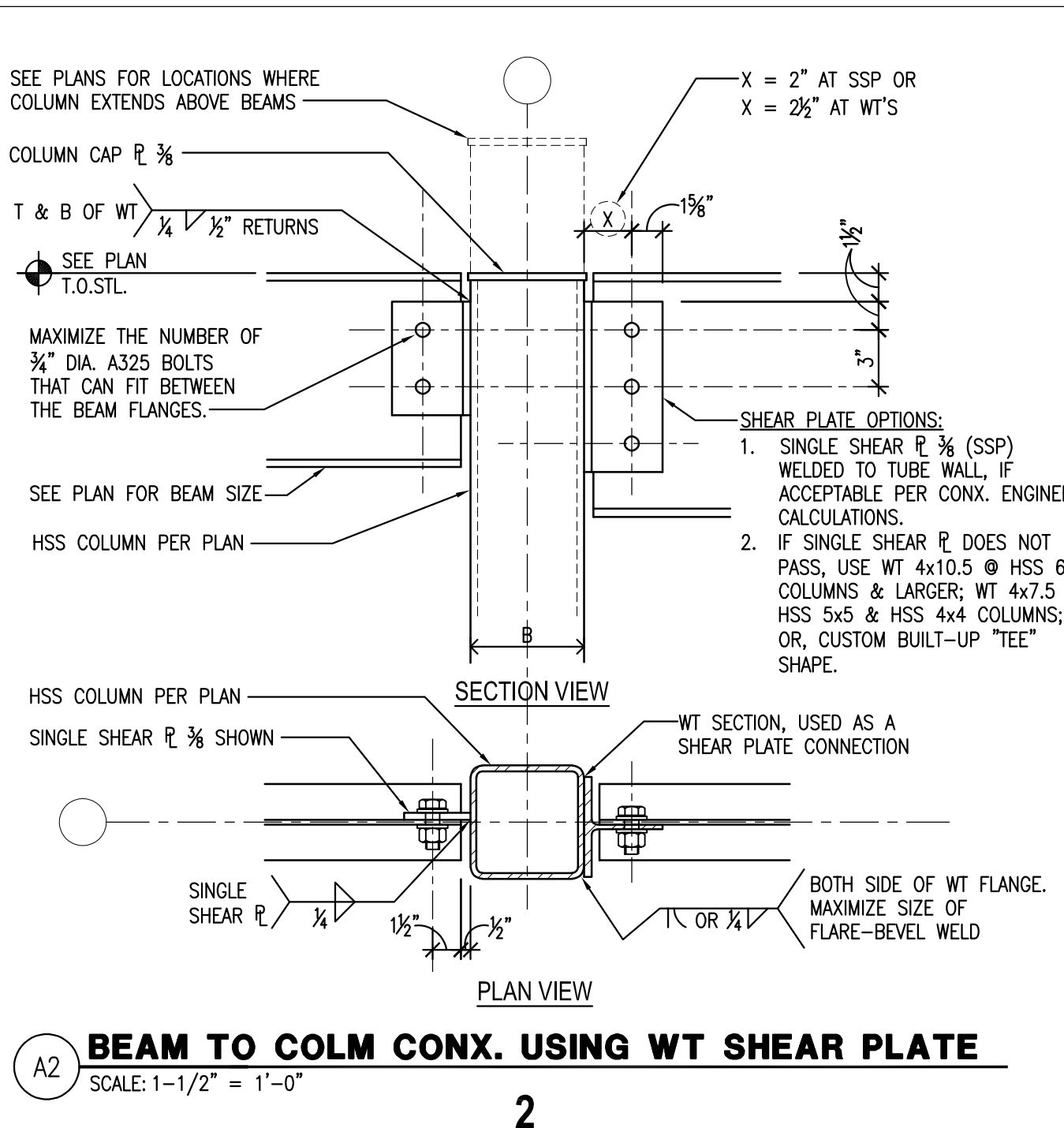
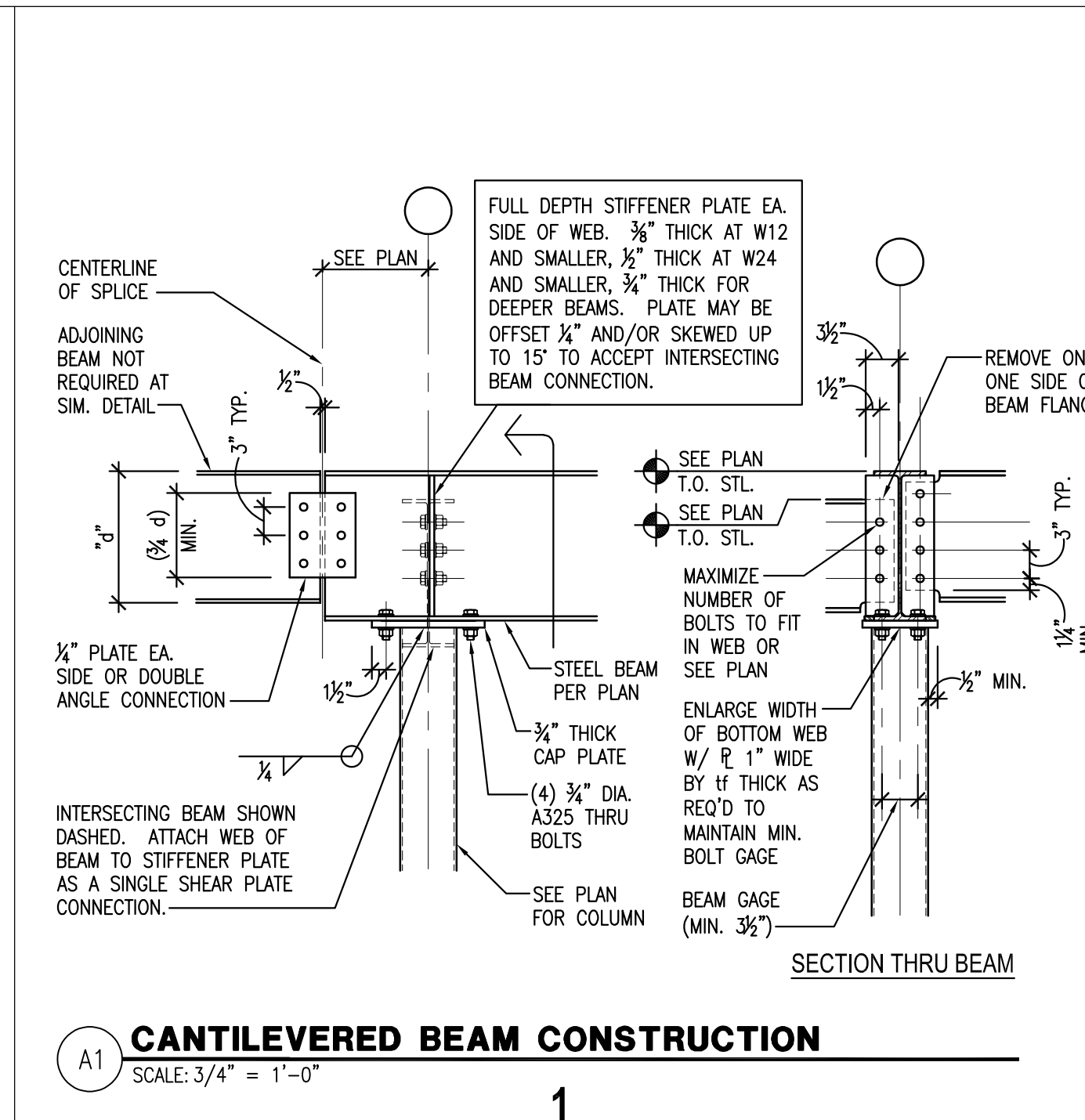
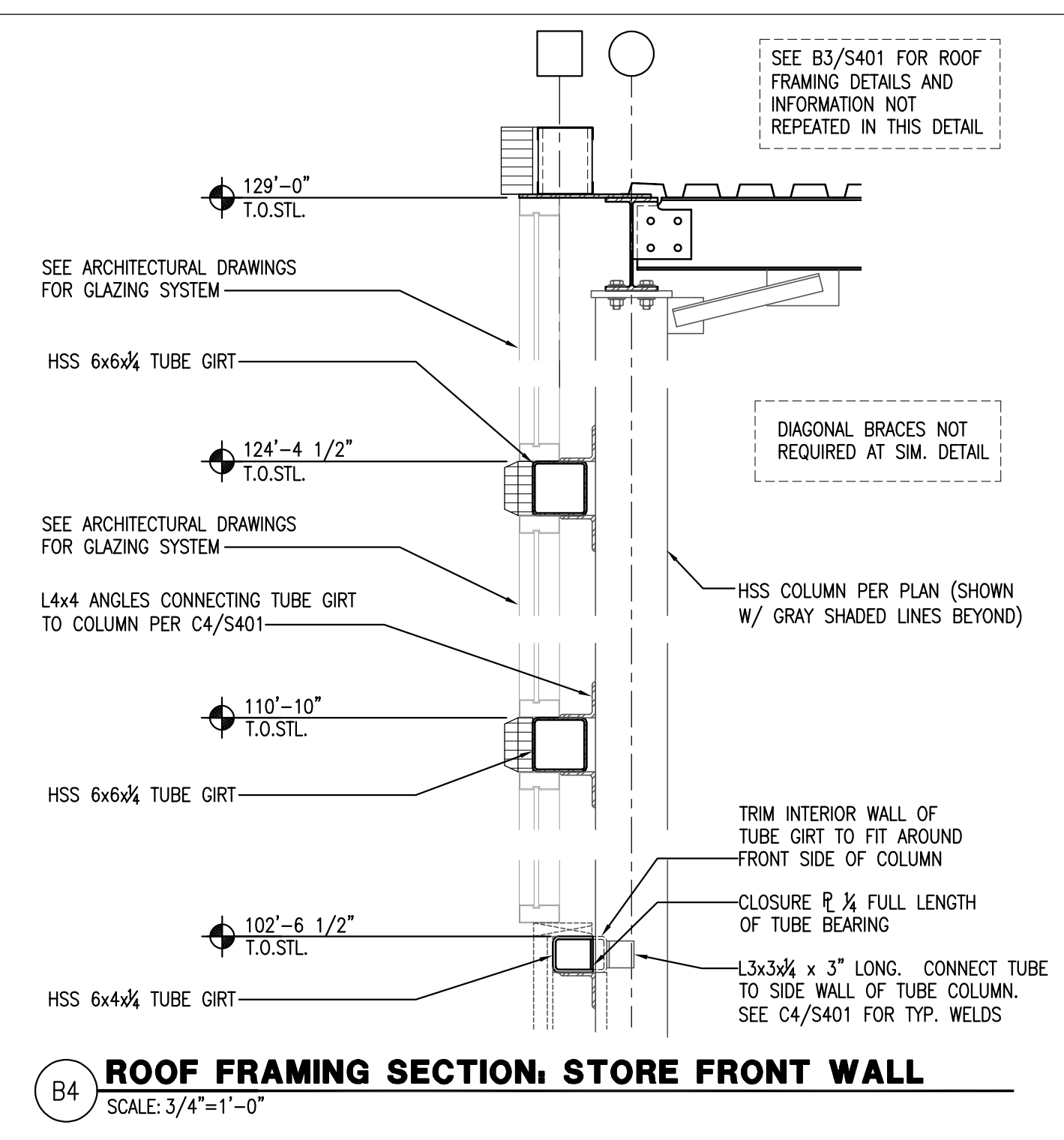
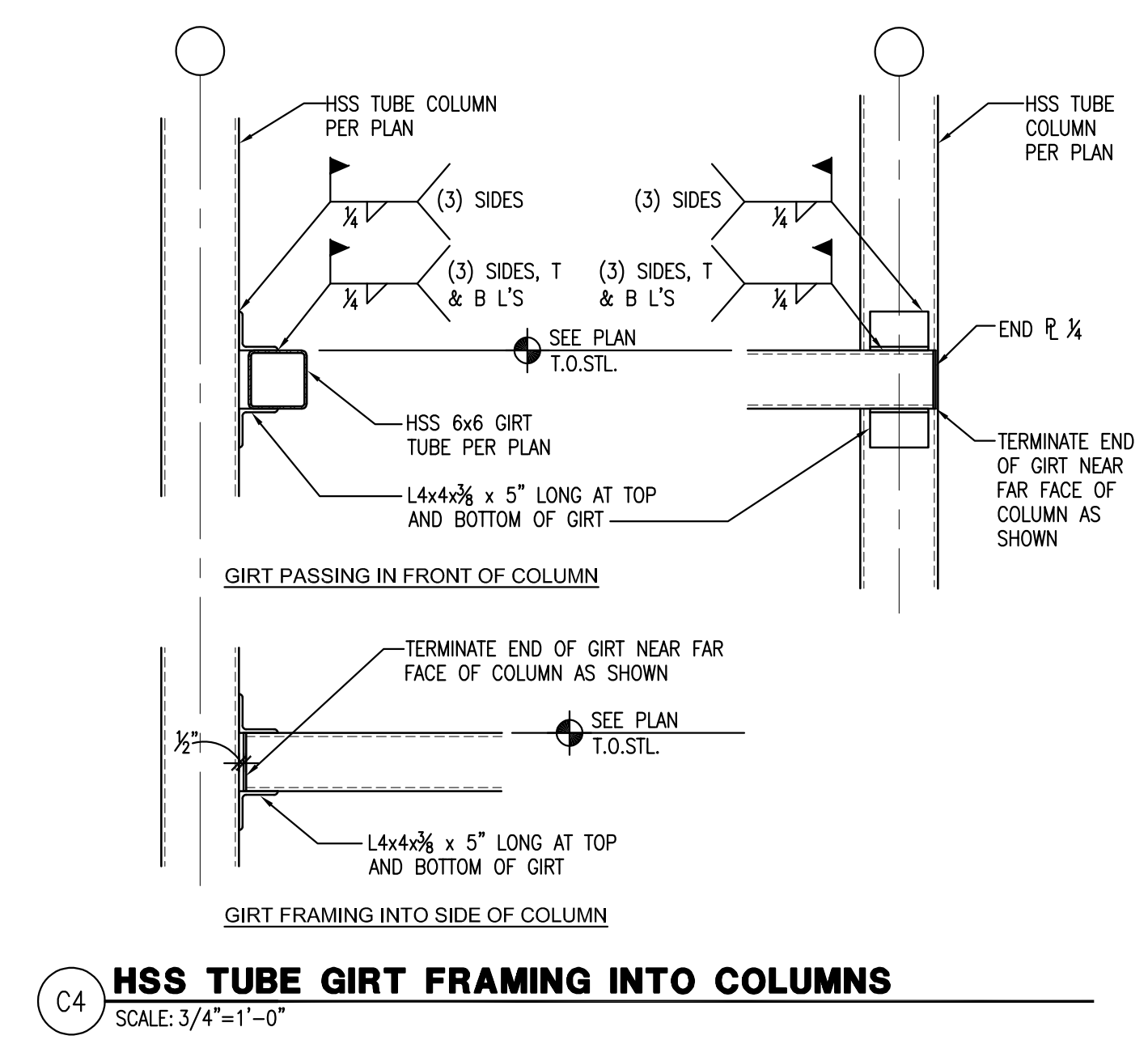
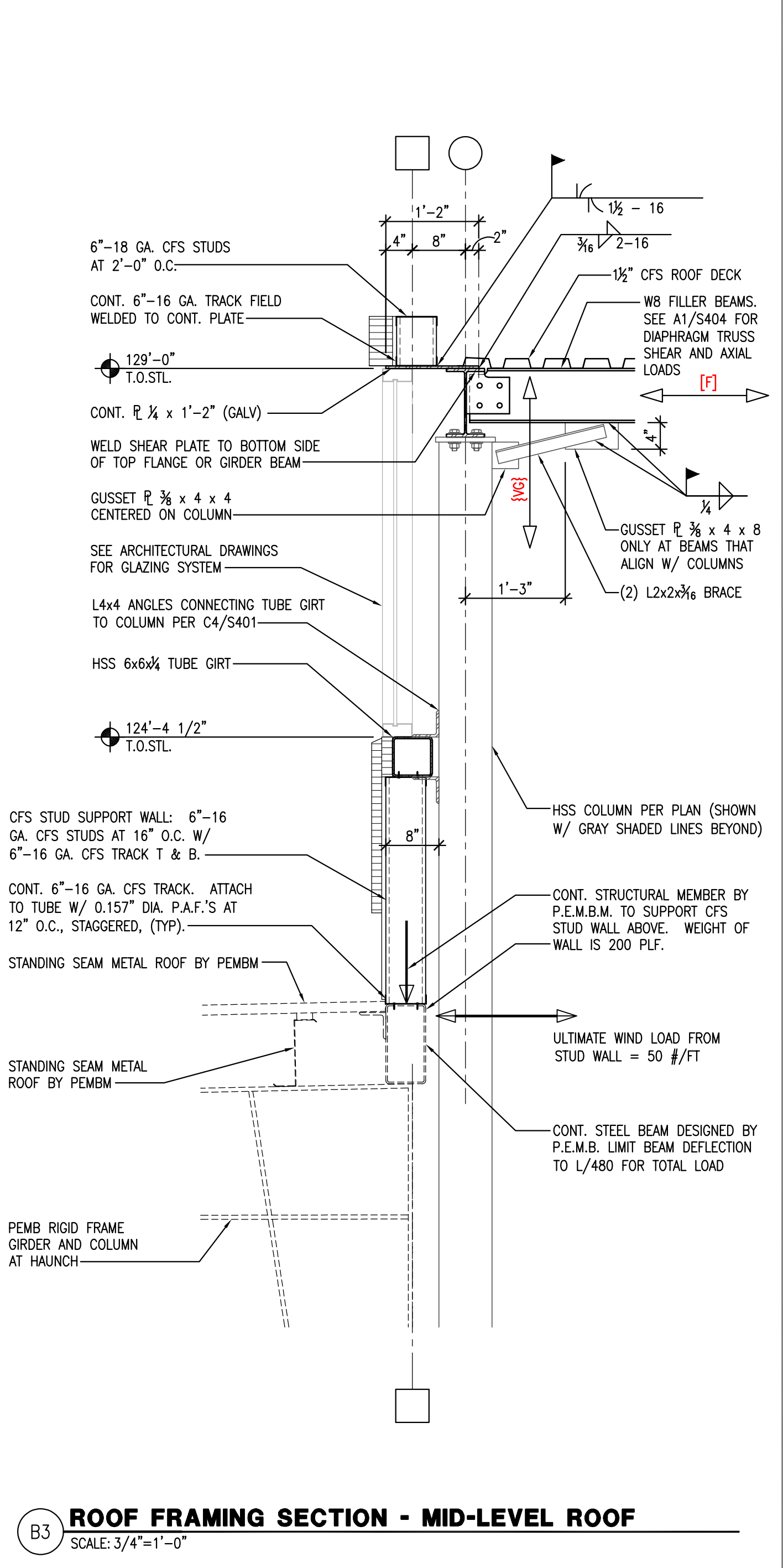
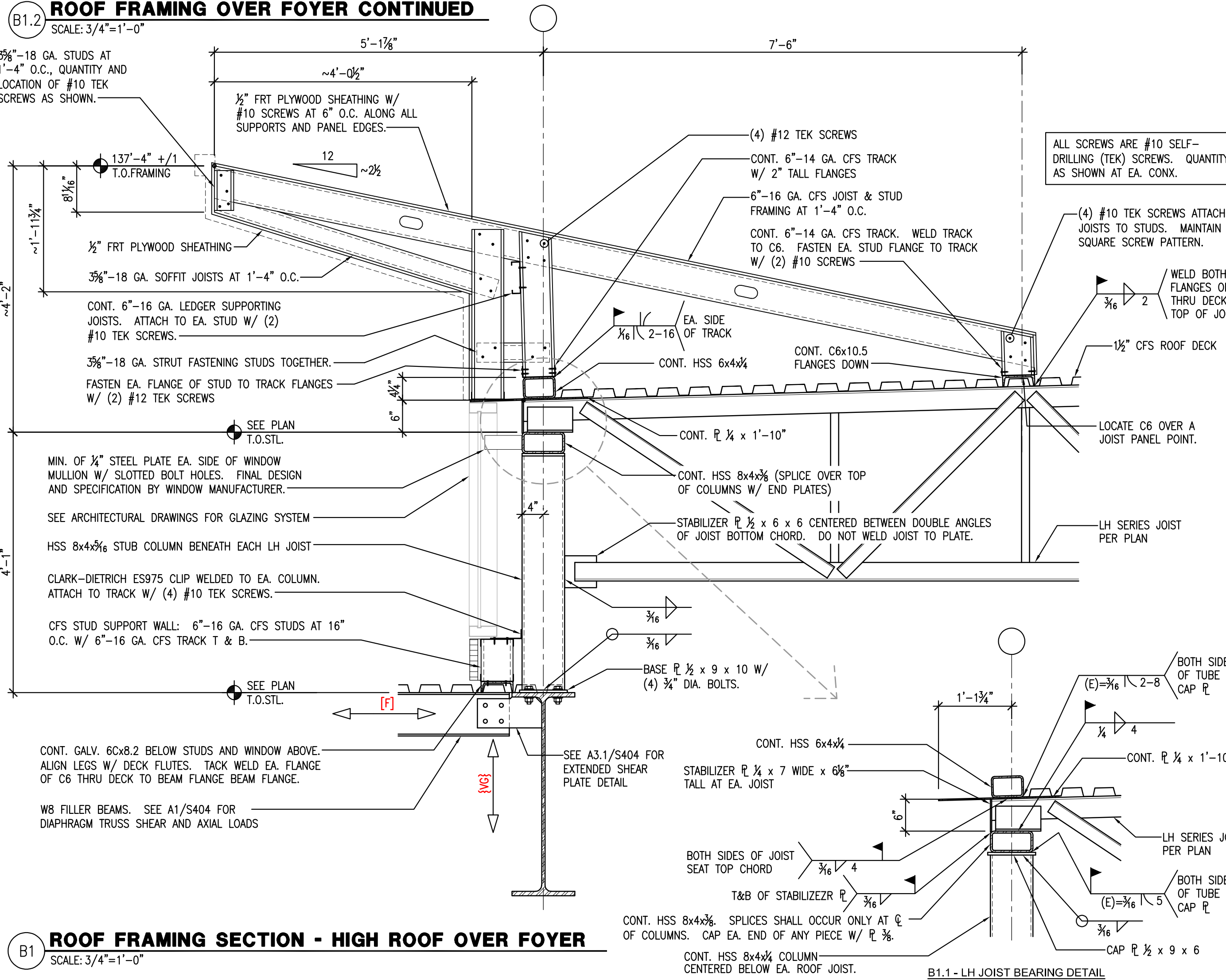
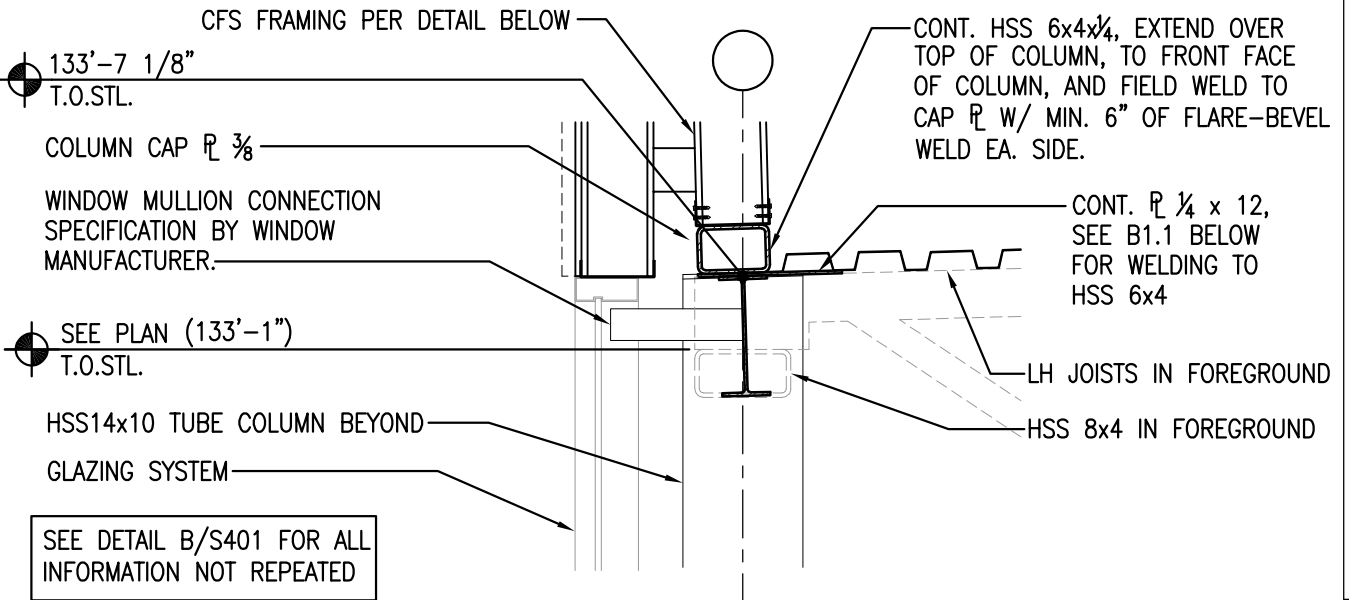
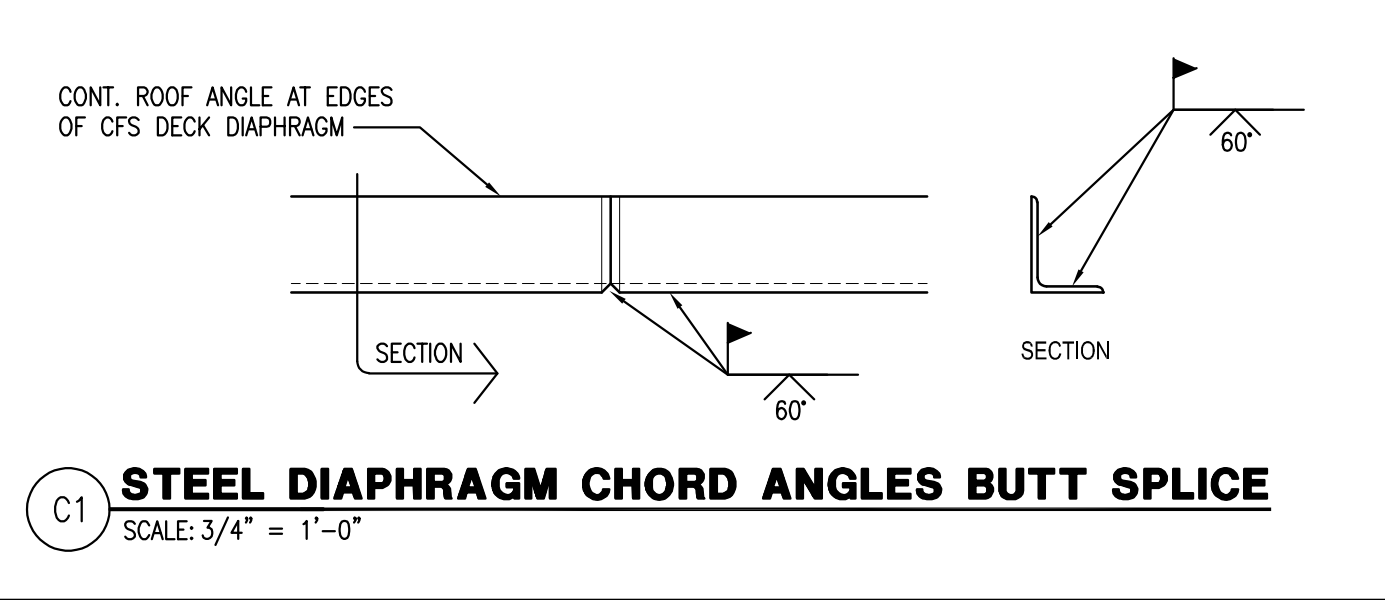
216118

