



A BANK BARN RESTORATION FOR:

FRIENDS OF JERUSALEM MILL
(GUNPOWDER FALLS STATE PARK)
2801 JERUSALEM RD.
KINGSVILLE MD. 21087

GENERAL NOTES - :
THESE NOTES APPLY TO ALL DRAWINGS:
1. PROVIDE EROSION PROTECTION AS REQ'D. BY ALL GOVERNING AUTHORITIES.
2. PROVIDE P.T. WD. WHERE IN CONTACT W/ FLASHING, CONCRETE, OR EARTH.
3. ALL WOOD FASTENERS, HANGERS, ETC. IN CONTACT W/ P.T. WD. TO RECEIVE MIN. G90 GALV., OR GREATER AS REQ'D BY FASTENER MFR. FOR P.T. CHEM. AND/OR ENVIRO.
4. STRUCTURAL DESIGN CONFORMS WITH 2021 MARYLAND BUILDING CODE
5. PROPOSED USE GROUP - ASSEMBLY A-3 (UPPER LEVEL)
6. MODERATE, HAZARDOUS STORAGE (LOWER LEVEL)
7. CONSTRUCTION TYPE - IV-HLT HEAVY TIMBER
8. OCCUPANT LOAD WILL BE 100 PERSONS OR LESS
9. THE SCOPE OF WORK WILL ADDRESS THE PORTIONS OF THIS DESIGN TO BE IMPLEMENTED WITH THE FIRST PHASE OF CONSTRUCTION

OVERALL BUILDING HEIGHT: 39'-9"
BUILDING COVERAGE AREA: 2654 S.F.

GENERAL STRUCTURAL NOTES:
1. FOUNDATIONS DESIGNED FOR ASSUMED 2500 PSF ALLOW. NET SOIL BEARING. OWNER TO RIV FOR SOIL TESTS AND FOR REDDESIGN & INCREASED CONSTRUCTION COSTS IF SOIL INADEQUATE.
2. PLACE FILL & BACKFILL IN MAX. 8" LIFTS. DO NOT BUILD ON WEAK SOIL.
3. DESIGN LIVE LOADINGS AS FOLLOWS:
- G.S.L: 30 PSF
- WIND: 115 MPH (EXPOSURE B) - SEISMIC USE GROUP: 1
- SITE CLASS: D
- LDT FLOOR: 120 PSF
- SEISMIC RESIST: WOOD FRAME
4. CONC. WORK TO MEET TO ALL PROVISIONS OF LATEST EDS. OF ACI 308 & 318. INCL. FREEZING & HOT WEATHER PROVISIONS. TYPE I CEMENT PER ASTM C595. MAX. 1 IN. STONE PER ASTM C33. SLUMP 4 IN +/- 1 IN. NO CALC. CHLORIDE. CONCRETE TO HAVE 28 DAY MIN. COMPRESSIVE STRENGTH OF 5000 PSI EXCEPT 4000 PSI AT EXPOSED EXT. WORK. AIR ENTRAIN CONCRETE AT EXT. HORIZ. SURFACES: EN 4/- 1.5%. CONC. TO BE MOST CURED OR PROTECTED W/ MEMBRANE.
5. WOOD FRAMING DESIGN COMPLIES W/ NDS 2015
6. CONSTRUCTION TYPE - IV-HLT HEAVY TIMBER