Plot Date: 11/9/2023 2:06:05 AM s401-galilee baptist roof details.dwg Chris Sekol



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GALILEE BAPTIST CHURCH
 NEW BUILDING FOR:
 6300 WOODYARD ROAD
 UPPER MARLBORO, MD 20772



| DRAWING | DATE |
|---|-------------|
| <input checked="" type="checkbox"/> Bid Set | 17 NOV 2023 |

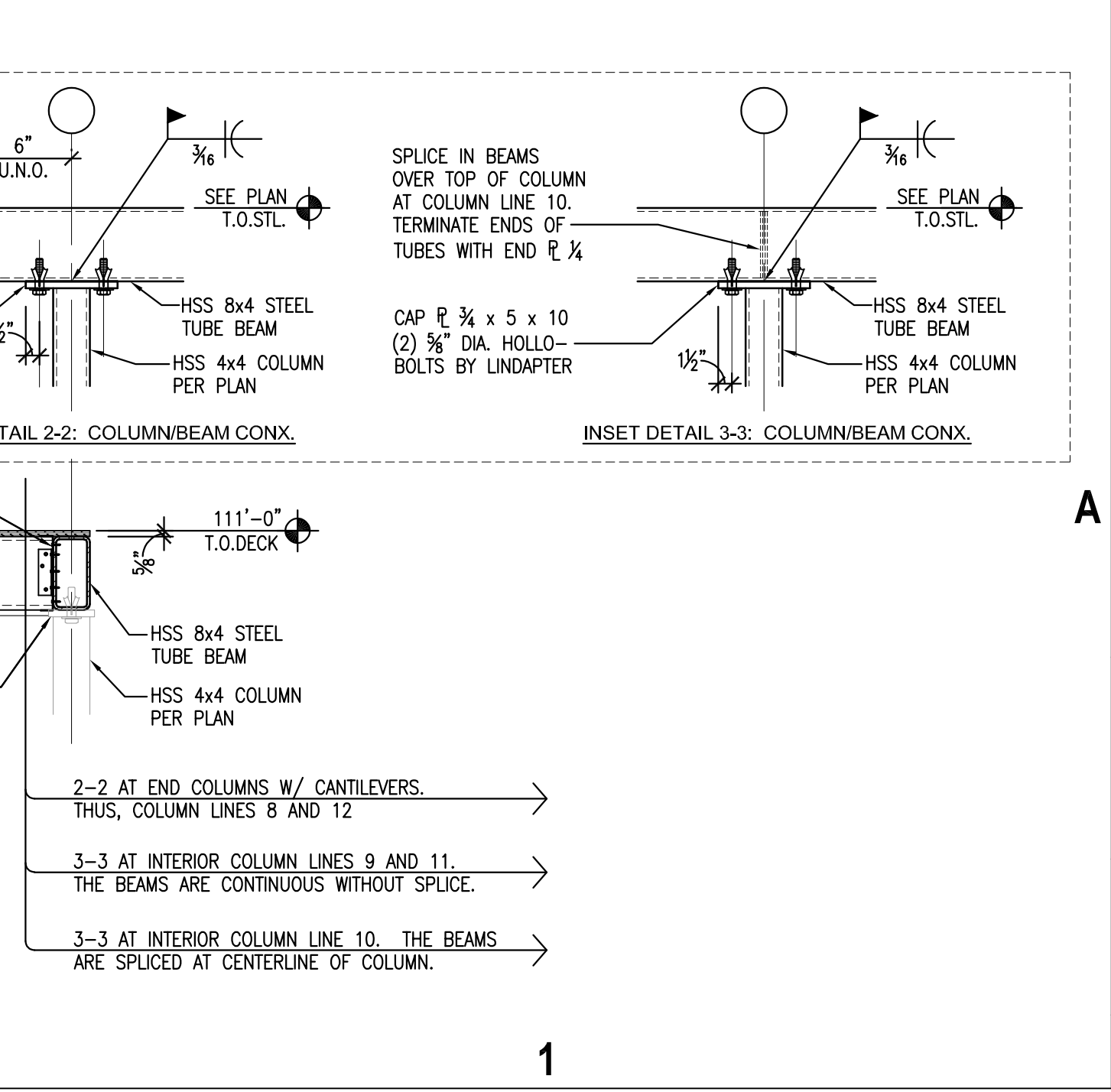
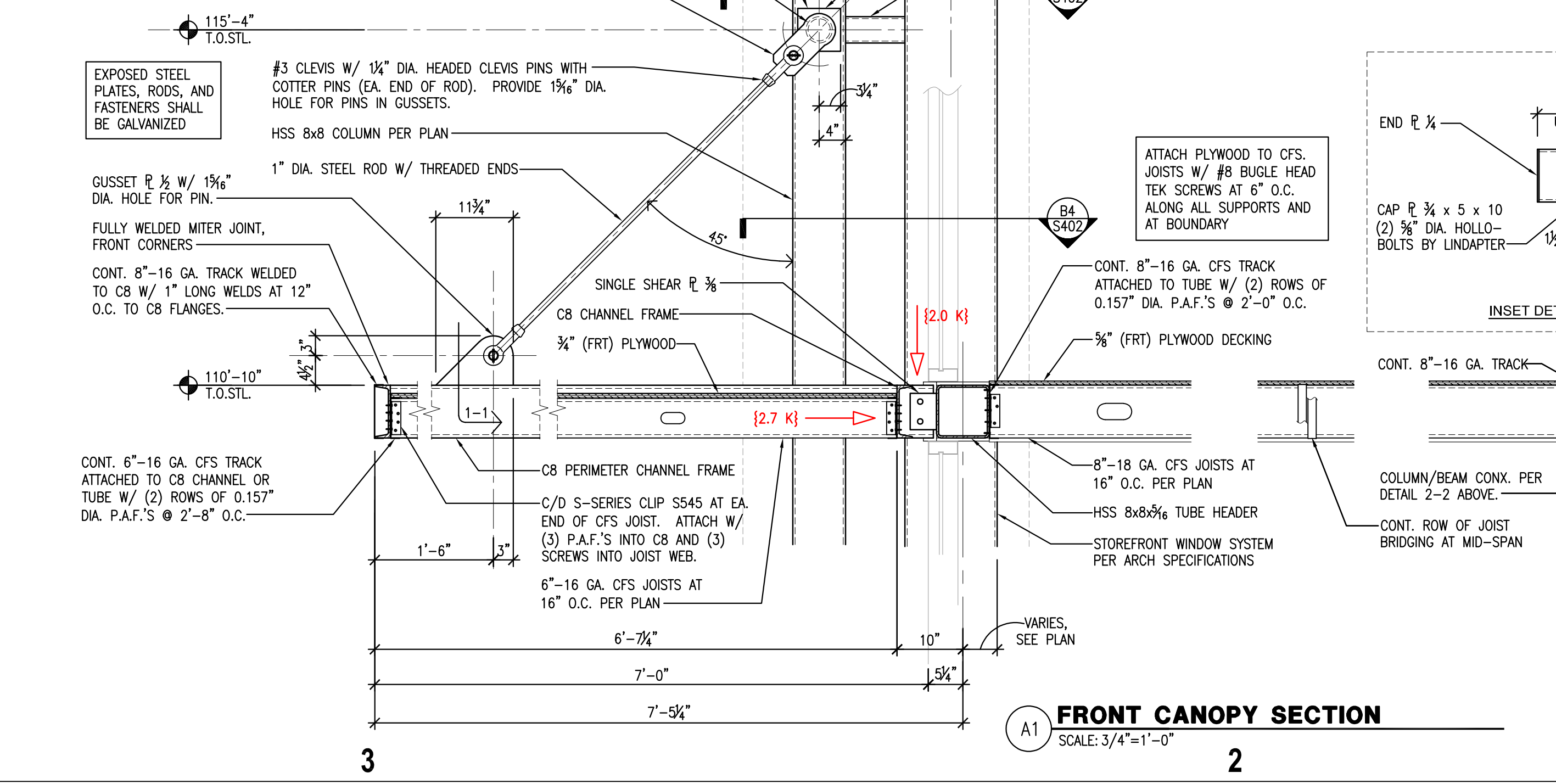
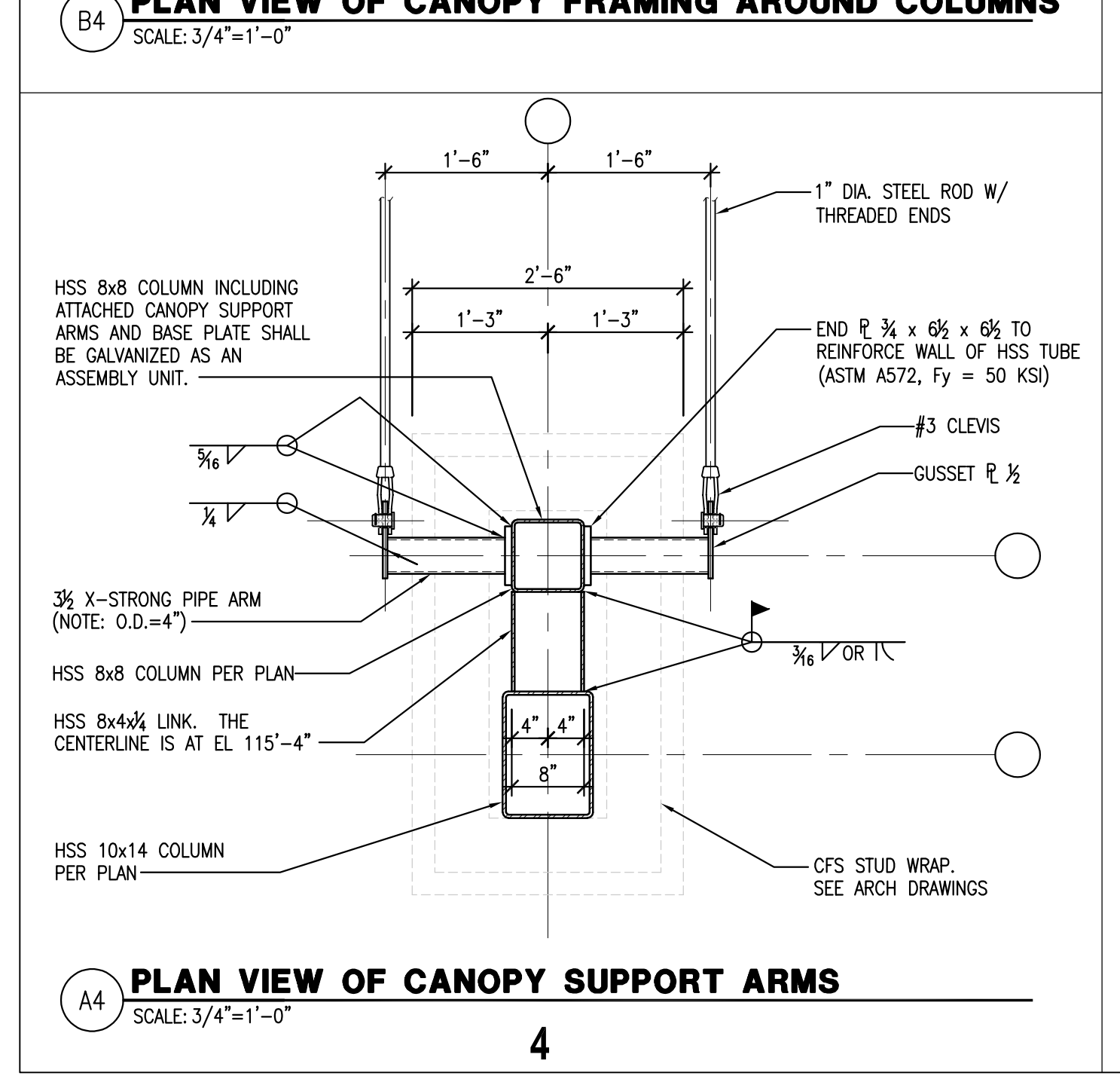
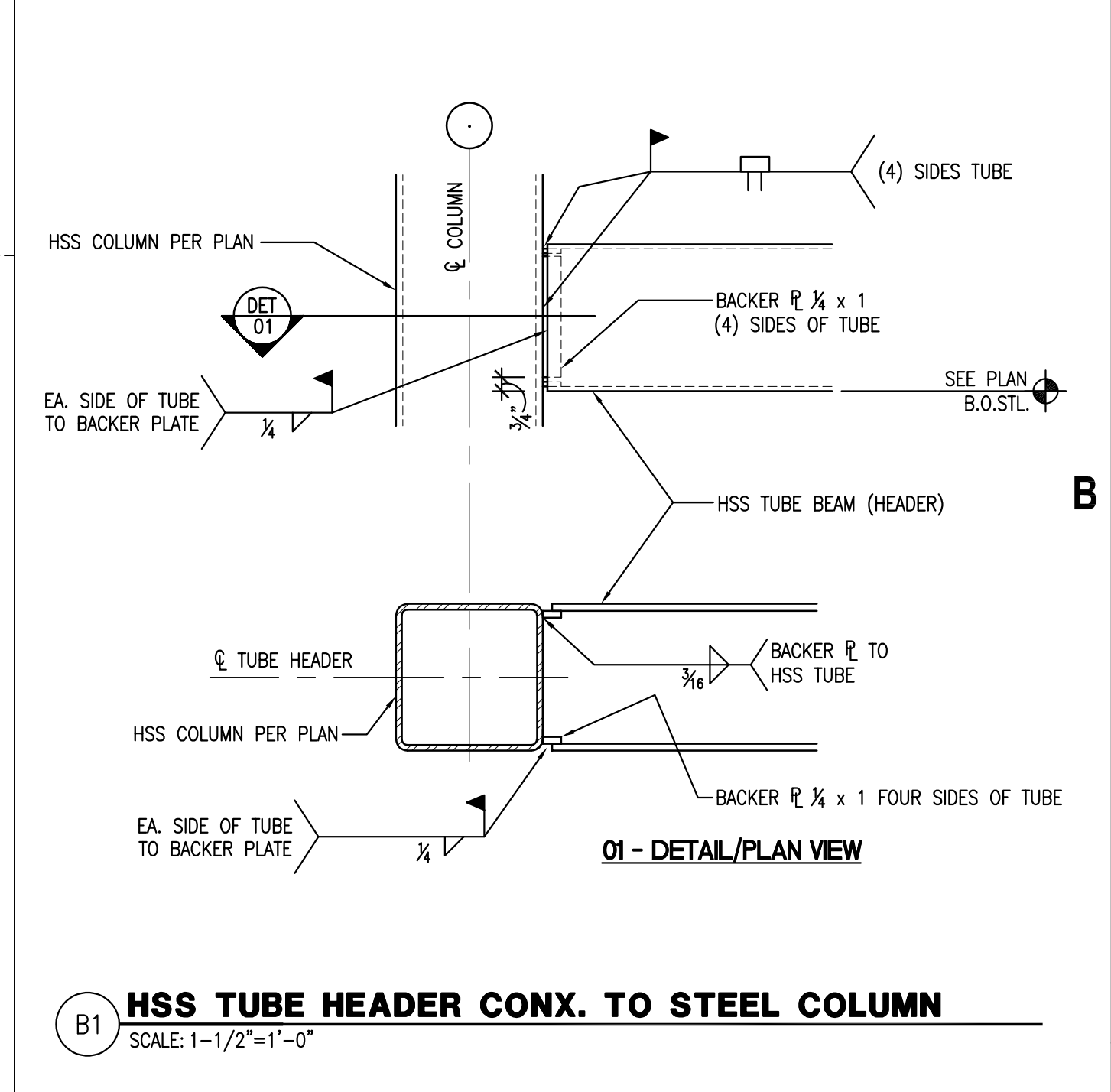
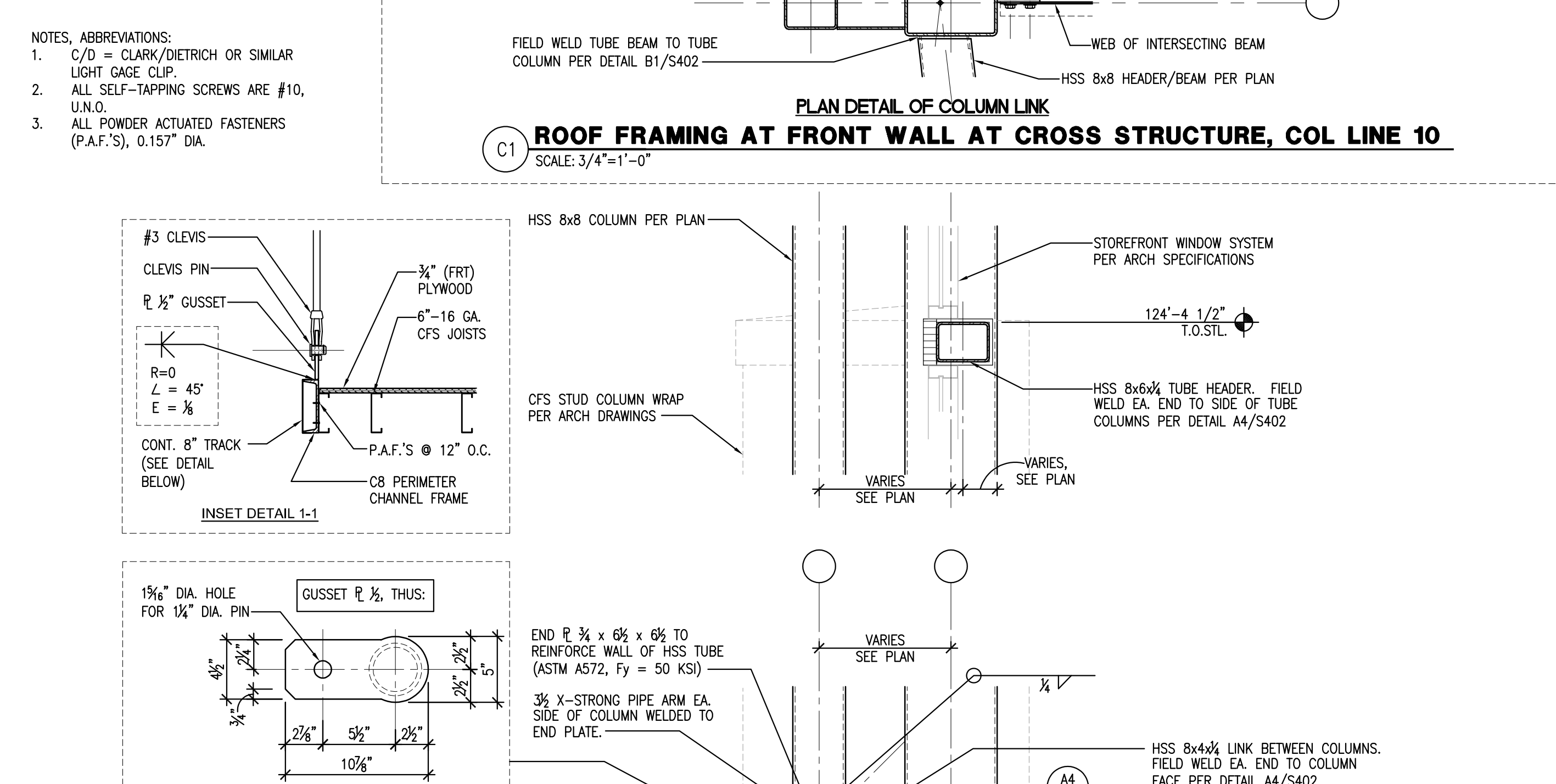
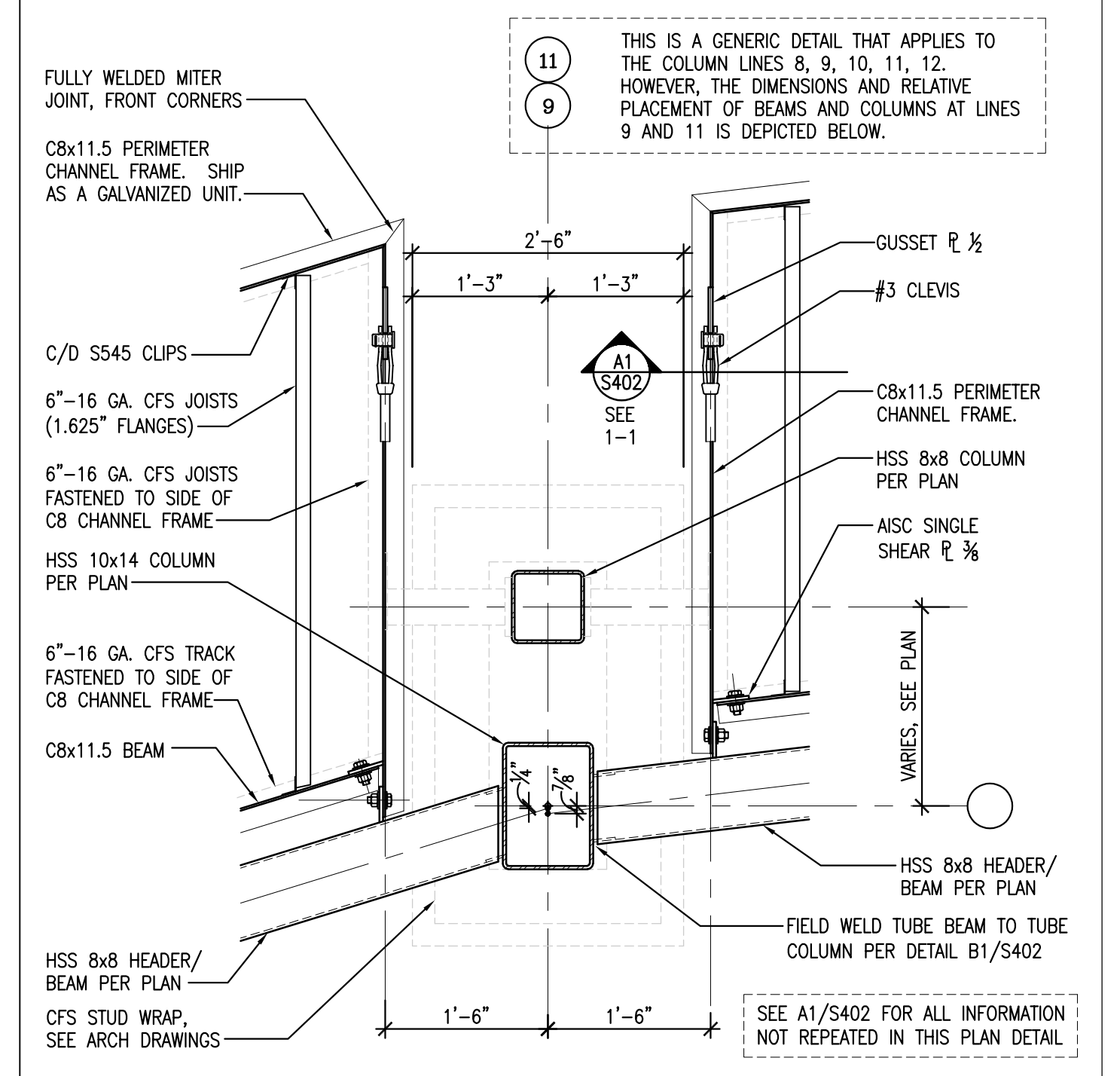
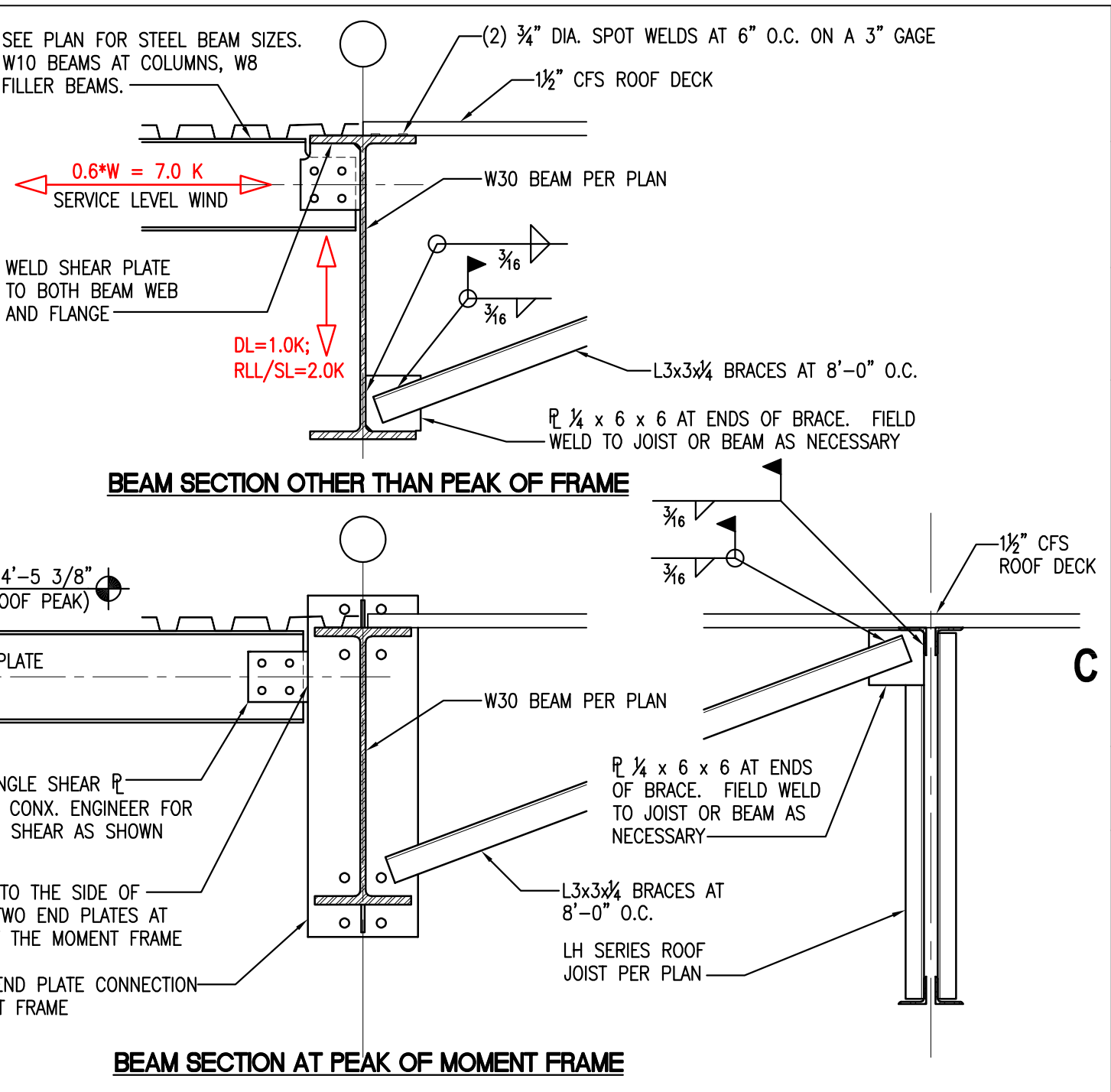
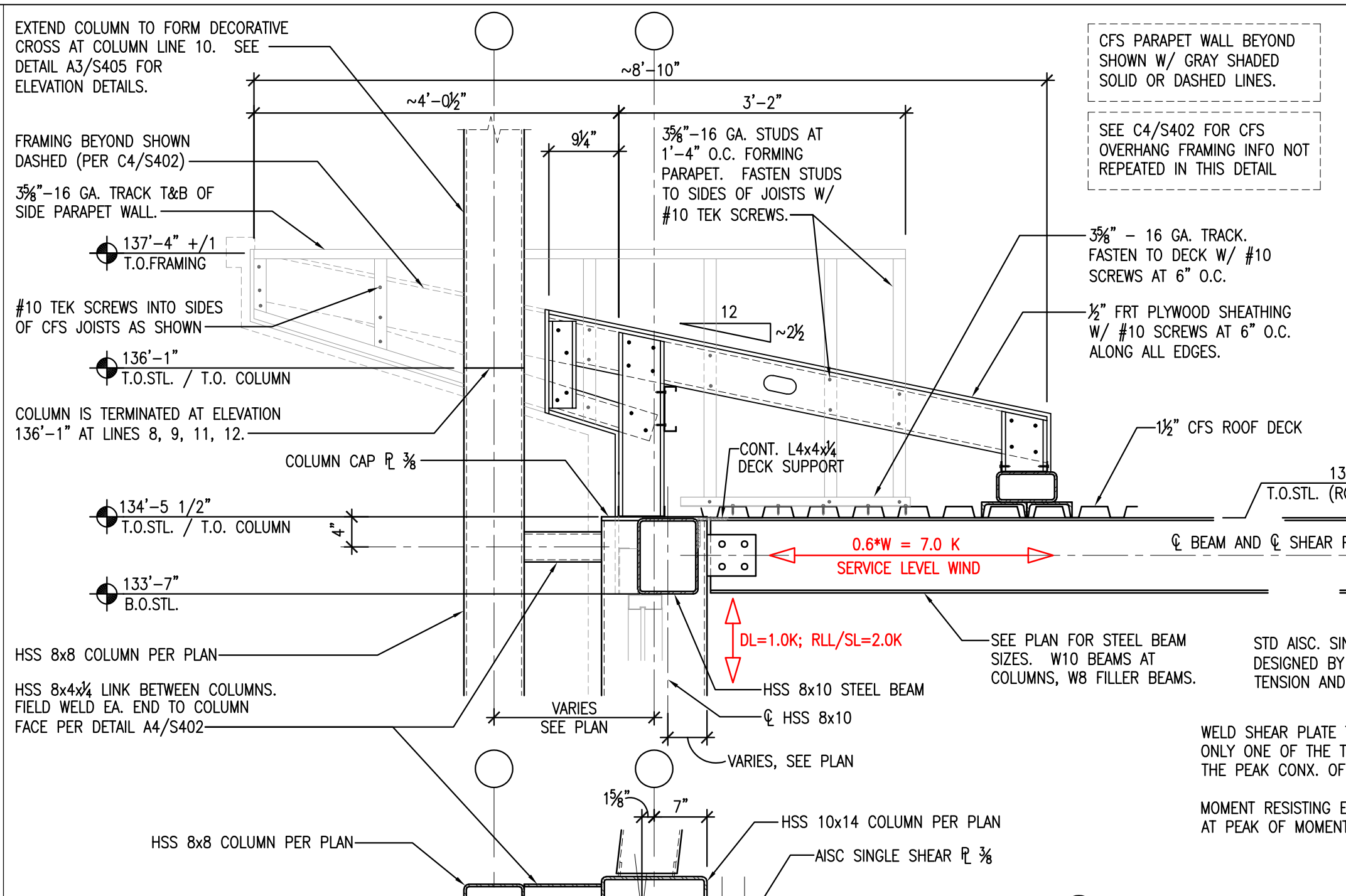
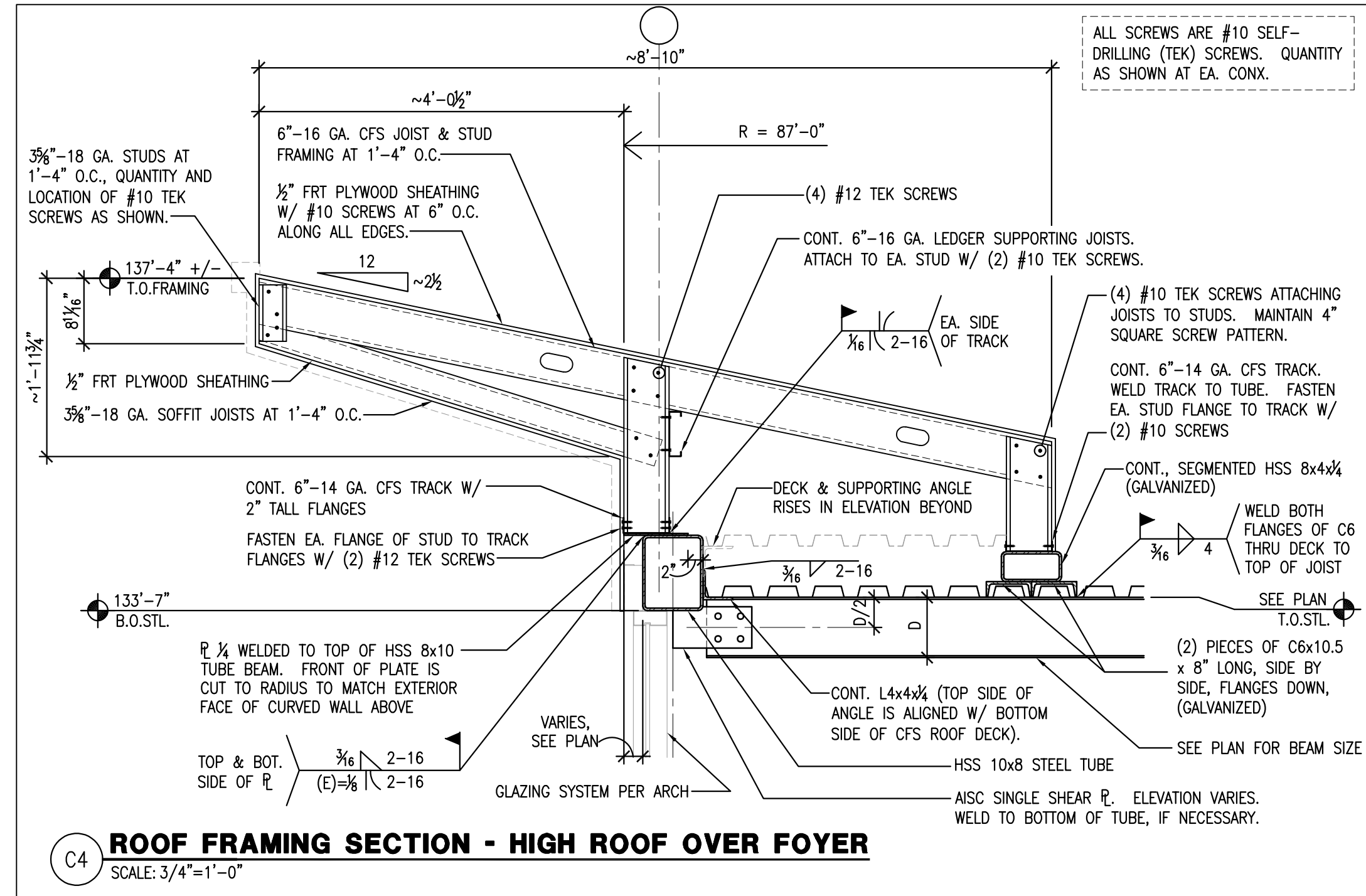
REVISIONS

FRAMING DETAILS
S401
 OF 18 SHEETS
216118

Plot Date: 11/9/2023 2:06:46 AM s401-galilee baptist roof details.dwg Chris Sekol