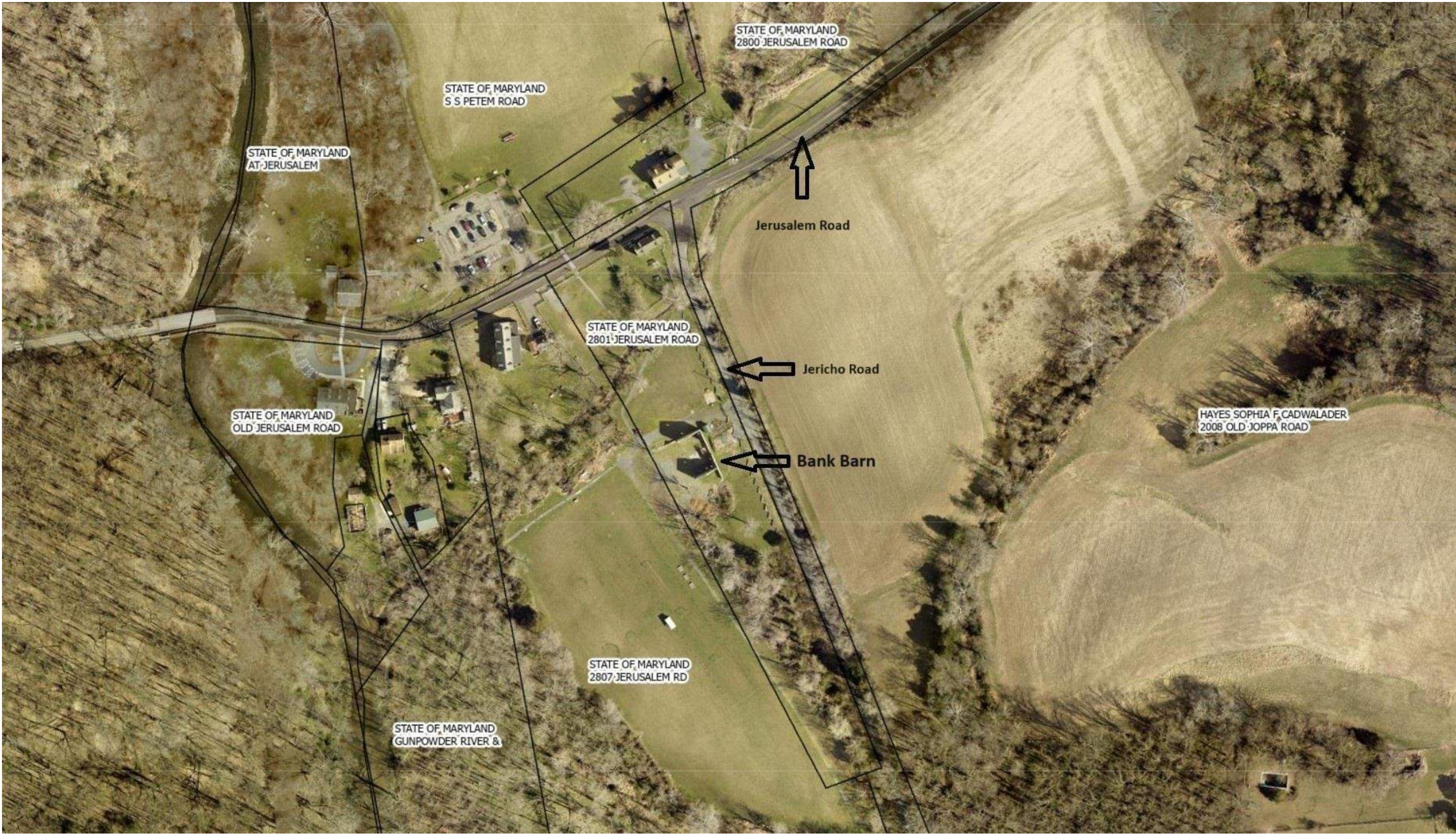
APPLICABLE BUILDING CODES:
1. 2021 MARYLAND BUILDING CODE
2. 2018 MARYLAND LIFE SAFETY CODE
3. MARYLAND ACCESSIBILITY CODE (2010 ADA STANDARDS)
4. 2021 MARYLAND ENERGY CODE
5. MARYLAND FIRE PREVENTION CODE
6. 2021 MARYLAND PLUMBING CODE
7. 2019 MARYLAND FIRE ALARM CODE
8. 2021 MARYLAND ELECTRICAL CODE

SCOPE OF WORK :

1. PROVIDE NEW FOUNDATIONS & FOOTERS PER PLANS. (EXISTING STONE FOUNDATION WALLS TO REMAIN.)
2. PROVIDE NEW CONCRETE FLOOR PER PLANS.
2. PROVIDE NEW FRAMING PER PLANS.
3. INSTALL NEW WINDOWS & DOORS PER PLANS.
4. PROVIDE NEW EXTERIOR ROOF, SIDING & TRIM PER PLANS.
5. PROVIDE NEW INTERIOR FINISHES PER PLANS.

DRAWING SCHEDULE

- PAGE A-1 - COVER PAGE , CODE NOTES , SCOPE OF WORK
PAGE A-2 - LOWER LEVEL PLAN , WINDOW & DOOR SCH.
PAGE A-3 - UPPER FLOOR PLAN , WINDOW & DOOR SCH.
PAGE A-4 - ELEVATIONS
PAGE A-5 - CROSS SECTION A , STAIR DETAIL , BARRED OPENING DETAILS
PAGE A-6 - CROSS SECTION B , DBL. DOOR DETAILS
PAGE A-7 - CROSS SECTION C , DOOR DETAILS
PAGE A-8 - ROOF FRAMING PLAN , HDB/HD HOLDDOWNS DETAILS,PLAN VIEW
PAGE A-9 - UL303 FIRE DETAIL & NOTES
PAGE A-10 - FOUNDATION PLAN
PAGE A-11 - 3D RENDERINGS
PAGE S-01 - GENERAL NOTES
PAGE S-1 - STRUCTURAL FOUNDATION & LOWER FLOOR PLAN



CONTRACTOR:

717-419-1809

DAVID S KING

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2813 JERUSALEM RD.
KINGSVILLE MD. 21087

PROJECT # 8141

DRAWN BY:
I.L. ZOOK

REVISIONS:
I.L. ZOOK - 6.7.2024
I.L. ZOOK - 4.2.2025
I.L. ZOOK - 5.17.2025
I.L. ZOOK - 8.8.2025
I.L. ZOOK - 8.29.2025
I.L. ZOOK - 9.25.2025
I.L. ZOOK - 11.1.2025

PAGE: 1

DATE:
8.29.2025

DRAWING TITLE:
COVER PAGE

SHEET NO.