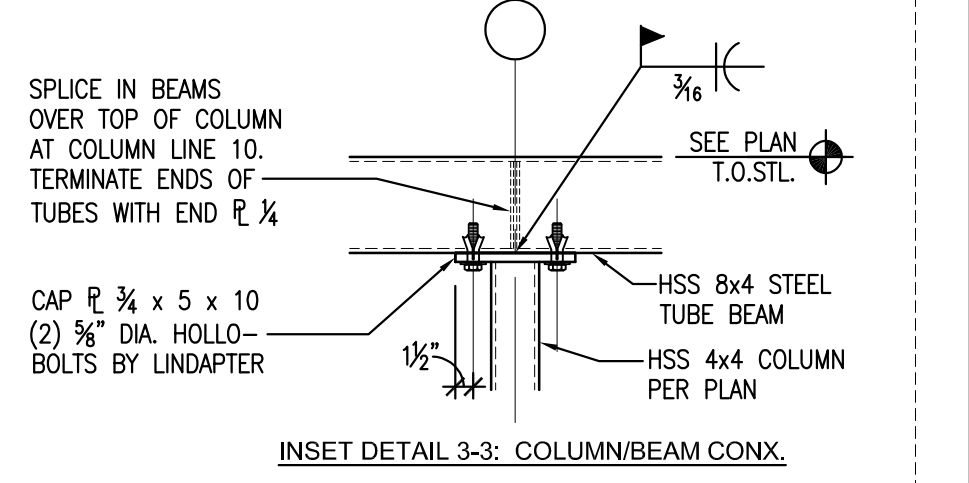
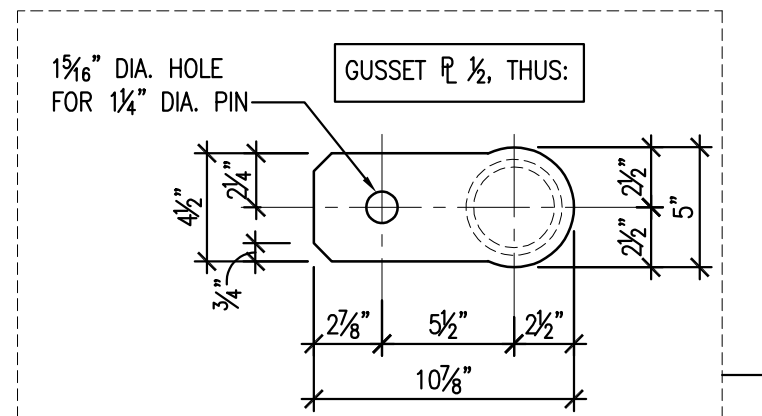
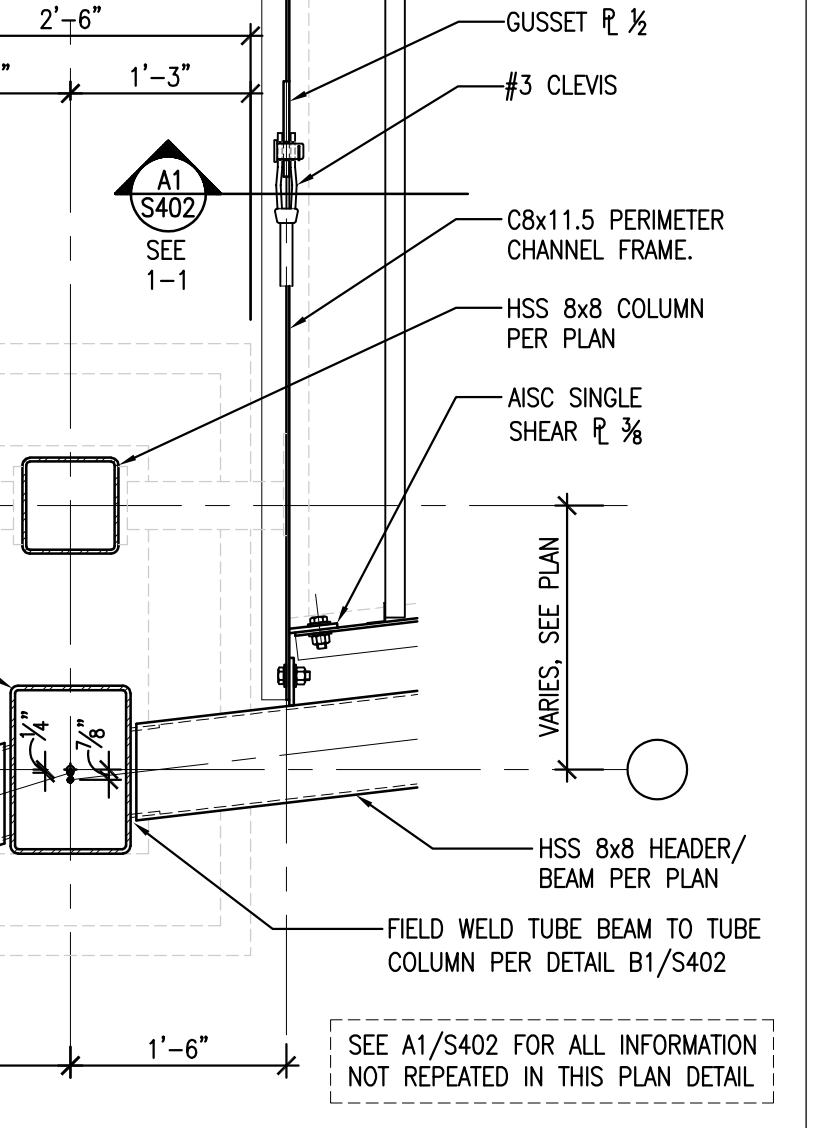


NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
6300 WOODYARD ROAD
UPPER MARLBORO, MD 20772



- NOTES, ABBREVIATIONS:
 1. C/D = CLARK/DIETRICH OR SIMILAR LIGHT GAGE CLIP.
 2. ALL SELF-TAPPING SCREWS ARE #10, U.N.O.
 3. ALL POWDER ACTUATED FASTENERS (P.A.F.'S), 0.157" DIA.

THIS IS A GENERIC DETAIL THAT APPLIES TO THE COLUMN LINES 8, 9, 10, 11, 12. HOWEVER, THE DIMENSIONS AND RELATIVE PLACEMENT OF BEAMS AND COLUMNS AT LINES 9 AND 11 IS DEPICTED BELOW.



| DRAWING | DATE |
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| <input checked="" type="checkbox"/> Bid Set | 17 NOV 2023 |

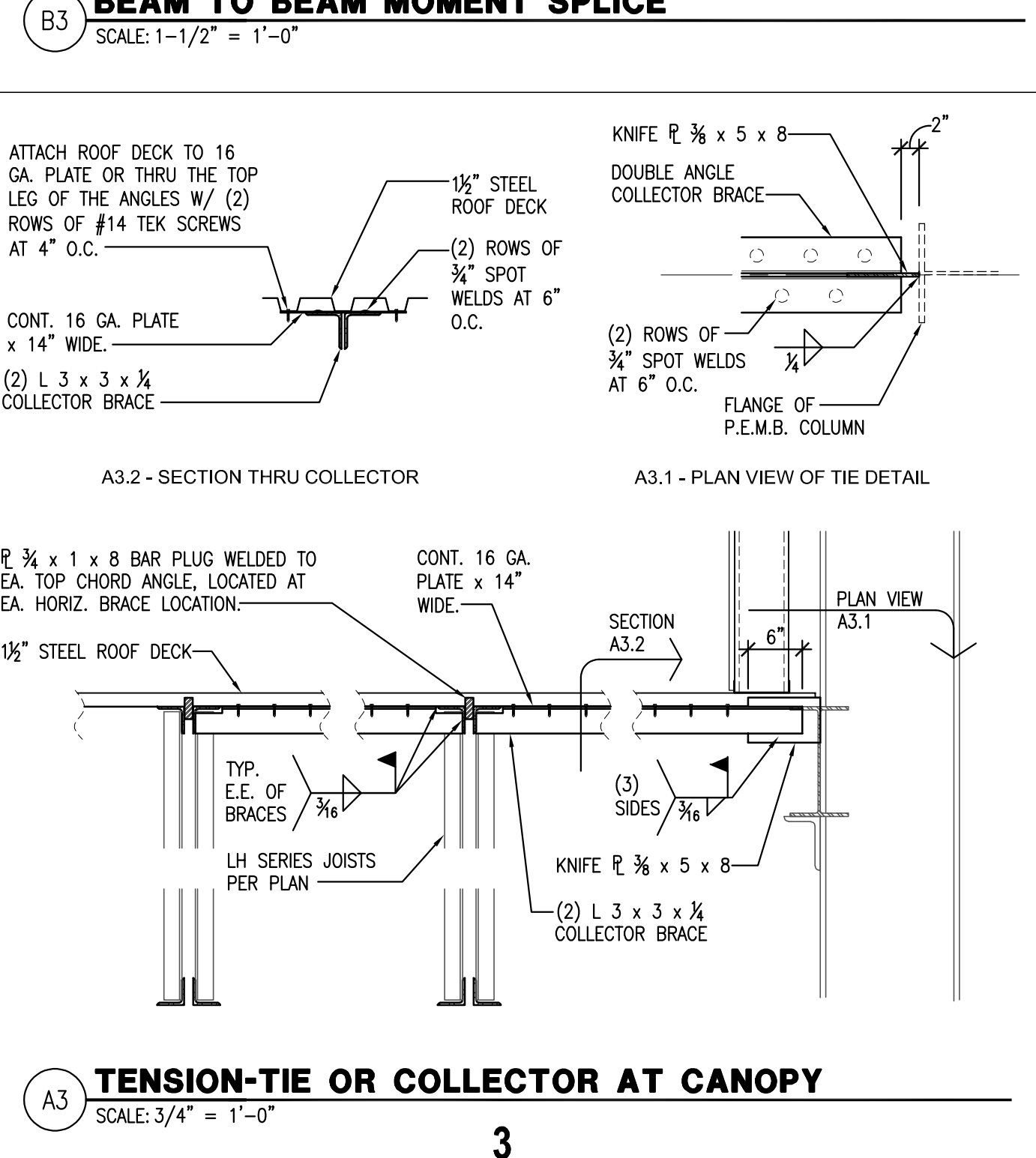
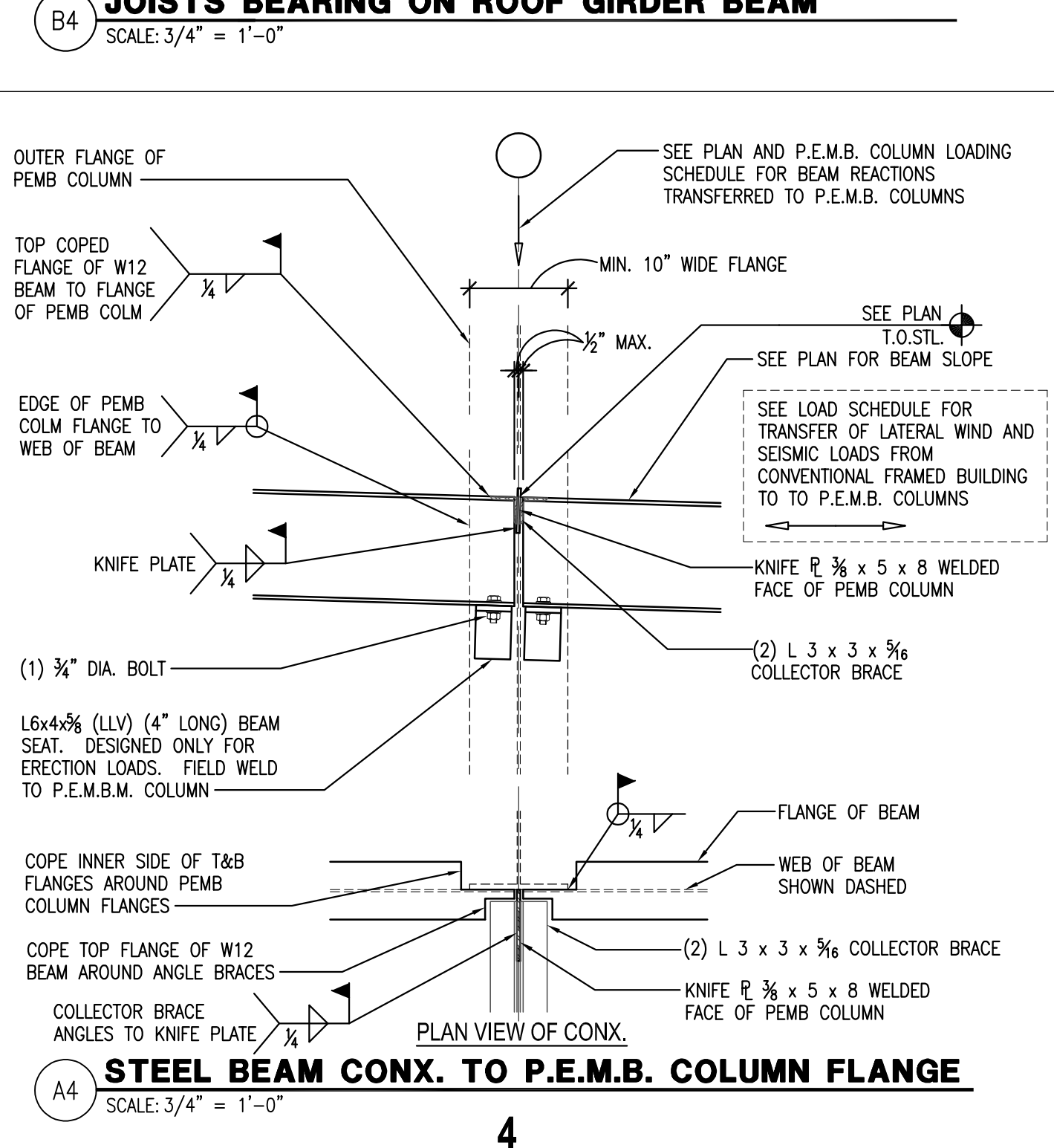
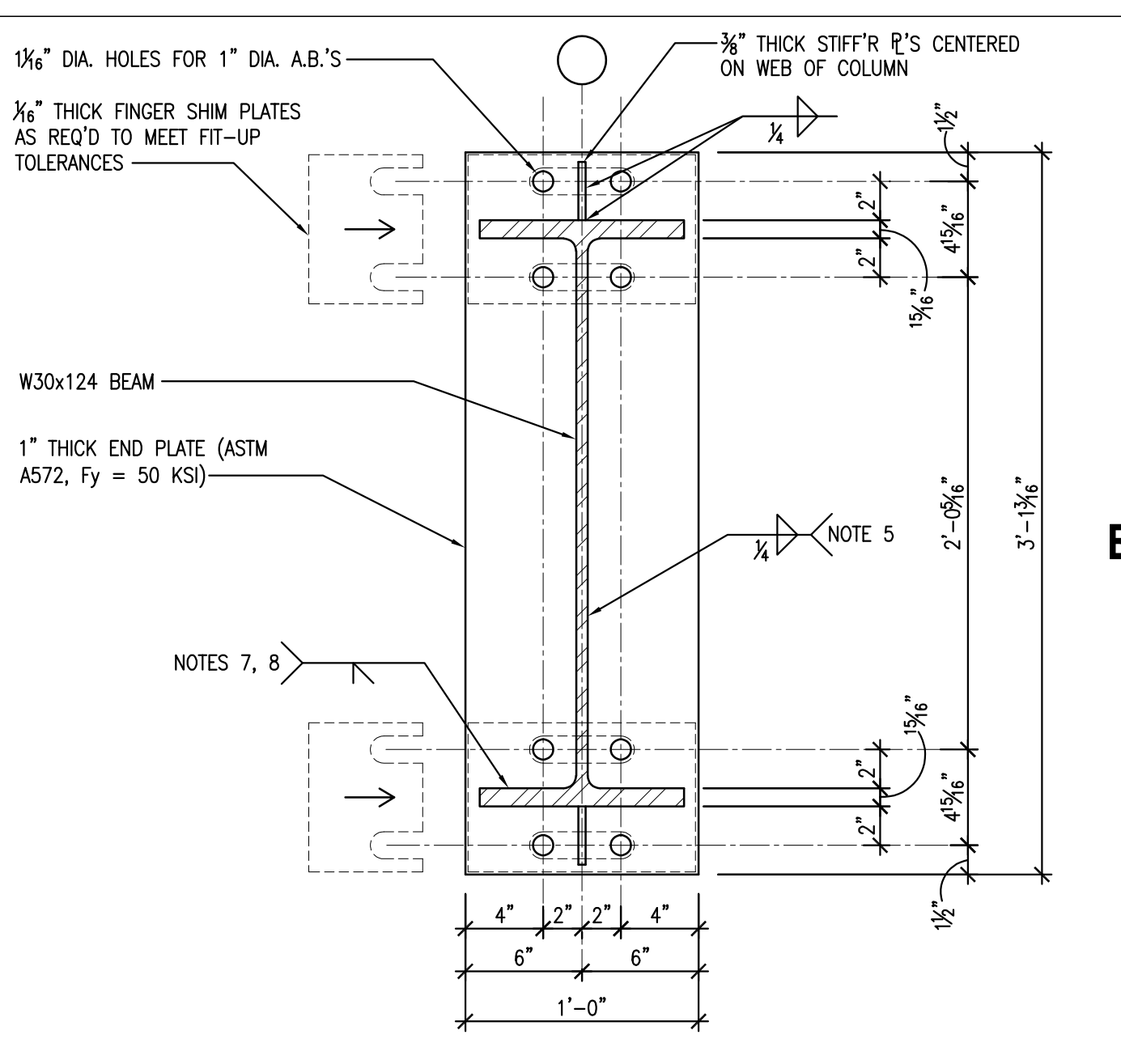
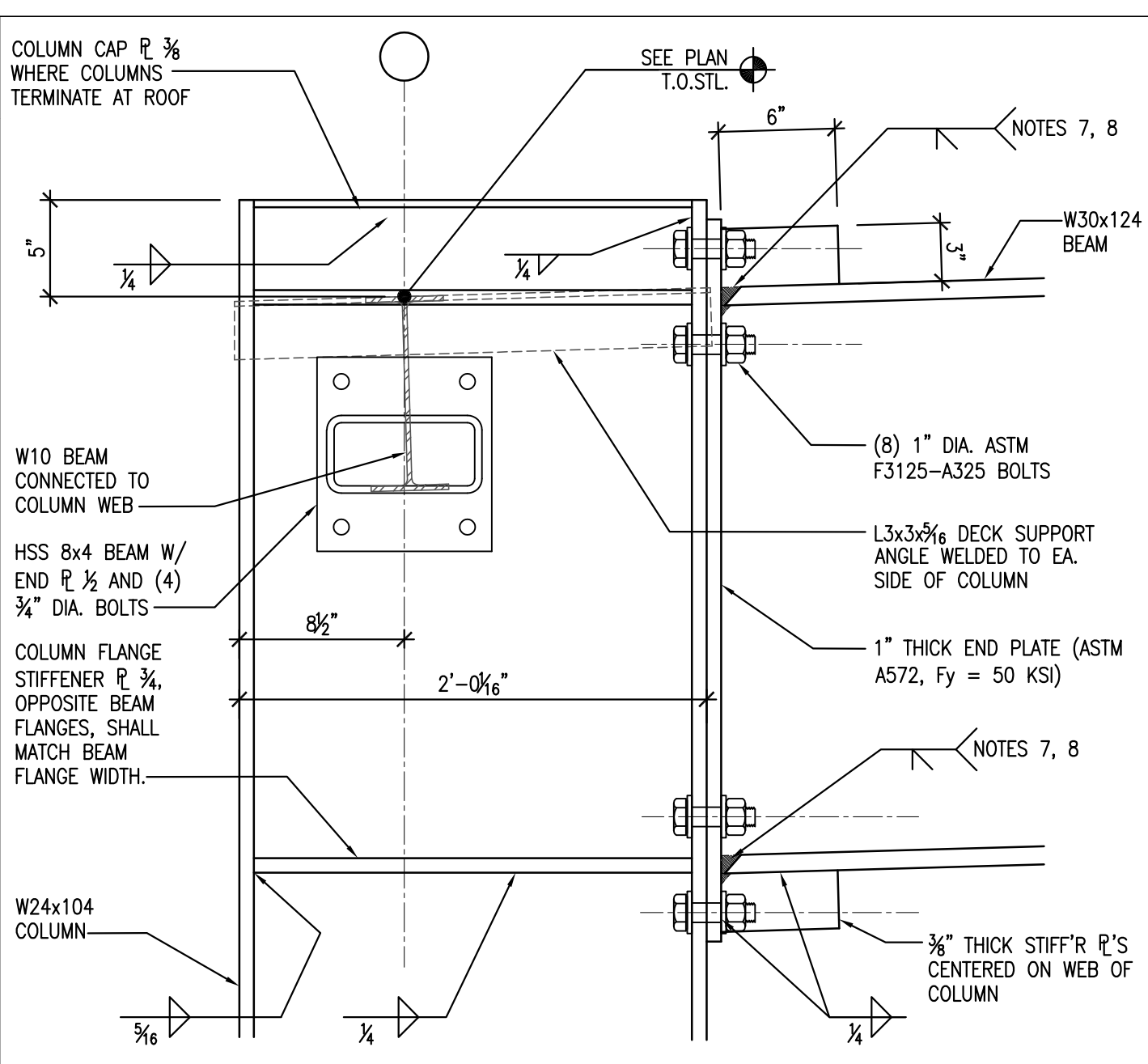
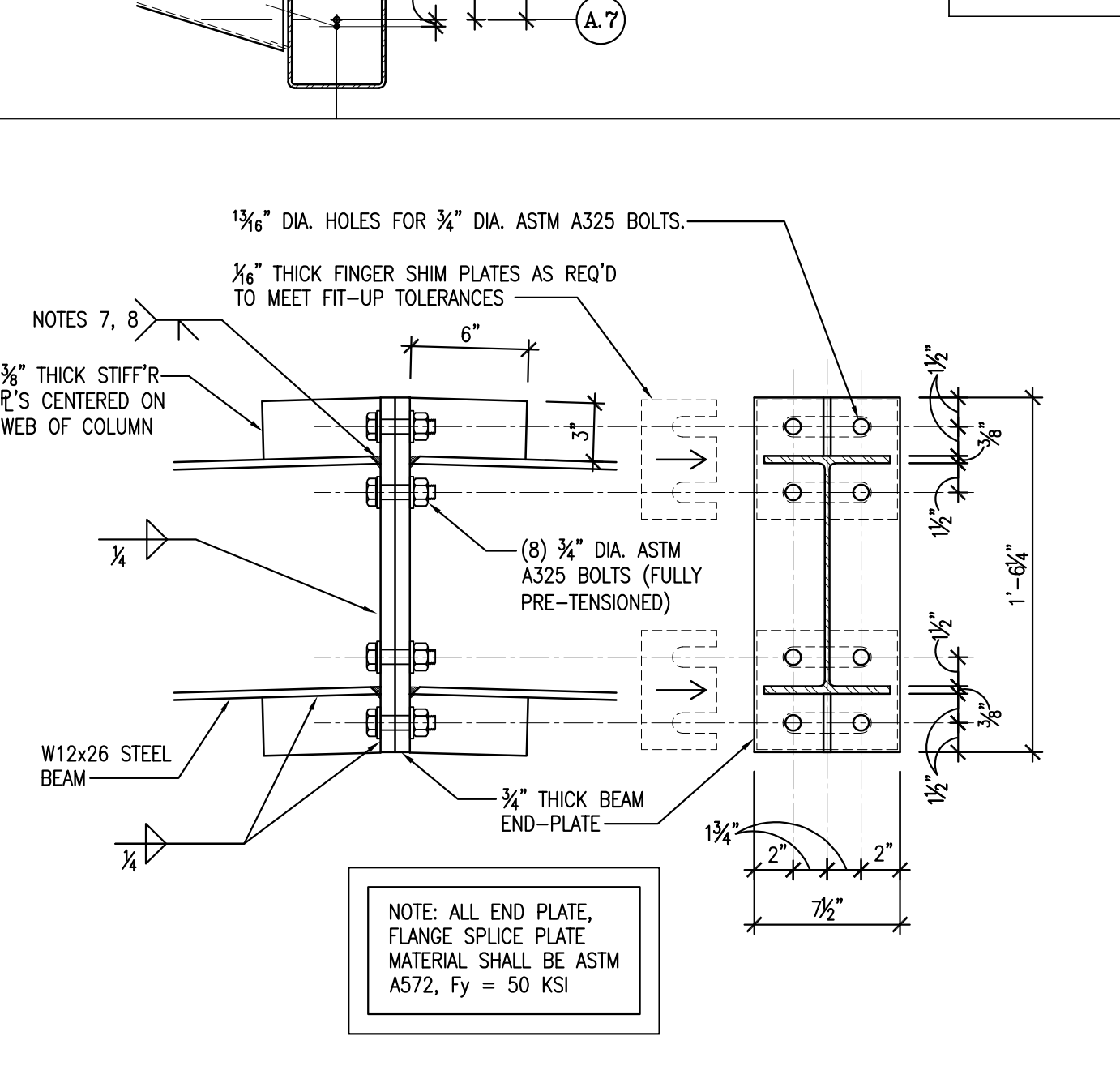
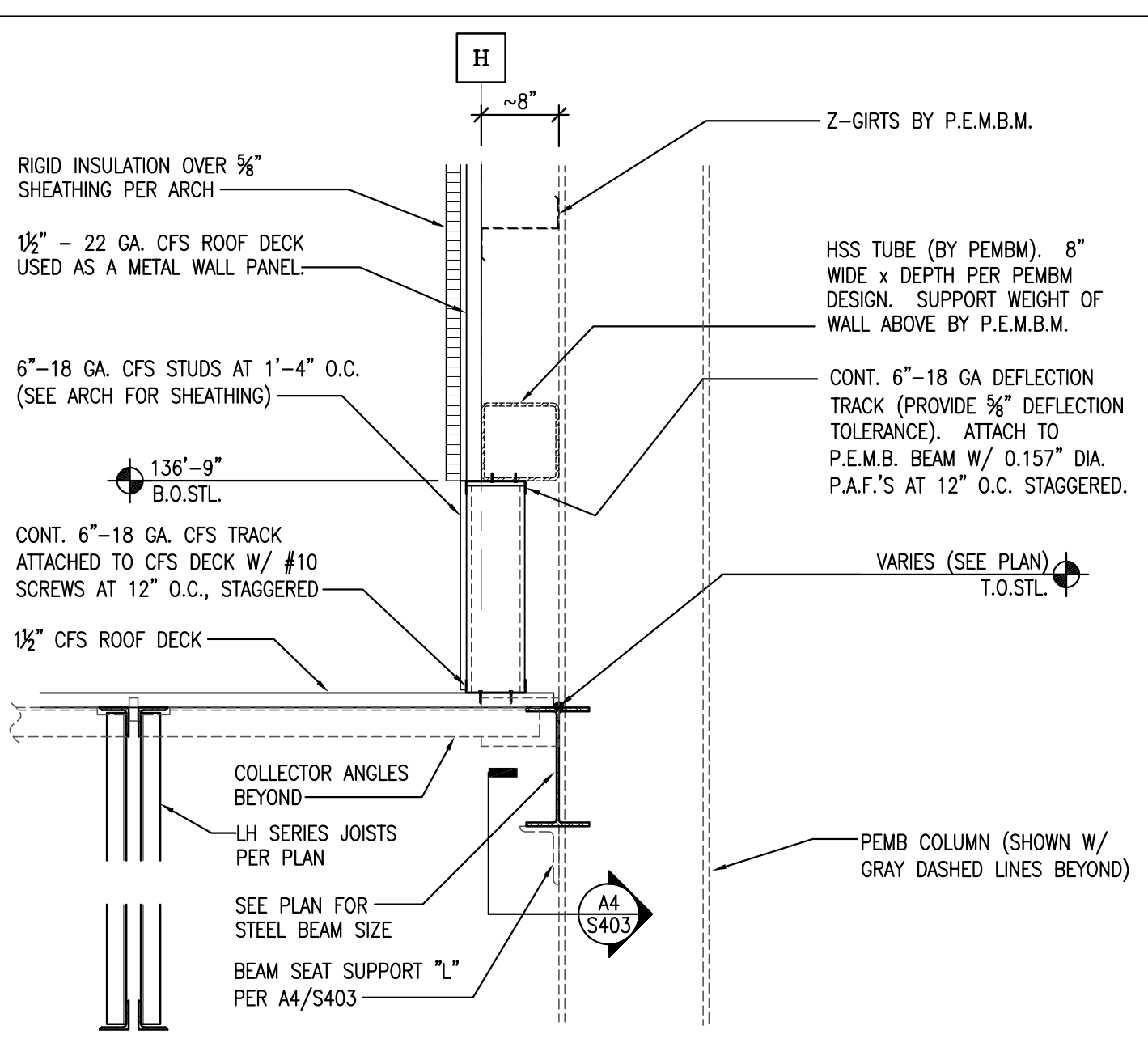
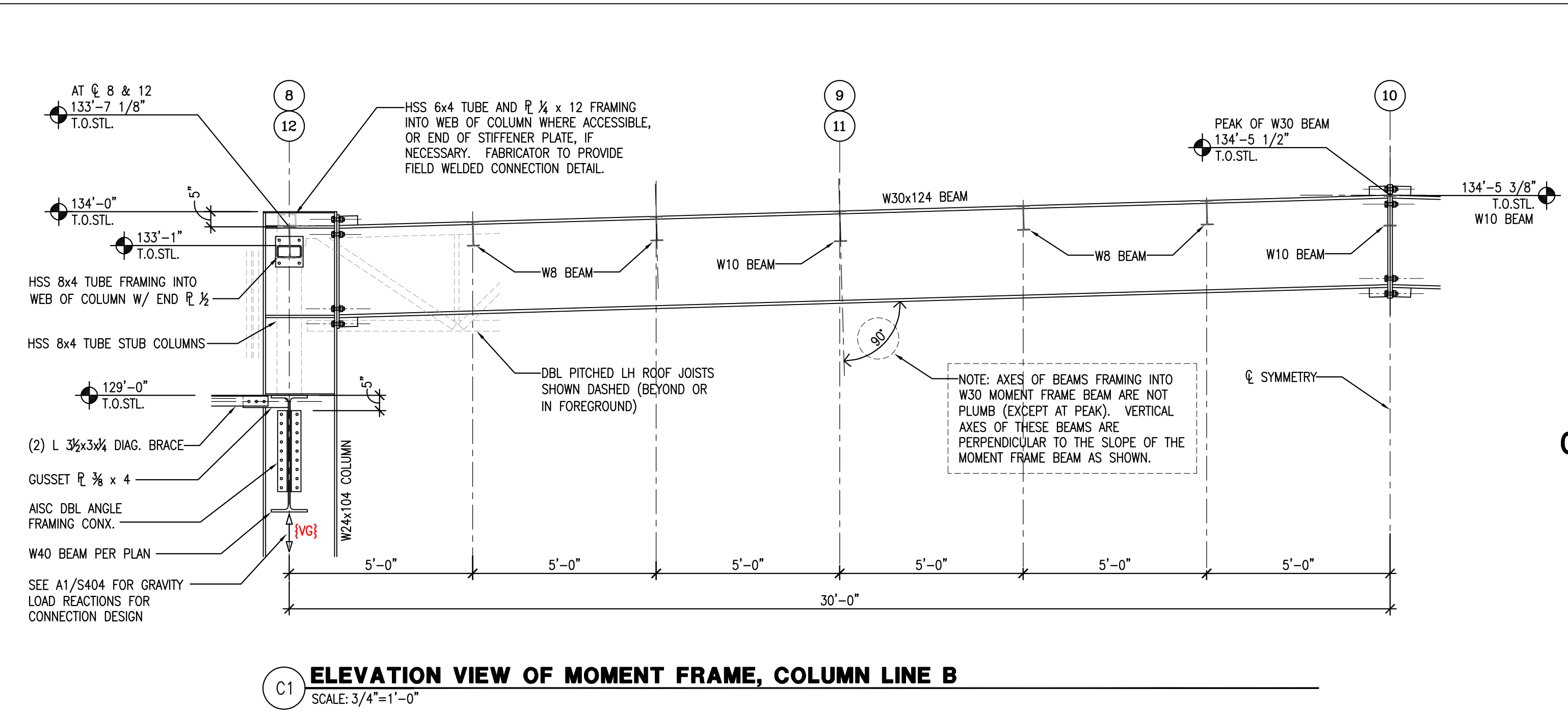
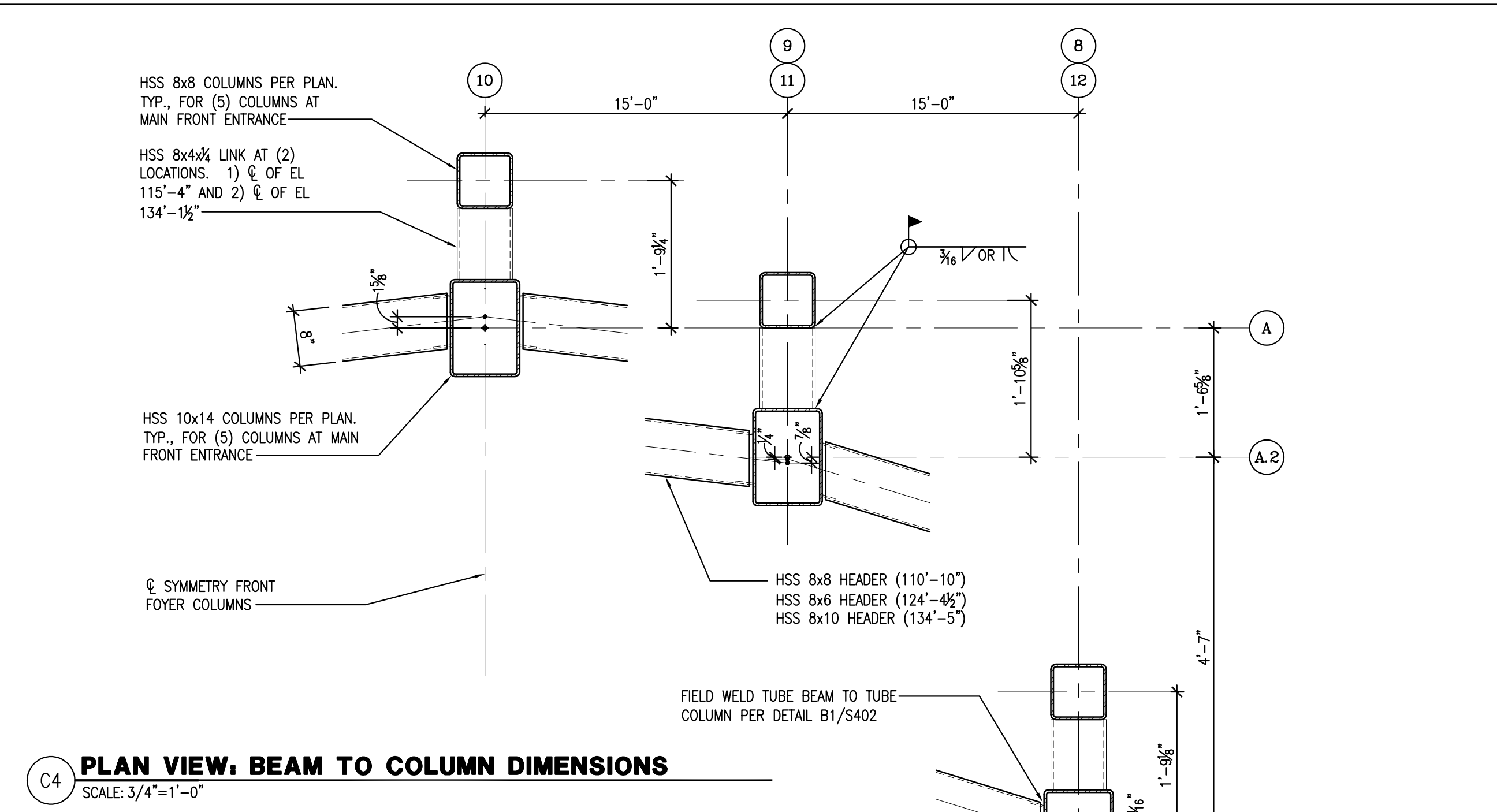
REVISIONS