A-1

LOWER LEVEL - DOOR SCHEDULE						
DOOR LABEL	DOOR SIZE	MATERIAL/HINGE	GLASS	NOTES	FIRE RATING	
D-1	3668 - G.C. TO VERIFY	WOOD	NONE	ADA COMPLIANT	NO	
D-2	3668 - G.C. TO VERIFY	WOOD	NONE	ADA COMPLIANT	NO	
D-3	3068	WOOD	NONE	8" BARRED OPENING ABOVE	NO	
D-4	3068	WOOD	NONE	8" BARRED OPENING ABOVE	NO	
D-5	6068	WOOD	NONE	8" BARRED OPENING ABOVE	NO	
D-6	3068	WOOD	NONE	8" BARRED OPENING ABOVE	NO	
D-7	3068	WOOD	NONE	8" BARRED OPENING ABOVE	NO	
D-8	3068	STEEL	NONE	ADA COMPLIANT	1-HR.	
D-9	3068	STEEL	NONE	ADA COMPLIANT	1-HR.	
D-10	3068	STEEL	NONE	ADA COMPLIANT	1-HR.	
D-11	3068	STEEL	NONE	ADA COMPLIANT	1-HR.	

BARN DOOR PAINT DETAILS:
ALL EXTERIOR DOORS, WINDOWS, LOUVERS, SIDING & TRIM TO BE PAINTED
W/ (2) COATS OF LINSEED OIL BASED PAINT (EXTERIOR ONLY)

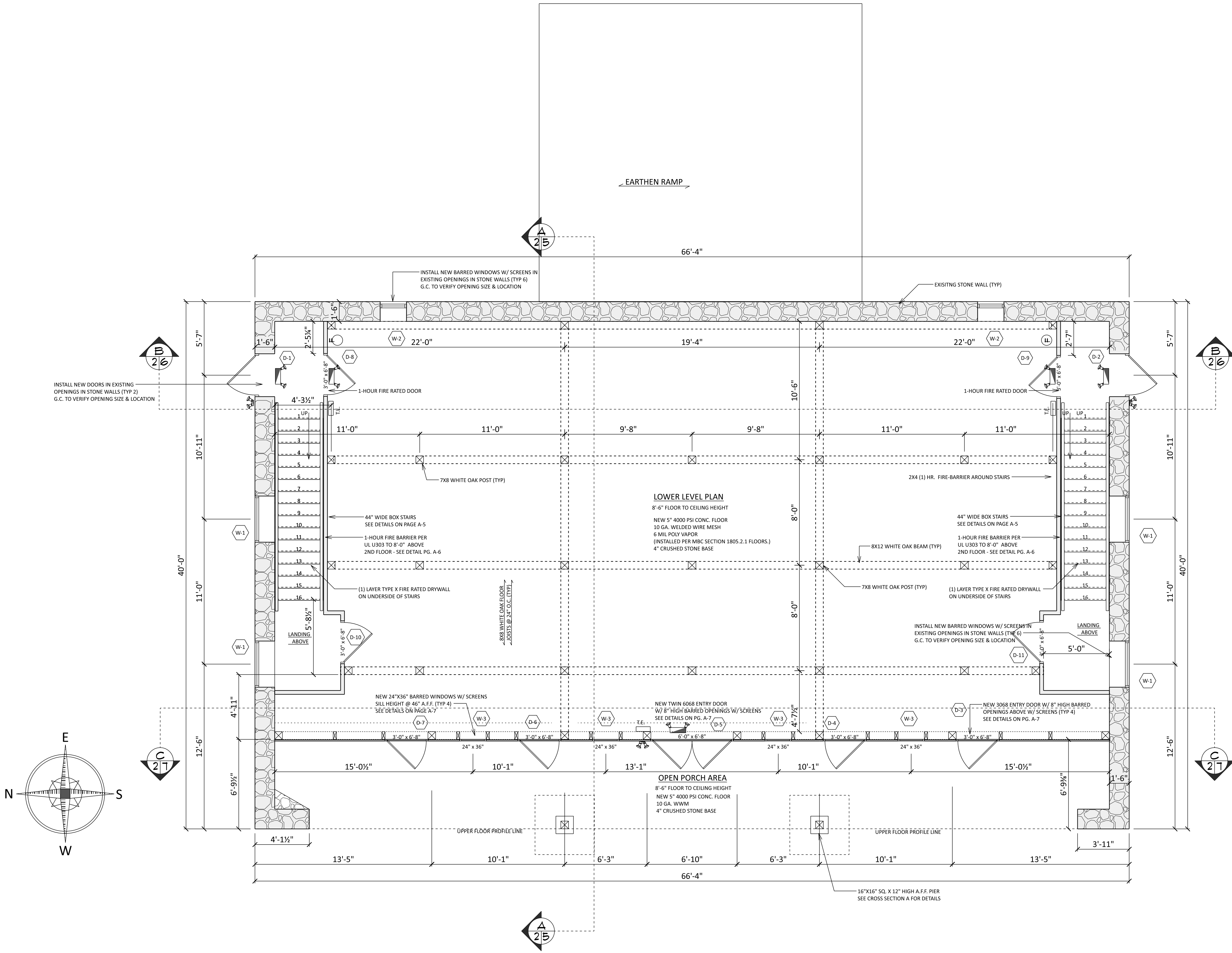
LOWER LEVEL - WINDOW SCHEDULE						
WINDOW LABEL	WINDOW SIZE	QTY	MATERIAL	TYPE	NOTES	FIRE RATING
W-1	3'-6" X 4'-0" - G.C. TO VERIFY	4	WOOD	BARRED	SCREEN ON INSIDE	NO
W-2	2'-0" X 3'-0" - G.C. TO VERIFY	2	WOOD	BARRED	SCREEN ON INSIDE	NO
W-3	2'-0" X 3'-0"	4	WOOD	BARRED	SCREEN ON INSIDE	NO

SYMBOL LEGEND:

- DETAIL NUMBER ABOVE SHEET WHERE LOCATED.
- SECTION OR ELEV. NUMBER ABOVE SHEET WHERE LOCATED.
- ELEVATION NUMBER (OUTSIDE SYMBOL) AROUND SHEET WHERE LOCATED.
- DOOR NUMBER
- ELEVATION BULLET
- ILLUMINATED EXIT SIGN - SEE ELEC.
- ILLUMINATED EGRESS LIGHT - SEE ELEC.
- FIRE EXTINGUISHER: MIN. 4-A 10-BC U.N.O. BRACKET MOUNT W/ TOP OF F.E. AT MAX. 48" A.F.F. & U.S. OF F.E. BETW 8" & 27" A.F.F. NOTE: FIRE EXTINGUISHERS MUST BE PLACED LESS THAN 75'-0" OF TRAVEL FROM ALL AREAS
- TACTILE EXIT SIGN

DOOR NOTES

FINISH: SATIN CHROME, BHMA 626 IF ON BRASS; BHMA 652 IF ON STEEL
FIRE-RATED DOORS & FRAMES TO COMPLY W/ UL/FM, OR WHI.
ALL HARDWARE TO CONFORM W/ H.C. REQ'TS, INCL. LEVER HDWR & CLOSER FORCE.
(SPECIFICALLY IBC SECTION 010.3.3 AND A117.1 SECTION 404.2.8.)
ALL HARDWARE ON FIRE-RATED DOORS TO BE FIRE-RATED.
(HARDWARE TO CONFORM TO BHMA GRADE 1, A156.2 (BORED LATCHSETS), A156.1 (HINGES), AND A156.4 (CLOSERS).)
ALL FIRE-RATED DOORS TO HAVE CLOSERS, (MOUNTED ON PUSH SIDE OF DOORS)



LOWER LEVEL PLAN

SCALE: 1/4" = 1'-0"

CONTRACTOR:

717-419-1809

DAVID S KING

CLASSIC
DESIGN LLC
1201 ROTHVILLE RD
LITITZ PA 17543
Drafting & Architectural Services...

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PAGE: 2 / 2

DATE:
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DRAWING TITLE:
LOWER LEVEL PLAN

SHEET NO.

A-2

UPPER LEVEL - DOOR SCHEDULE						
DOOR LABEL	DOOR SIZE	MATERIAL	HARDWARE	GLASS	NOTES	FIRE RATING
D-1	19'-6 1/2" X 9'-10"	WOOD	SEE DTL PG. A-6	NONE	ADA COMPLIANT	NO
D-2	3068	WOOD	FAKE EXT. HINGES	NONE	FIXED DOOR - NON OPERABLE	NO
D-3	3068	STEEL	PANIC BAR	NONE	ADA COMPLIANT DISGUISE AS GRANARY DOORS	1-HR.
D-4	3068	STEEL	PANIC BAR	NONE	ADA COMPLIANT DISGUISE AS GRANARY DOORS	1-HR.
D-5	2850	WOOD	SEE DTL PG. A-6	NONE	BUILT-IN	NO

BARN DOOR PAINT DETAILS:
ALL EXTERIOR DOORS, WINDOWS, LOUVERS, SIDING & TRIM TO BE PAINTED
W/ (2) COATS OF LINSEED OIL BASED PAINT (EXTERIOR ONLY)

UPPER LEVEL - WINDOW SCHEDULE					
WINDOW LABEL	WINDOW SIZE	QU.	MATERIAL	TYPE	NOTES
W-1	3'-6" X 4'-0"	2	WOOD	DBL. HUNG	SINGLE PANE
	G.C. TO VERIFY				

SYMBOL LEGEND:

1

A.1

DETAIL NUMBER ABOVE SHEET WHERE LOCATED.

1

A.1

SECTION OR ELEV. NUMBER ABOVE SHEET WHERE LOCATED.

1

A.1

ELEVATION NUMBER (OUTSIDE SYMBOL) AROUND SHEET WHERE LOCATED.

D-2

DOOR NUMBER

ELEVATION BULLET

ILLUMINATED EXIT SIGN - SEE ELEC.

ILLUMINATED EGRESS LIGHT - SEE ELEC.

F

FIRE EXTINGUISHER: MIN. 4-A 10-BC U.N.O. BRACKET MOUNT W/ TOP OF F.E. AT MAX. 48" A.F.F. & U.S. OF F.E. BETW 8" & 27" A.F.F. NOTE: FIRE EXTINGUISHERS MUST BE PLACED LESS THAN 75'-0" OF TRAVEL FROM ALL AREAS

T.E.