LICENSE EXPIRES: 06-21-2025
 DRAWING ISSUED: 11-17-2023

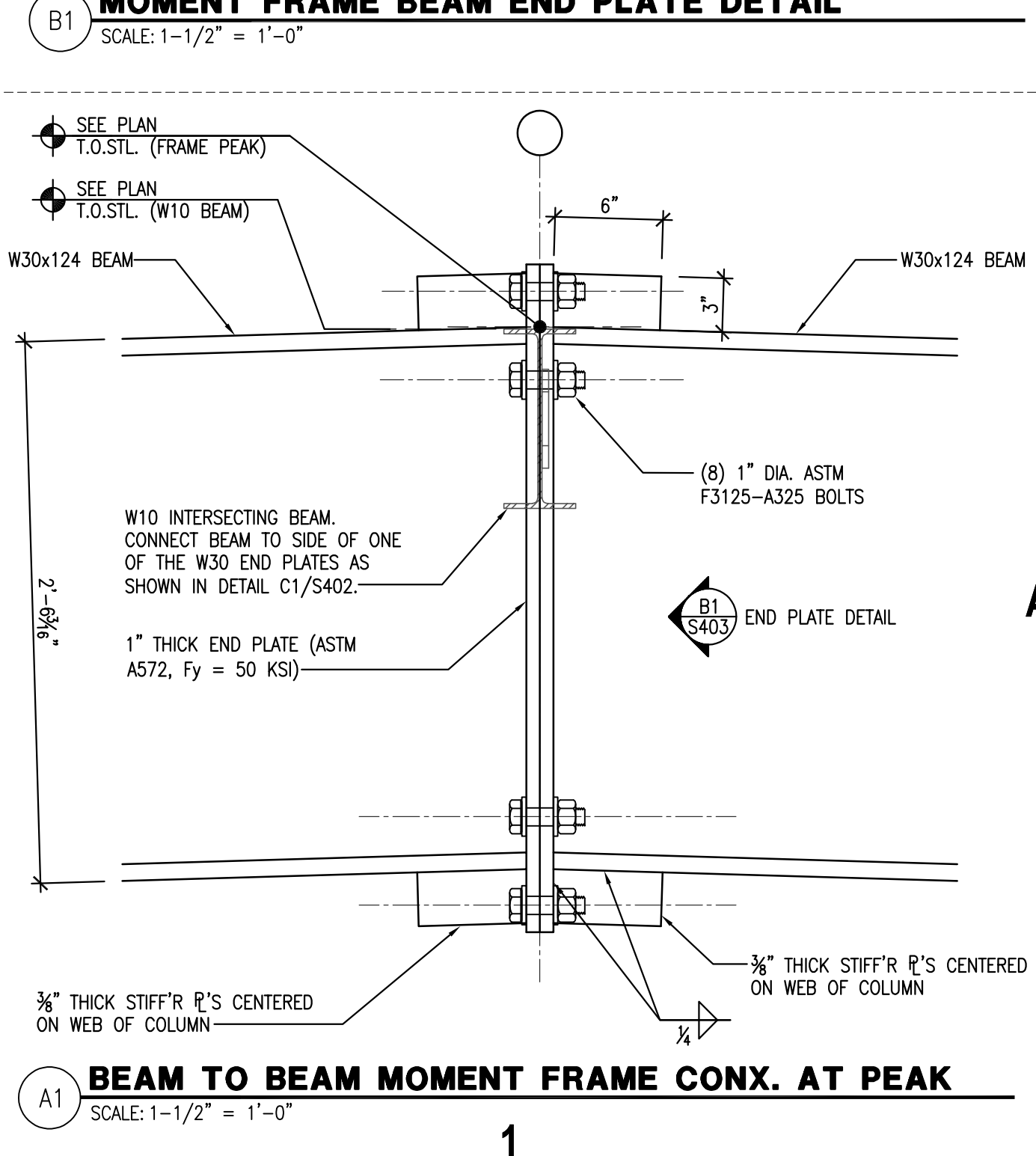
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NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
 6300 WOODYARD ROAD
 UPPER MARLBORO, MD 20772



NOTES ON WELDING REQUIREMENTS

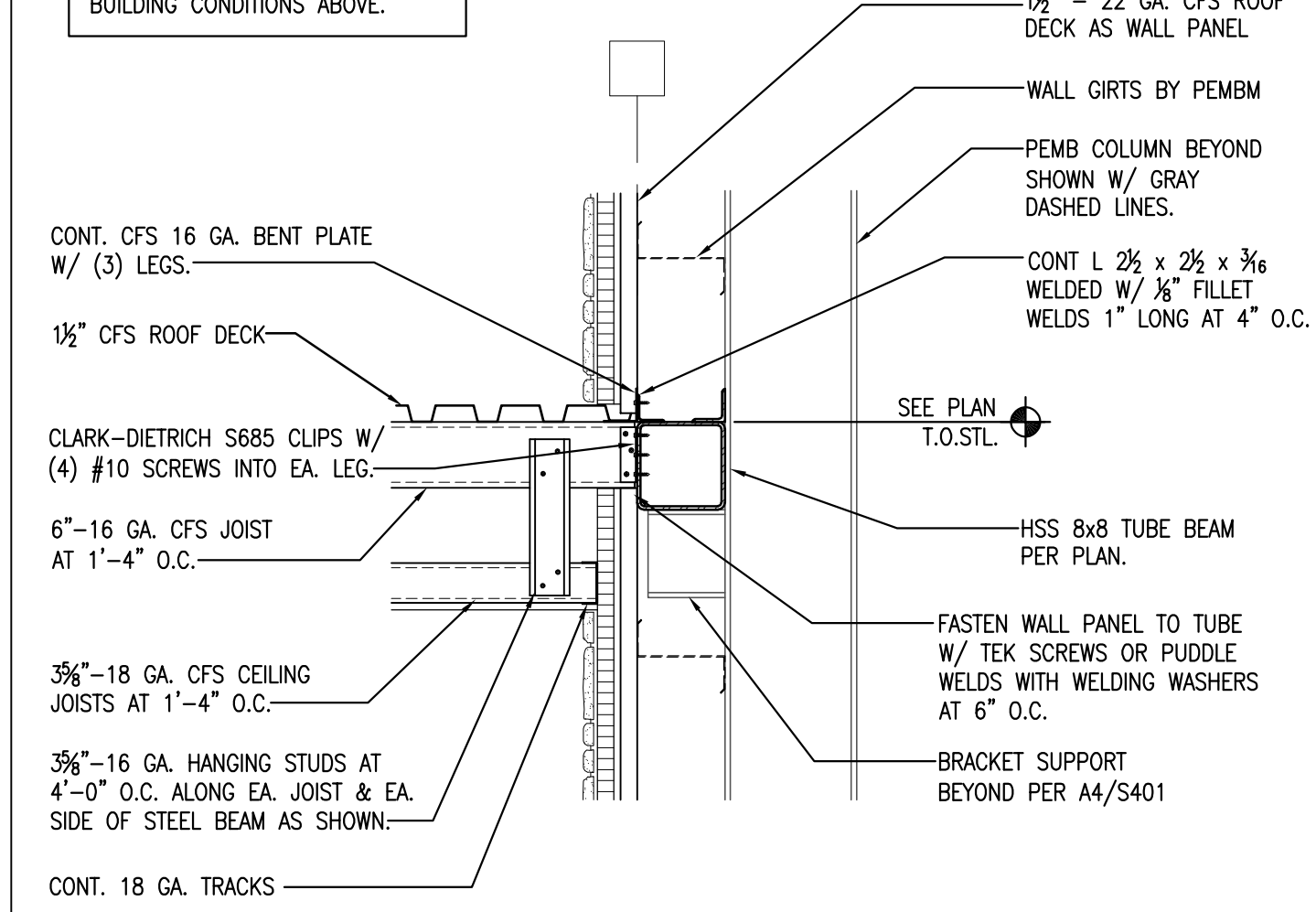
- PREPARE THE BEAM FLANGES WITH A 45° FULL-DEPTH BEVEL.
- FIT-UP THE END-PLATE AND BEAM WITH A MINIMUM ROOT OPENING.
- PRE-HEAT THE SPECIMENS AS REQUIRED BY AWS SPECIFICATIONS.
- PREPARE SURFACES FOR WELDING AS REQUIRED BY AWS SPECIFICATIONS.
- PLACE THE WEB FILLET WELDS FIRST.
- PLACE THE 5/16" BACKING FILLET WELDS (BACKING IS FOR THE FULL PENETRATION FLANGE WELDS TO THE END-PLATE) ON THE WEB SIDE OF THE BEAM FLANGES. PLACE THESE WELDS AFTER THE WEB HAS BEEN WELDED.
- BEFORE PLACING THE FULL PENETRATION FLANGE WELDS, BACKGROUZE THE ROOT OF THE BEVEL TO REMOVE ANY CONTAMINANTS FROM THE 5/16" BACKER FILLET WELDS.
- PLACE THE FULL PENETRATION FLANGE WELDS LAST.