TACTILE EXIT SIGN

DOOR NOTES

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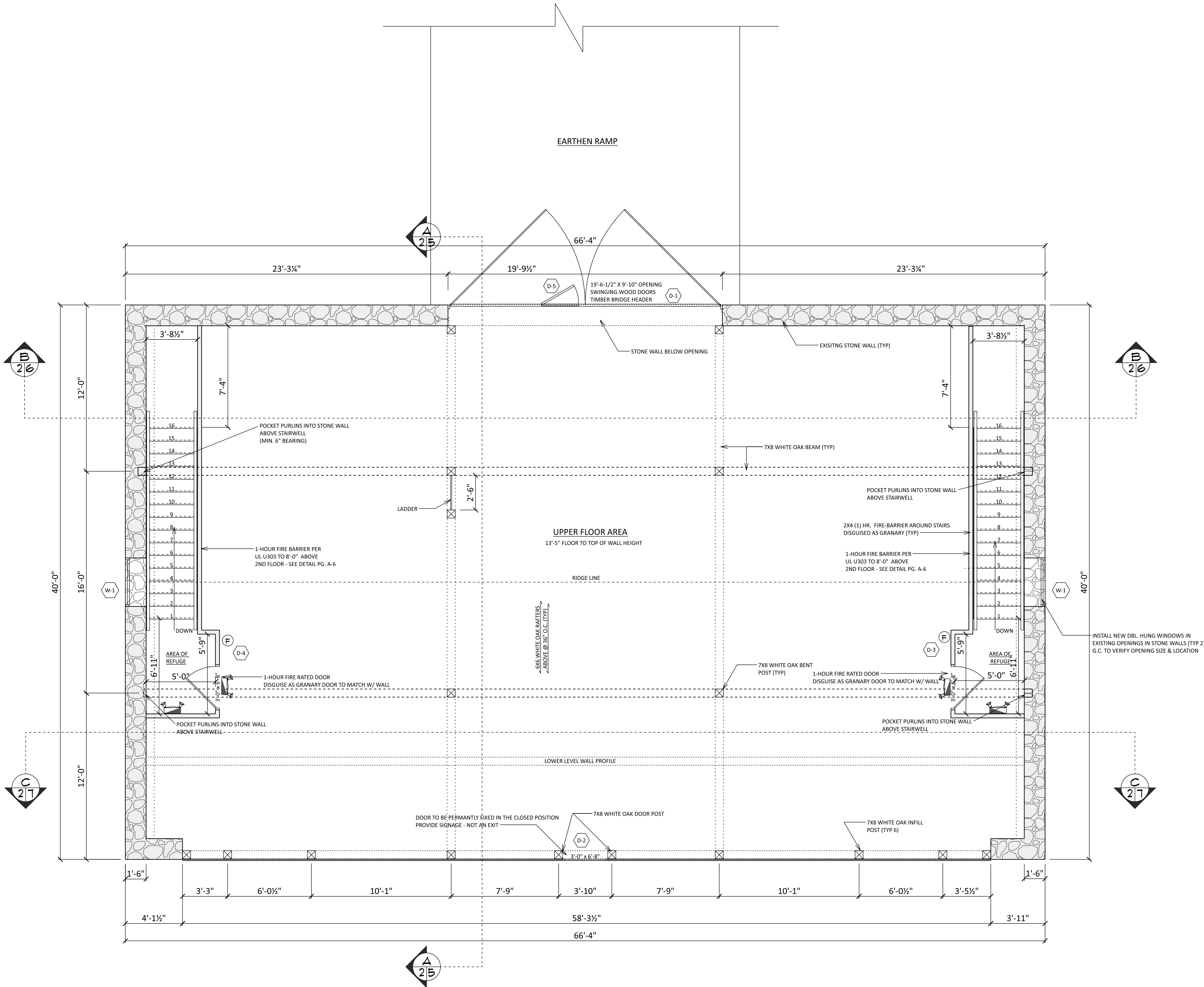
FIRE-RATED DOORS & FRAMES TO COMPLY W/ UL FM, OR WHI.

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ALL FIRE-RATED DOORS TO HAVE CLOSERS. (MOUNTED ON PUSH SIDE OF DOORS)



UPPER LEVEL PLAN
SCALE: 1/4" = 1'-0"