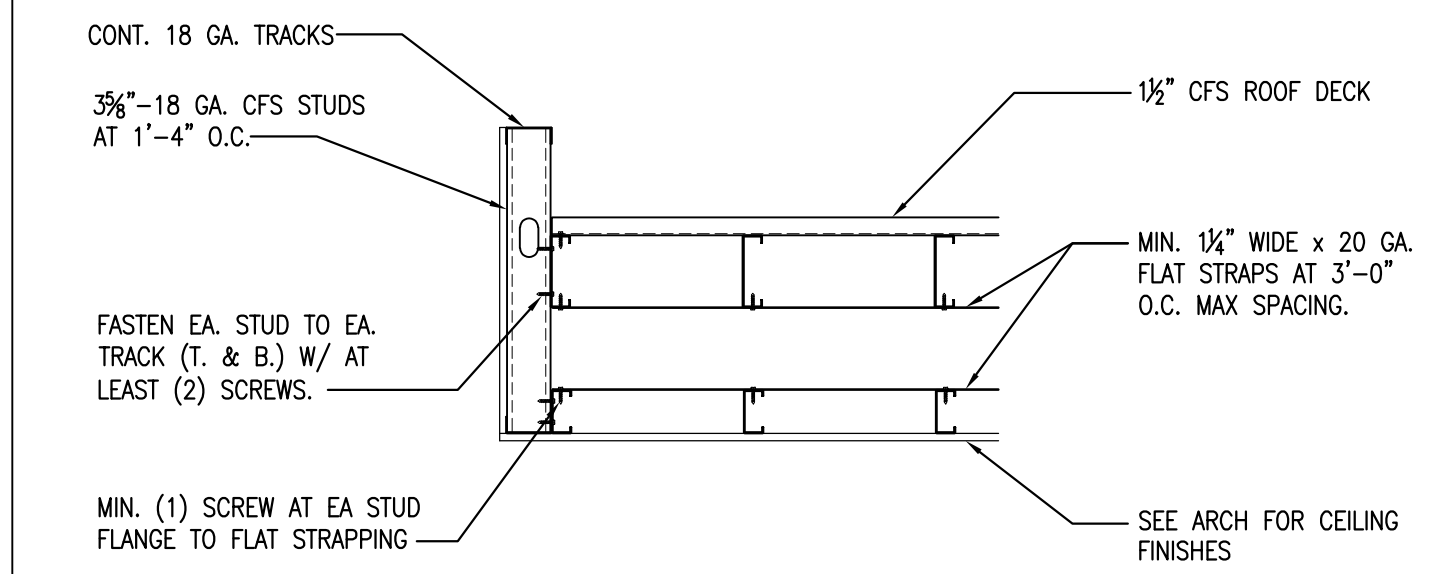
Plot Date: 11/9/2023 2:09:07 AM s401-galilee baptist roof details.dwg Chris Sekol

| DRAWING | DATE |
|---|-------------|
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| REVISIONS | |
| FRAMING DETAILS | |
| S403 | |
| OF 18 SHEETS | |
| 216118 | |

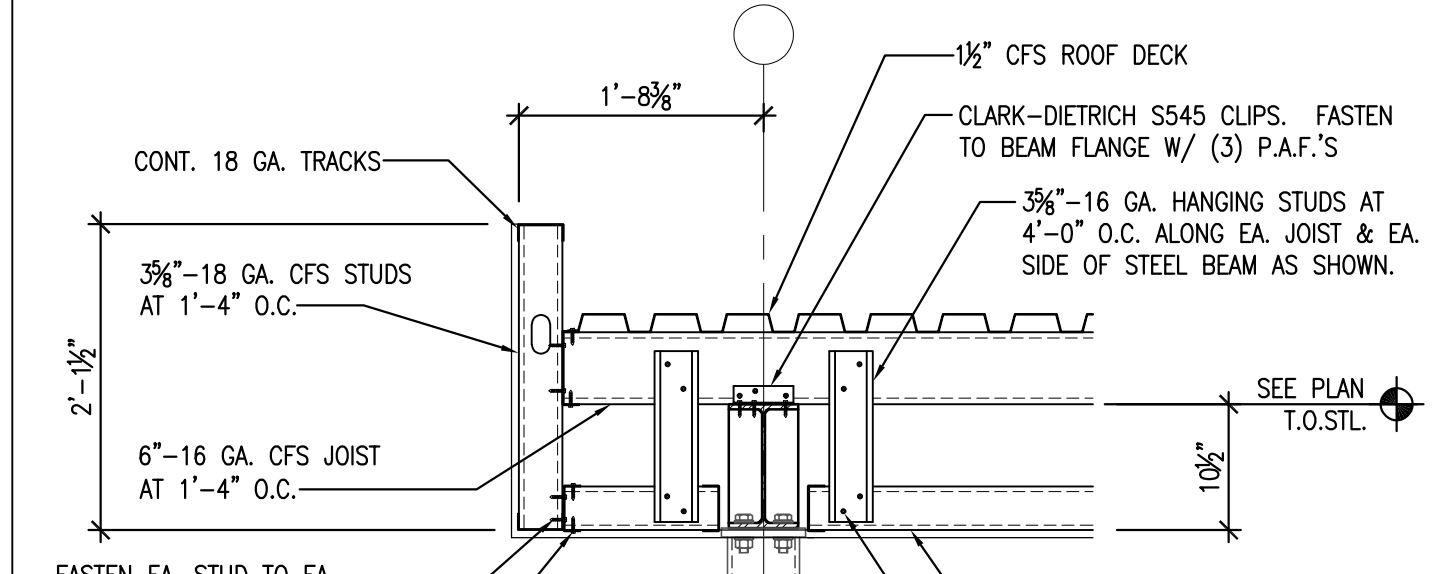
SEE ARCHITECTURAL BUILDING SECTIONS AND DETAILS FOR METAL BUILDING CONDITIONS ABOVE.



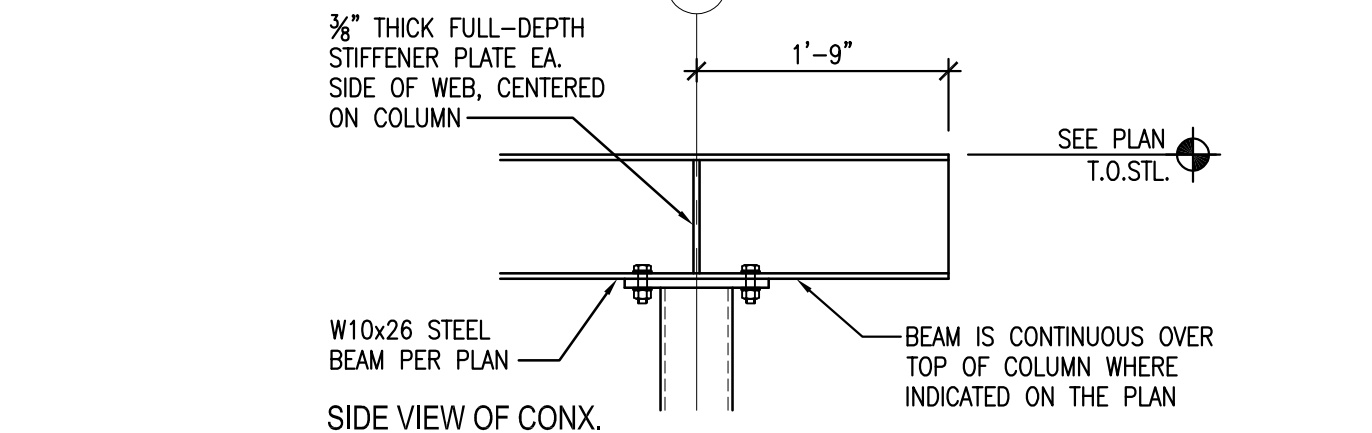
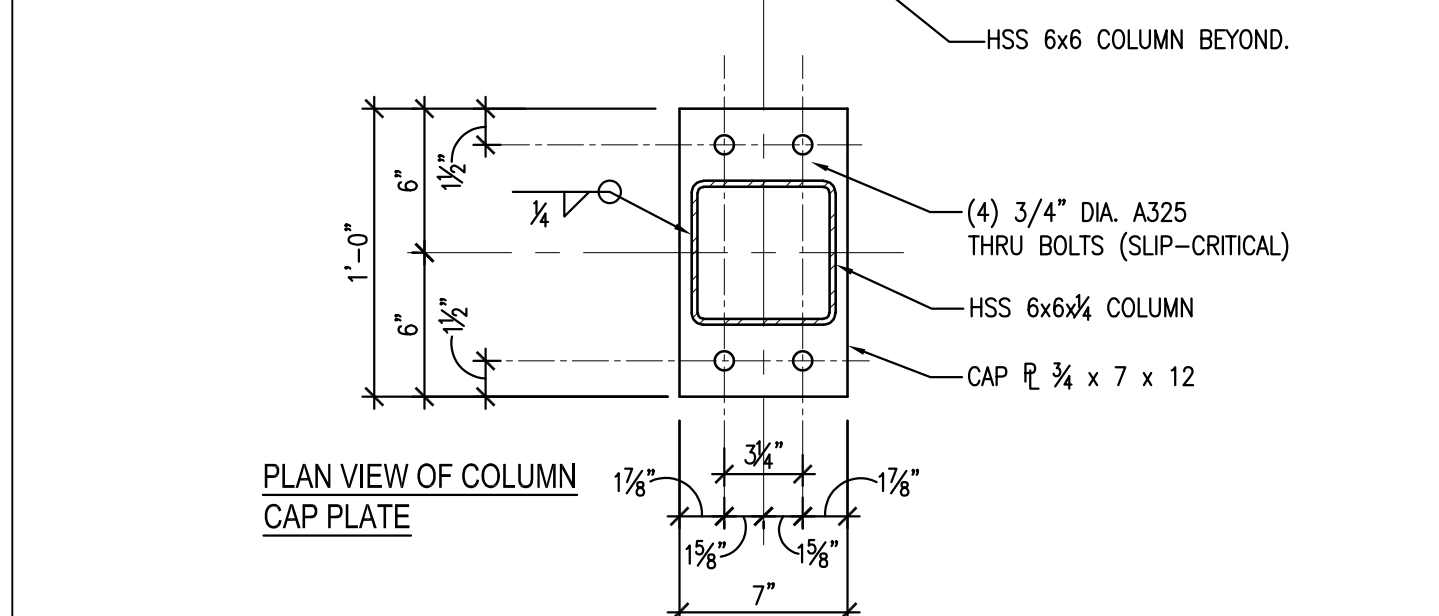
C4 SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"



B4 SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"

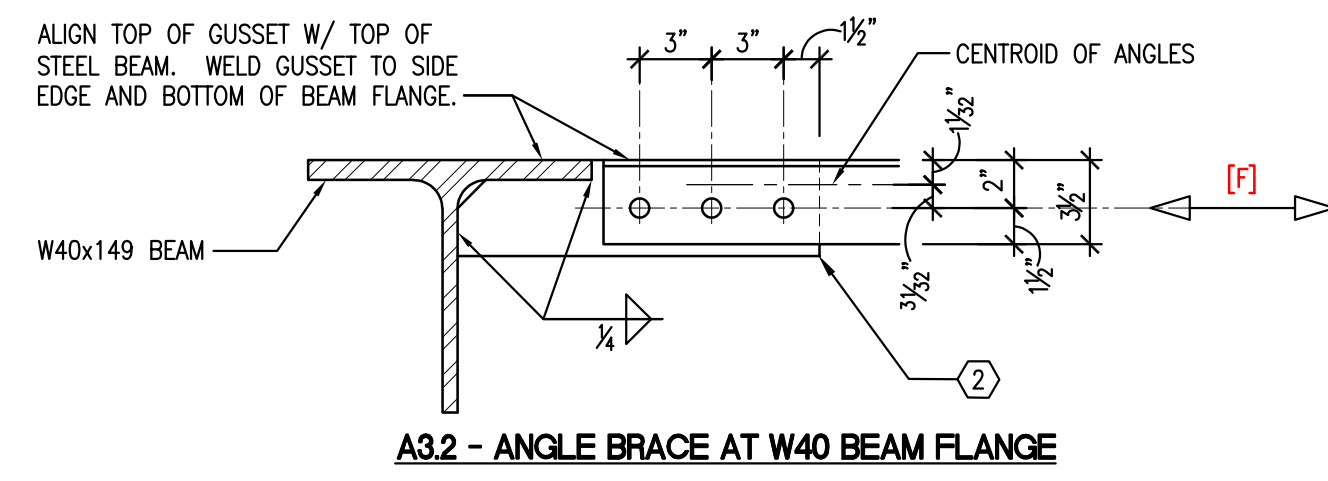


A4 SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"

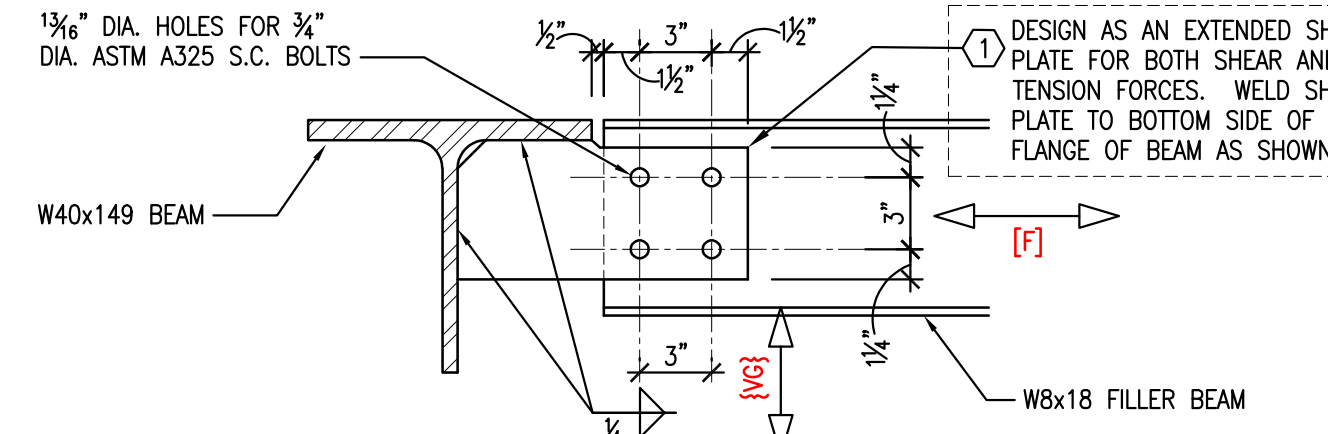


A4 SIDE ENTRANCE CANOPY SECTION
SCALE: 3/4" = 1'-0"

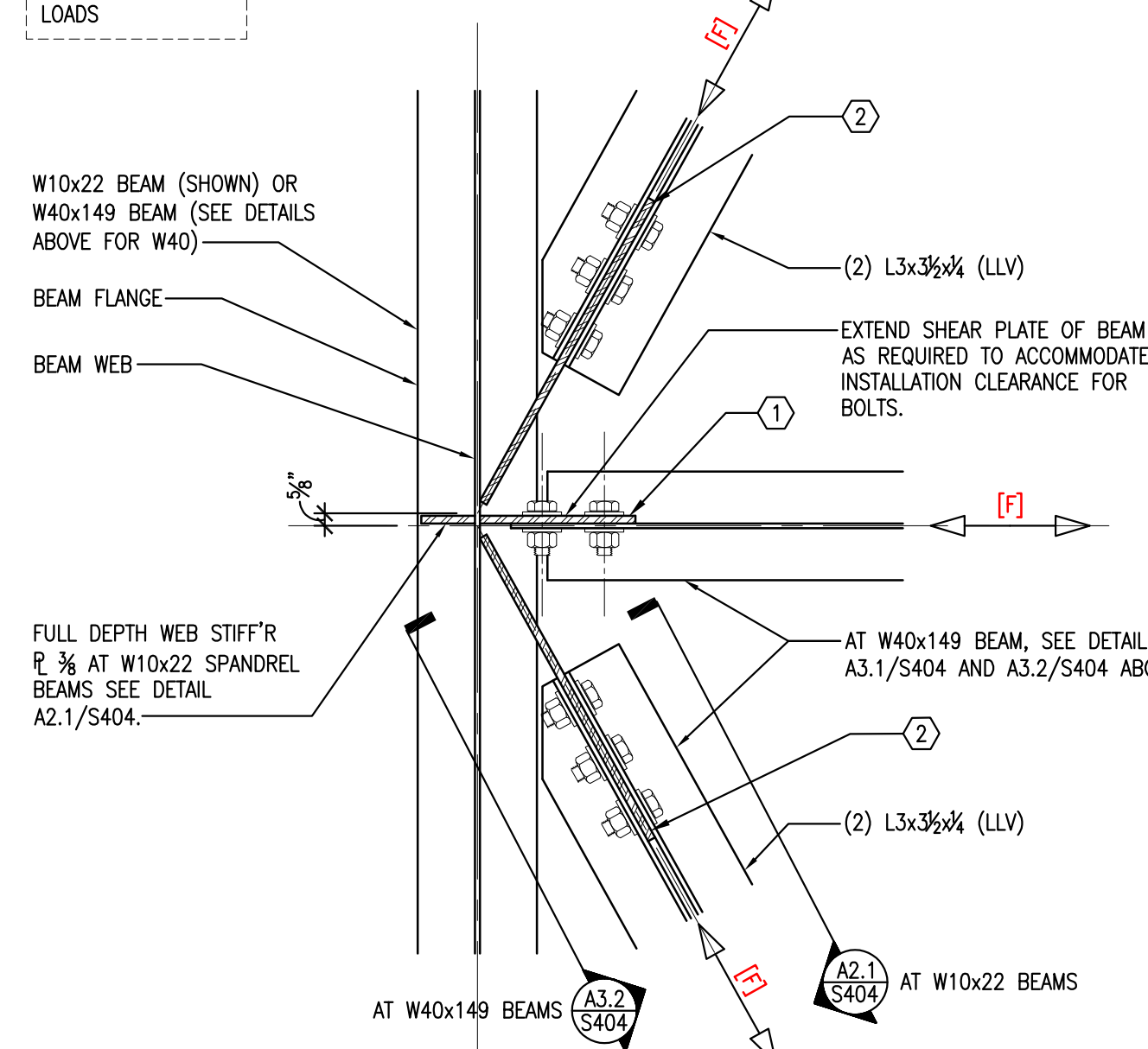
- KEYED NOTES FOR A1/S404 PLAN & CONNECTION DETAILS (BELOW):**
- SINGLE SHEAR $\frac{1}{8}$ " (MIN. PLATE THICKNESS) DESIGN FOR BOTH AXIAL TENSION AND SHEAR IN ONE OR BOTH DIRECTIONS (SEE LOADS ON A1/S404).
 - GUSSET $\frac{1}{8}$ " x 4" SANDWICHED BETWEEN DOUBLE ANGLE BRACES.
 - STANDARD ASS. DOUBLE ANGLE CONNECTION USING $\frac{3}{4}$ " THICK ANGLES. ADJUST ANGLE LOCATIONS TO ACCOMMODATE ROOM FOR ANGLE BRACE GUSSETS.
 - $\frac{1}{2}$ " DIAMETER THRU BOLT WITH PLATE WASHER $\frac{1}{8}$ " x 3 x 3 TO CONNECT THE PAIR OF BRACE ANGLES TOGETHER AT MID LENGTH.