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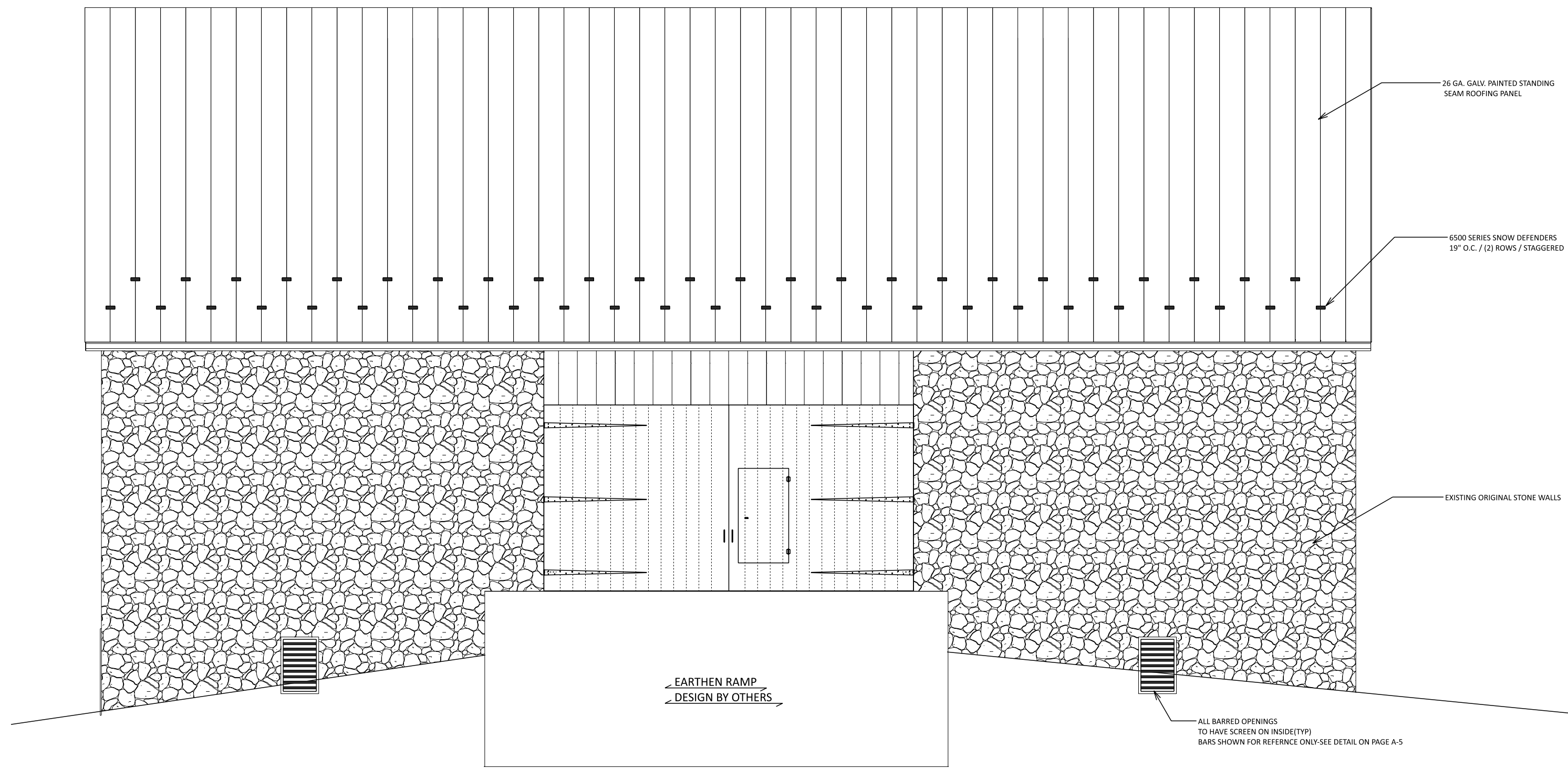
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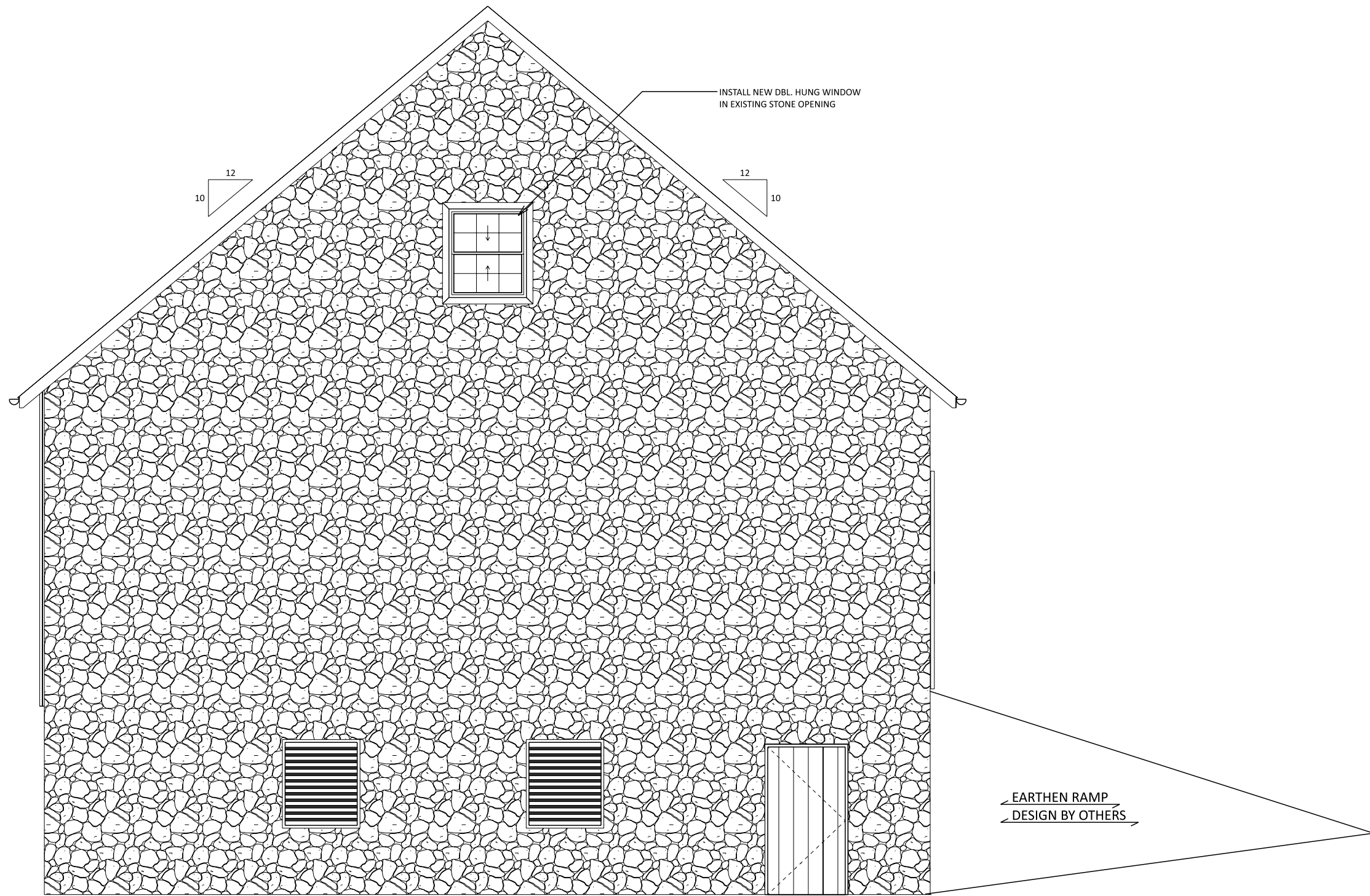
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UPPER LEVEL PLAN