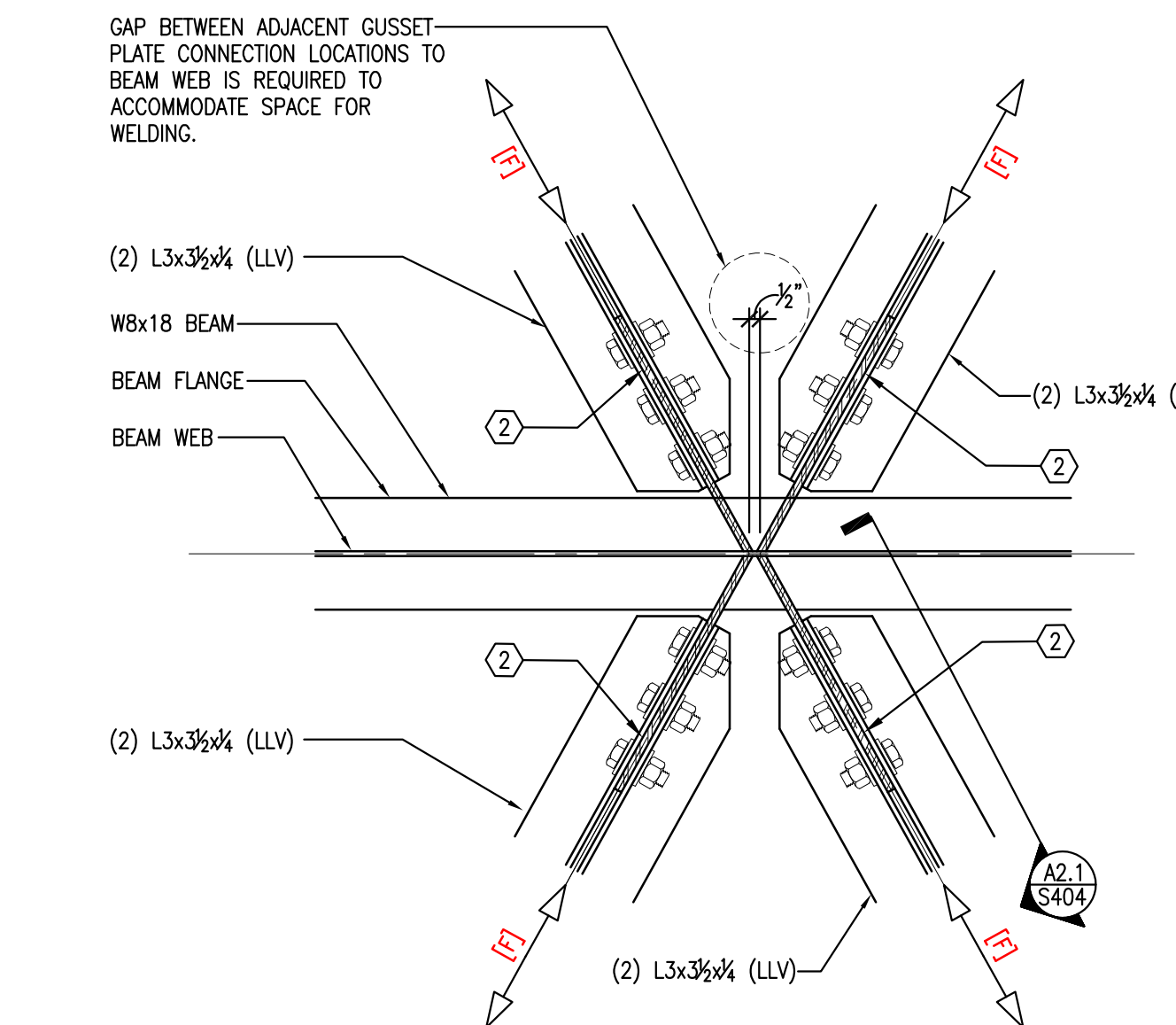
A32 - ANGLE BRACE AT W40 BEAM FLANGE



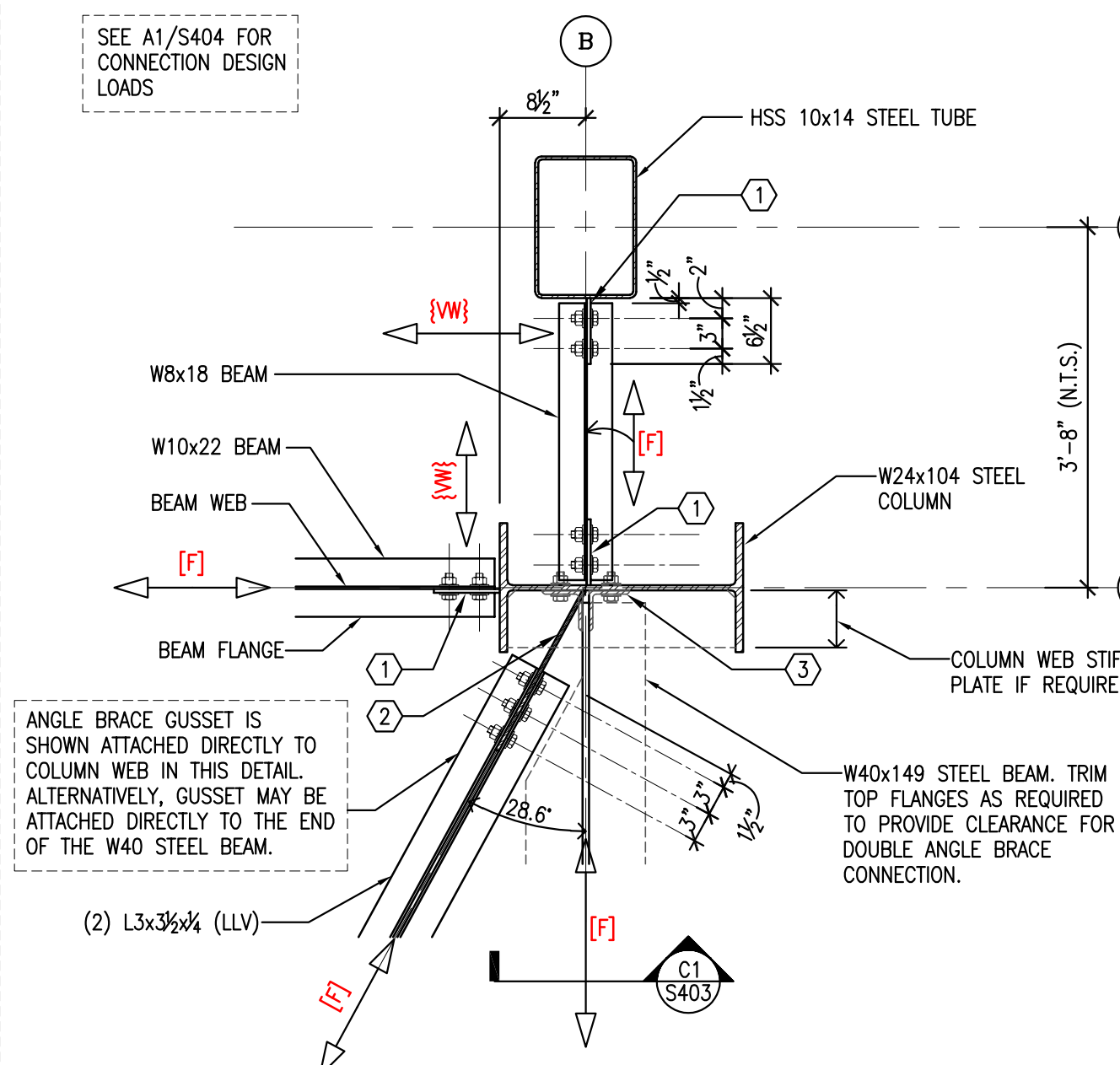
A31 - W8 FILLER BEAM AT W40 BEAM FLANGE



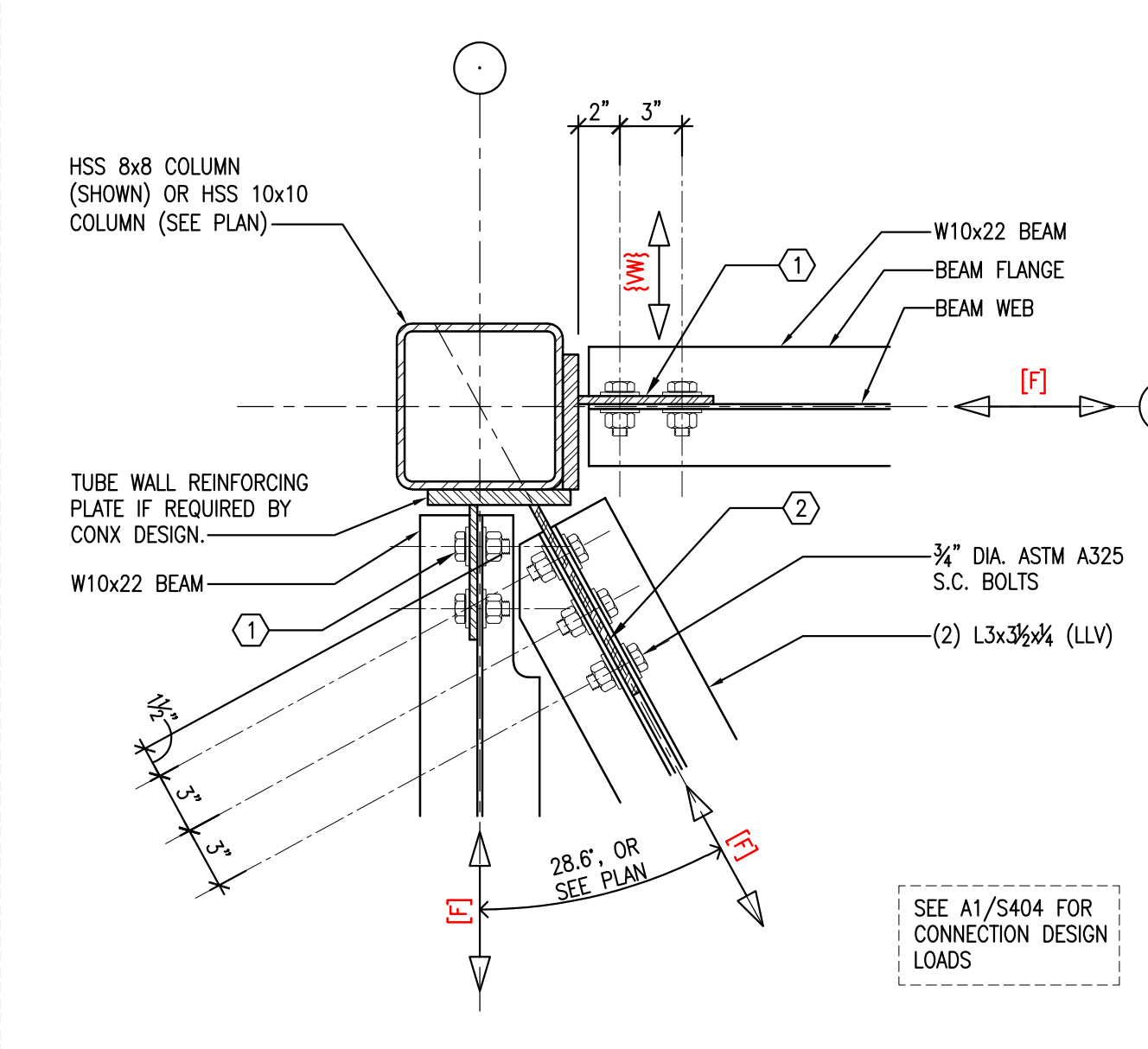
B3 DIAPHRAGM TRUSS CONX. CROSS BRACES
SCALE: 1-1/2" = 1'-0"



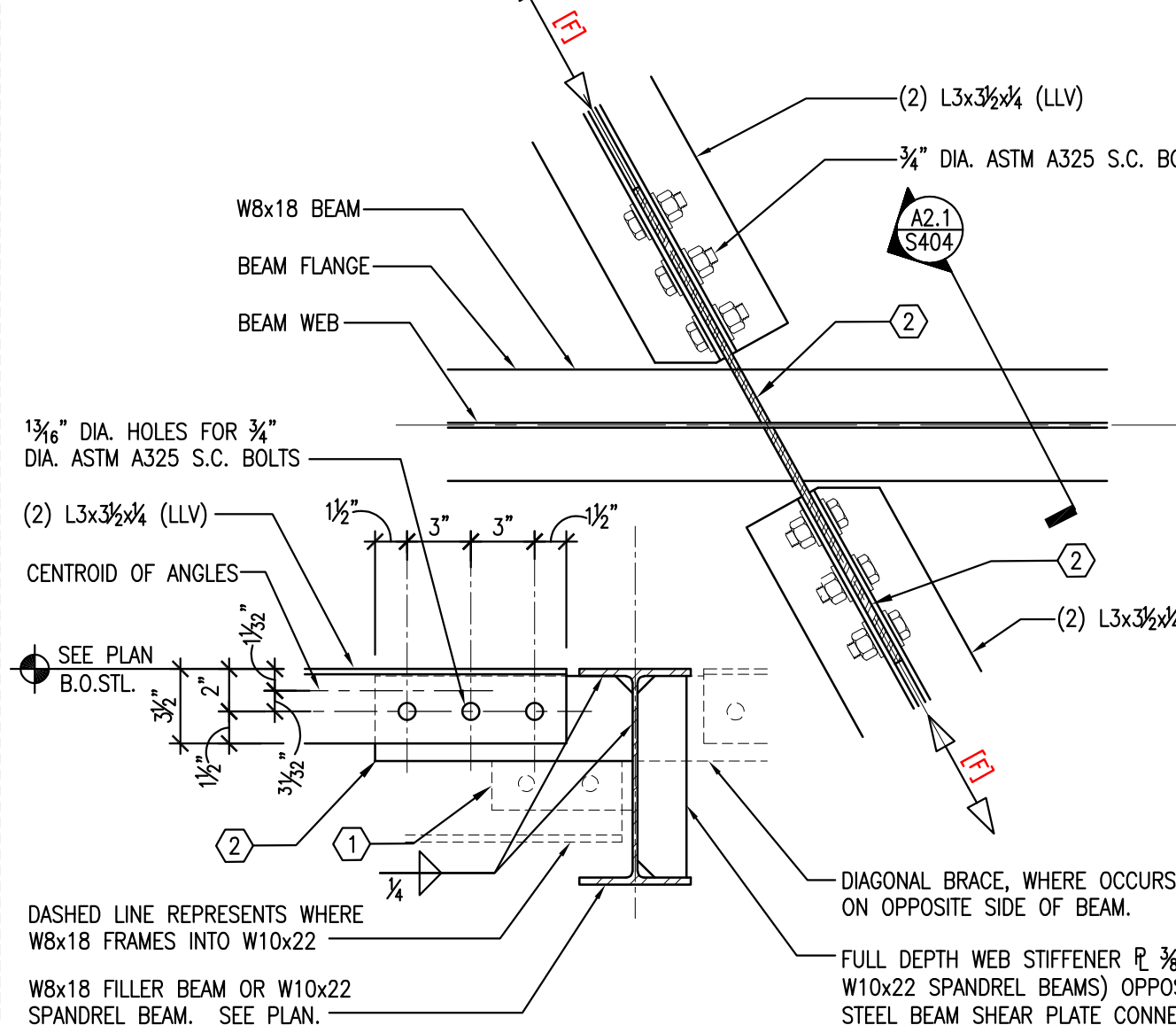
A3 DIAPHRAGM TRUSS CONX. CROSS BRACES
SCALE: 1-1/2" = 1'-0"



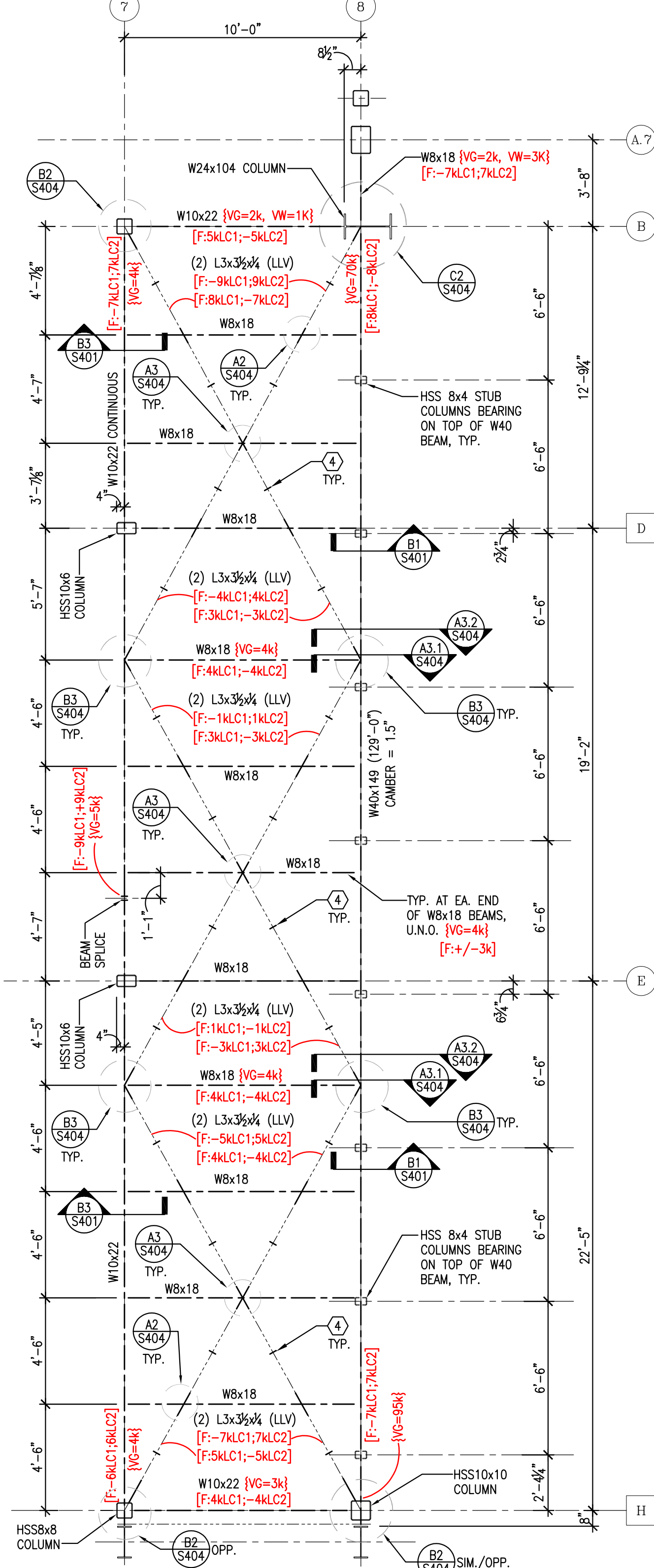
C2 DIAPHRAGM TRUSS CONX. AT COLUMN
SCALE: 3/4" = 1'-0"



B2 DIAPHRAGM TRUSS CONX. AT COLUMN
SCALE: 1-1/2" = 1'-0"



A2 DIAPHRAGM TRUSS CONX. DIAGONAL BRACE
SCALE: 1-1/2" = 1'-0"



A1 ENLARGED FRAMING PLAN, ROOF DIAPHRAGM TRUSS
SCALE: 3/4" = 1'-0"

- DIAPHRAGM TRUSS FRAMING NOTES**
- SEE SHEET S102 FOR ROOF FRAMING NOTES WHICH MAY APPLY.
 - TOP OF STEEL ELEVATION (T.O.STL.) = 129'-0", U.N.O.
 - RED COLORED TEXT INDICATES CONNECTION LOADS AT MEMBER ENDS. THE FABRICATOR'S PROFESSIONAL ENGINEER RESPONSIBLE FOR CONNECTION DESIGN SHALL DESIGN ALL BRACED FRAME CONNECTIONS, ANGLES, GUSSETS, AND BOLTS ACCORDING TO THE CONFIGURATIONS AND GEOMETRIES SHOWN FOR THE DESIGN LOADS AS PROVIDED.
 - ALL BOLTS SHALL BE ASTM F3125 (GRADE A325) SLIP CRITICAL.
 - ALL DIAGONAL BRACES: (2) L3x3/4 (LLV) W/ 3/8" GAP. PROVIDE 1/2" DIA. STITCH FASTENERS AND 3x3/8" PLATE WASHERS AT MID-LENGTH BUT STITCH FASTENERS SPACING SHALL NOT BE GREATER THAN 4'-0" O.C. STRUCTURAL CONNECTION DESIGN LOADS SHALL BE INTERPRETED ACCORDING TO THE FOLLOWING INSTRUCTIONS:
- THE NOTATION "VG" INDICATES A VERTICAL SHEAR FORCE DUE TO GRAVITY FORCES, "W" INDICATES A HORIZONTAL (OUT-OF-PLANE) SHEAR FORCE DUE TO WIND/SEISMIC, AND "F" INDICATES AN AXIAL FORCE ALONG THE AXIS OF THE MEMBER DUE TO WIND/SEISMIC.
 - FORCES DUE TO WIND OR SEISMIC ARE EXPRESSED AT STRENGTH LEVEL. CONNECTION ENGINEER SHALL APPLY THE APPLICABLE LOAD COMBINATION FACTORS. LATERAL FORCES MAY BE ASSUMED TO BE CONTROLLED BY WIND UNLESS NOTED OTHERWISE.
 - SIGN CONVENTION FOR AXIAL FORCES: A NEGATIVE SIGN (-) INDICATES TENSION, AND A PLUS SIGN (+) INDICATES COMPRESSION.
 - DIAGONAL BRACE REACTIONS ARE PROVIDED (IN KIIPS) IN SQUARE BRACKETS THUS: [-2.5k, LC1]. LC1 INDICATES LOAD CASE #1 AND LC2 INDICATES LOAD CASE #2.
 - BEAM END REACTIONS ARE PROVIDED (IN KIIPS) AT THE ENDS OF A BEAM IN CURLY BRACKETS, THUS: [110k, F10k]. LOADS SHOWN NEXT TO THE BEAM SIZE SHALL BE APPLIED TO EACH END.
 - IN ORDER TO SIMPLIFY THE LOAD DATA, MAXIMUM LOADS ARE SHOWN AT EACH MEMBER OR CONNECTION FACE AND THEREFORE LOADS MAY NOT BALANCE AT THE JOINT. TO COMPENSATE, AXIAL LOADS IN THE BEAMS OR BRACES MAY BE INCREASED TO BALANCE THE JOINT. ADDITIONAL OR MORE PRECISE LOAD DATA CAN BE PROVIDED UPON REQUEST.

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LICENSE EXPIRES: 06-21-2025
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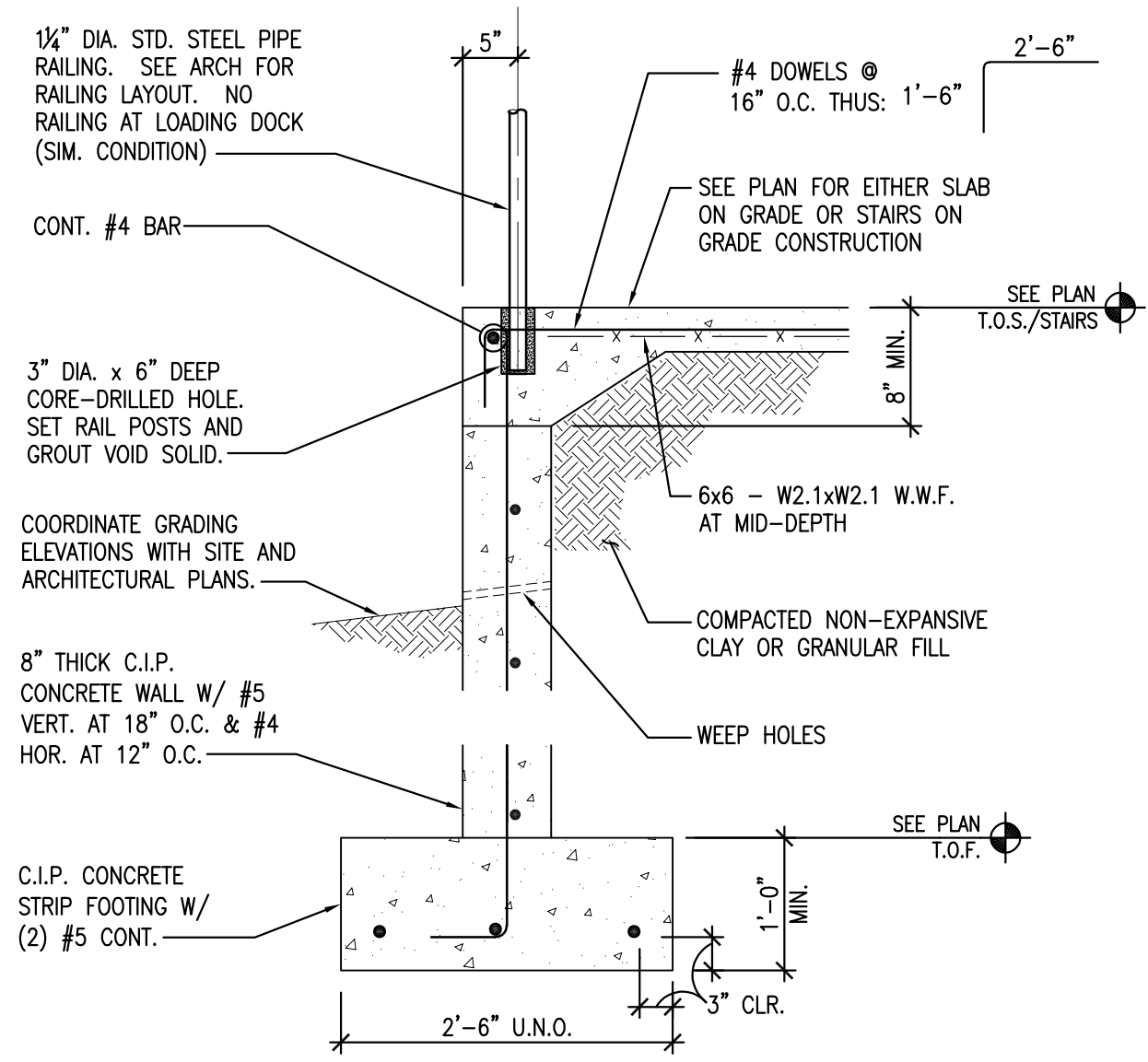
NEW BUILDING FOR:
GALILEE BAPTIST CHURCH
6300 WOODYARD ROAD
UPPER MARLBORO, MD 20772

DRAWING DATE
17 NOV 2023

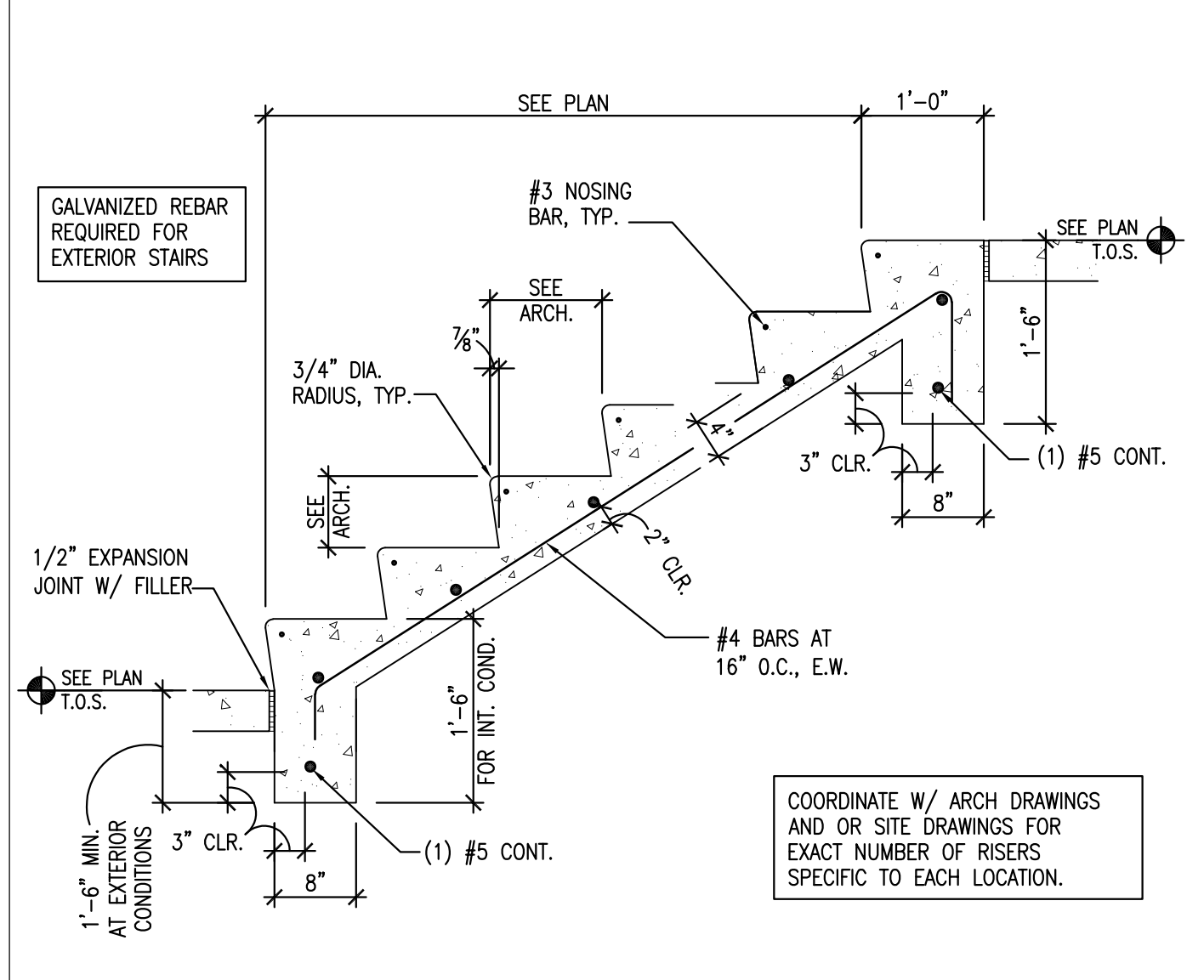
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FRAMING DETAILS

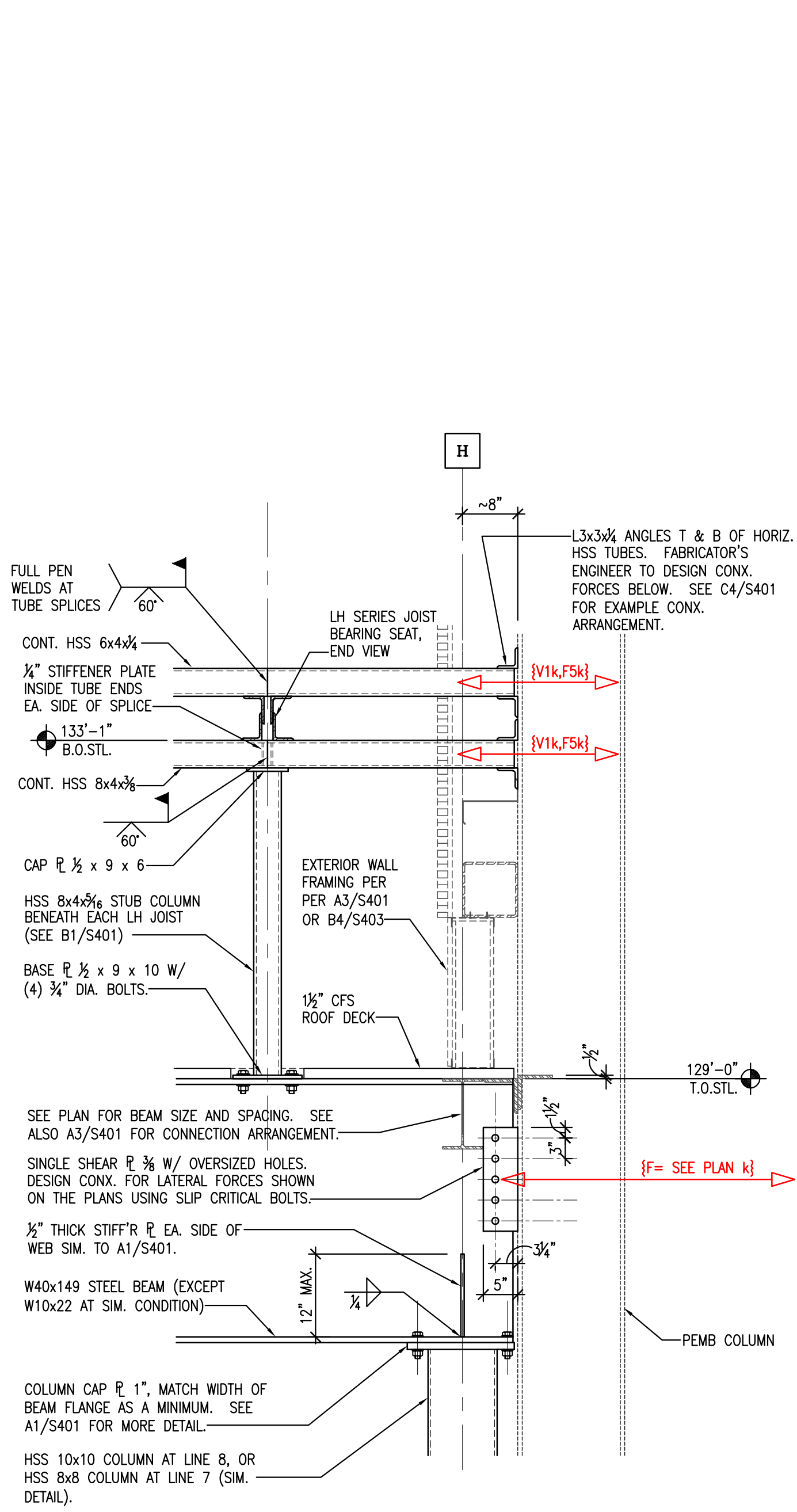
S404
OF 18 SHEETS
216118



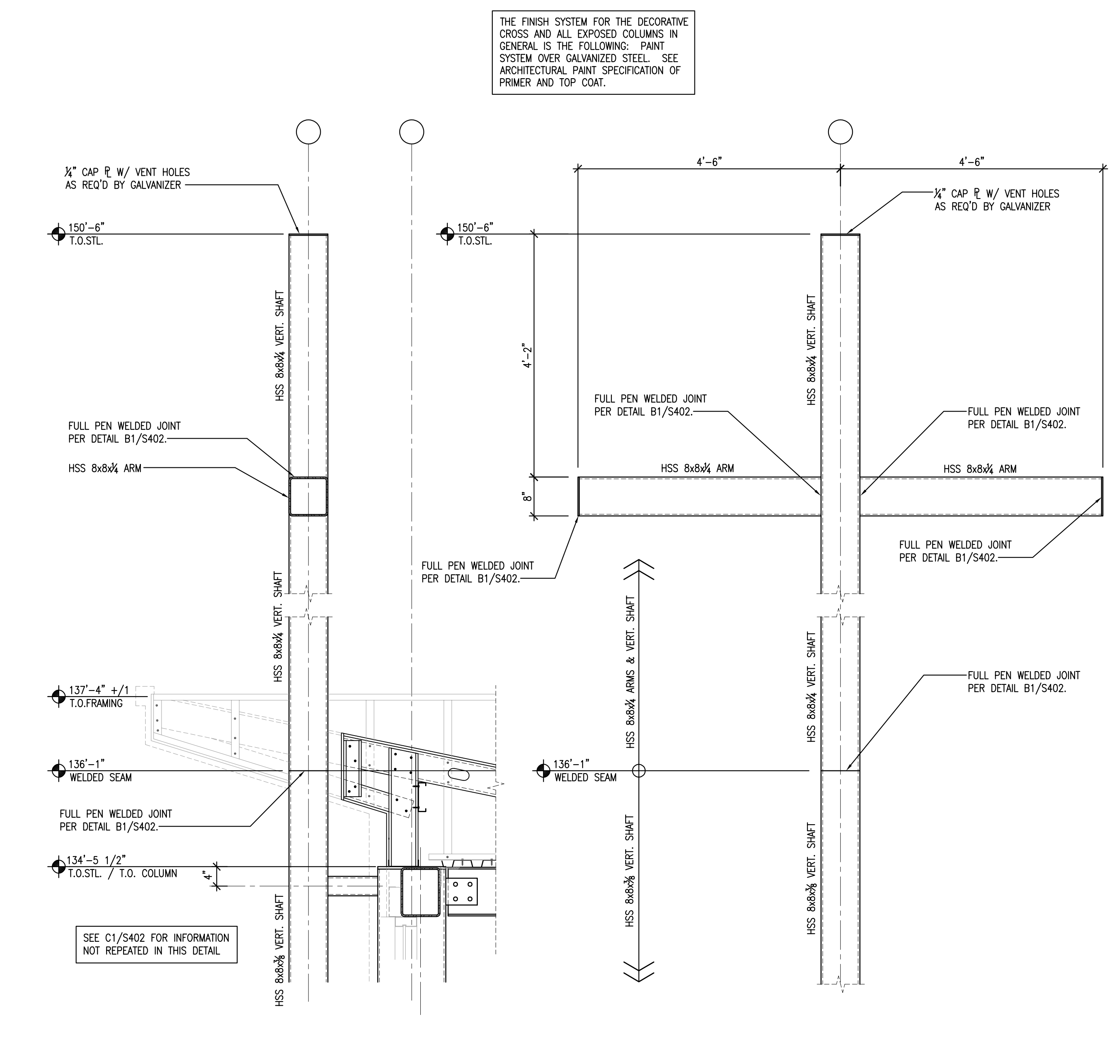
C4 RETAINING WALL AT EXTERIOR WALK
SCALE: 3/4" = 1'-0"



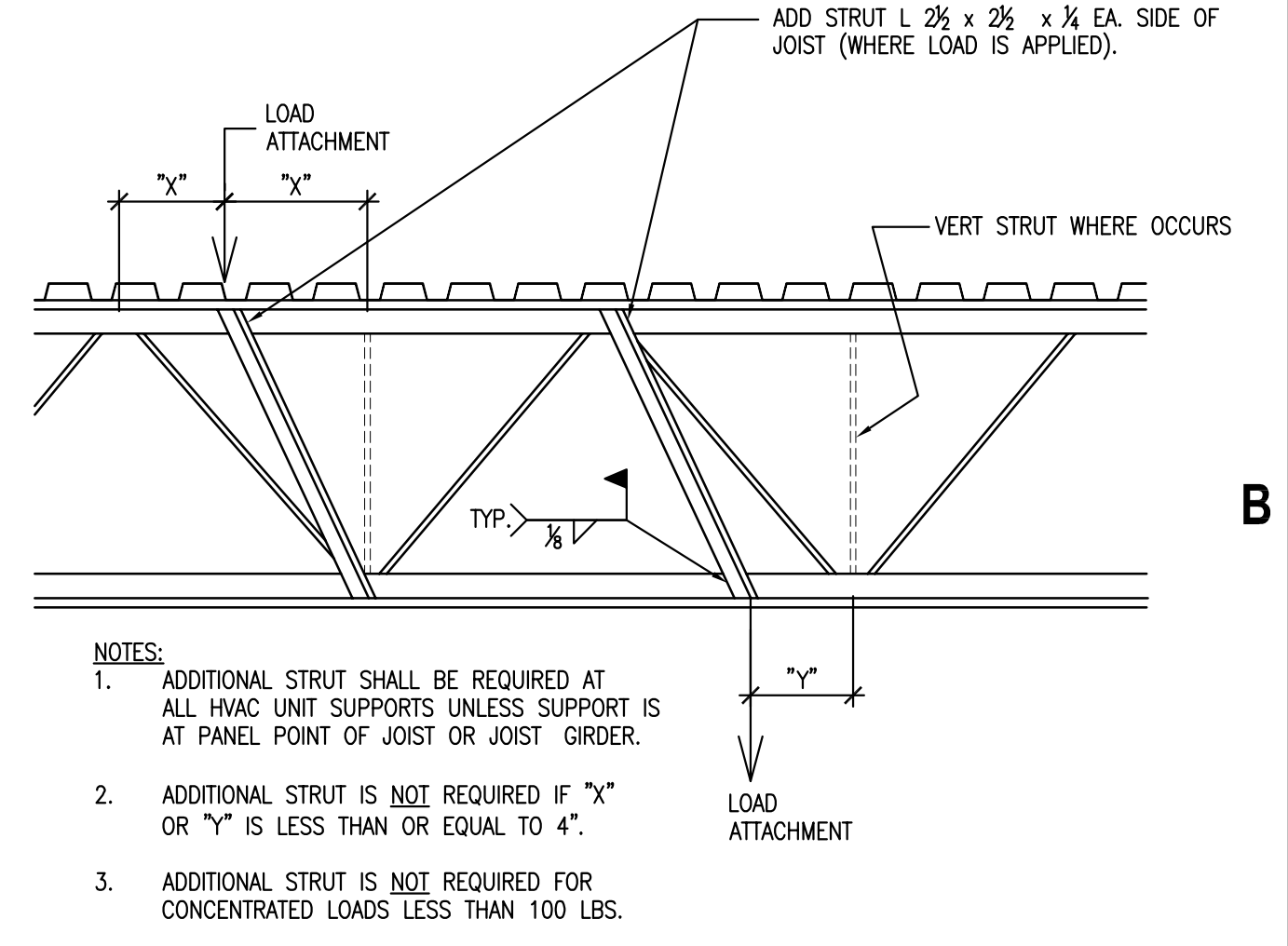
C3 TYPICAL CONCRETE STAIRS ON GRADE DETAIL
SCALE: 3/4" = 1'-0"



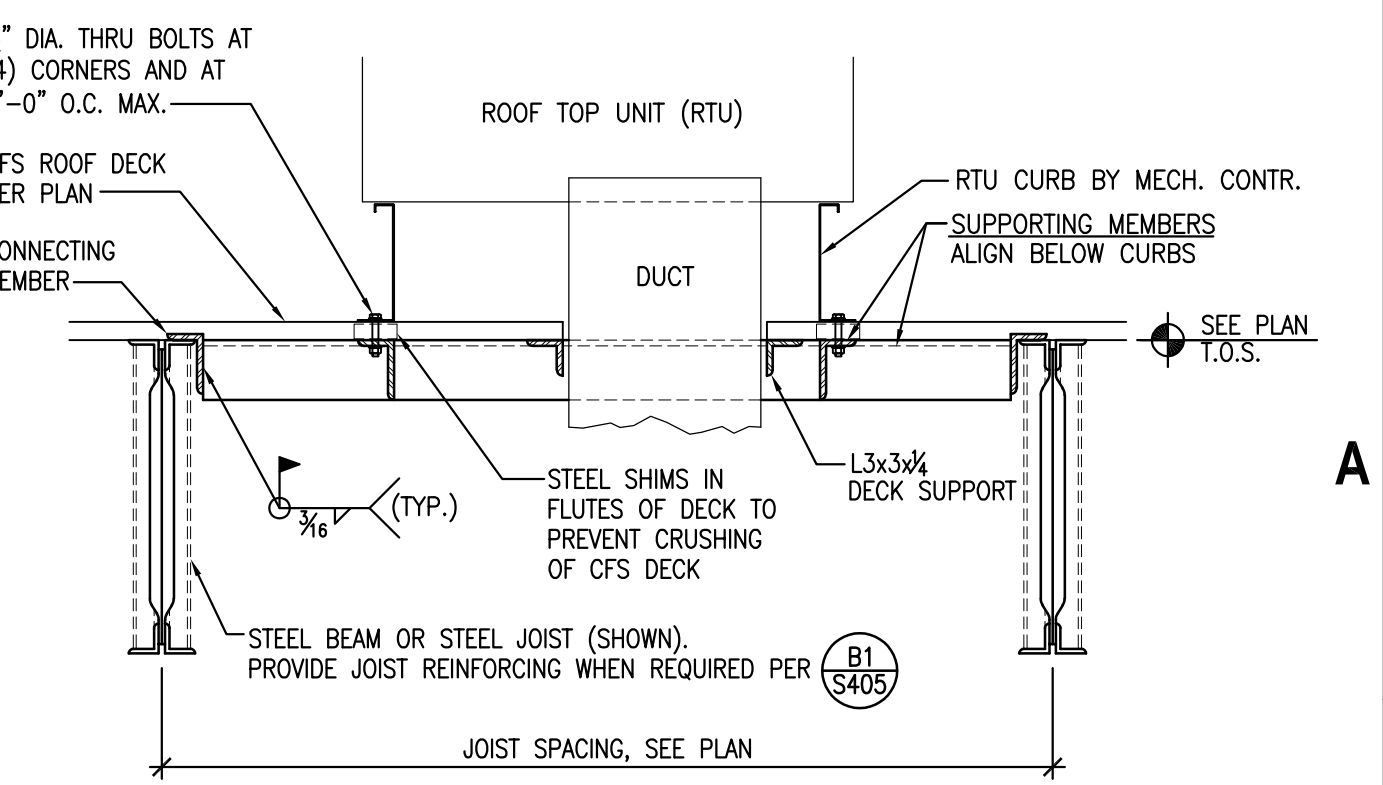
A4 ROOF GIRDER BEAM SUPPORT DETAIL
SCALE: 3/4" = 1'-0"



A3 ELEVATION VIEW OF FRONT ENTRANCE ORNAMENTAL CROSS
SCALE: 3/4" = 1'-0"



B1 JOIST REINFORCING DETAIL
SCALE: N.T.S.



| MAX. EQUIPMENT WT. | SUPPORTING MEMBER | CONNECTING MEMBER |
|--------------------|-------------------|----------------------------|
| 900 LBS. | L 3 x3 x1/4 | L 3 x3 x1/4 x6" LONG |
| 2400 LBS. | L 5 x3 x1/4 LLV | L 5 x3 x1/4 x6" LONG |
| 3500 LBS. | C 6x8.2 | BENT PL 8 x3 x1/4 x8" LONG |

A1 MECHANICAL RTU FRAME DETAIL
SCALE: N.T.S.

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