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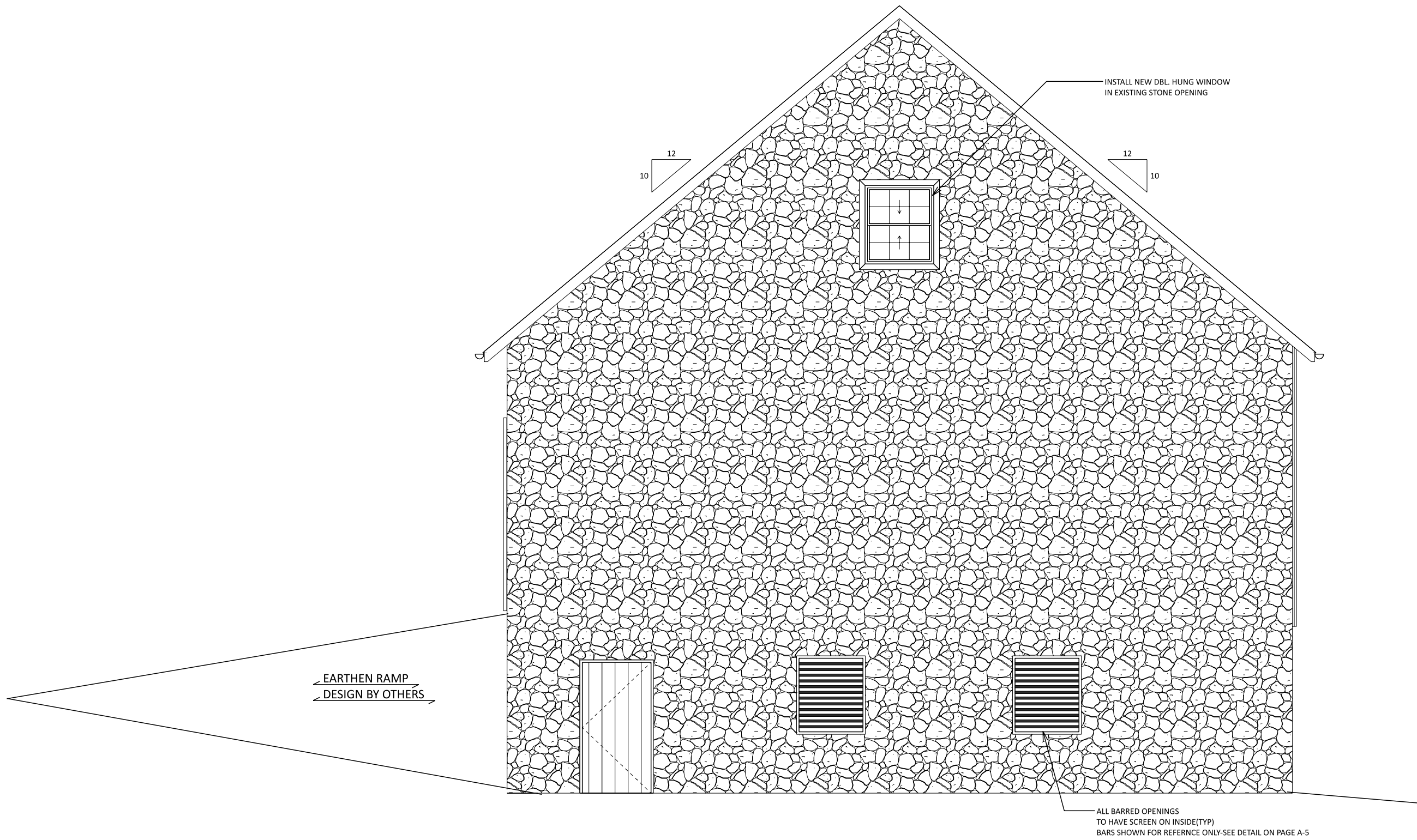
A-3



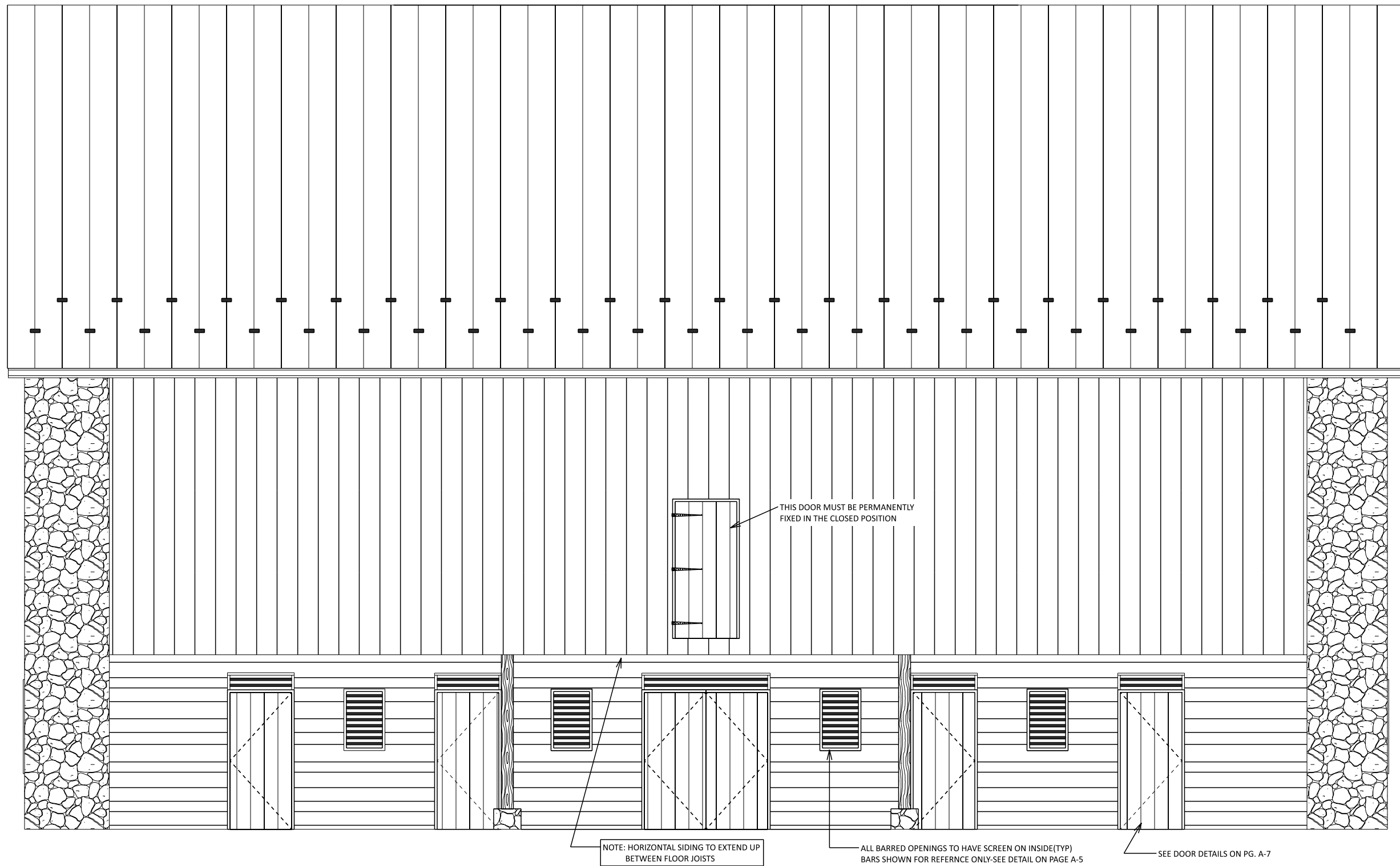
EAST ELEVATION
SCALE: 3/16" = 1'-0"



SOUTH ELEVATION
SCALE: 3/16" = 1'-0"



NORTH ELEVATION
SCALE: 3/16" = 1'-0"



WEST ELEVATION
SCALE: 3/16" = 1'-0"

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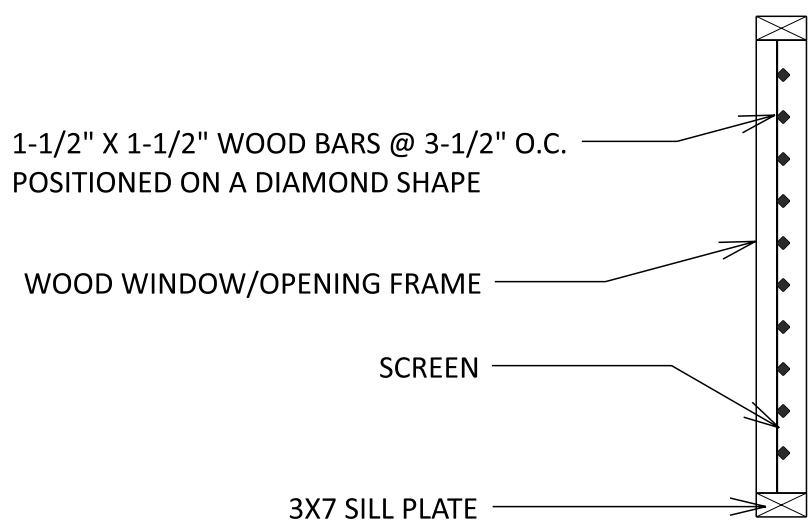
DATE:
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DRAWING TITLE:
ELEVATIONS

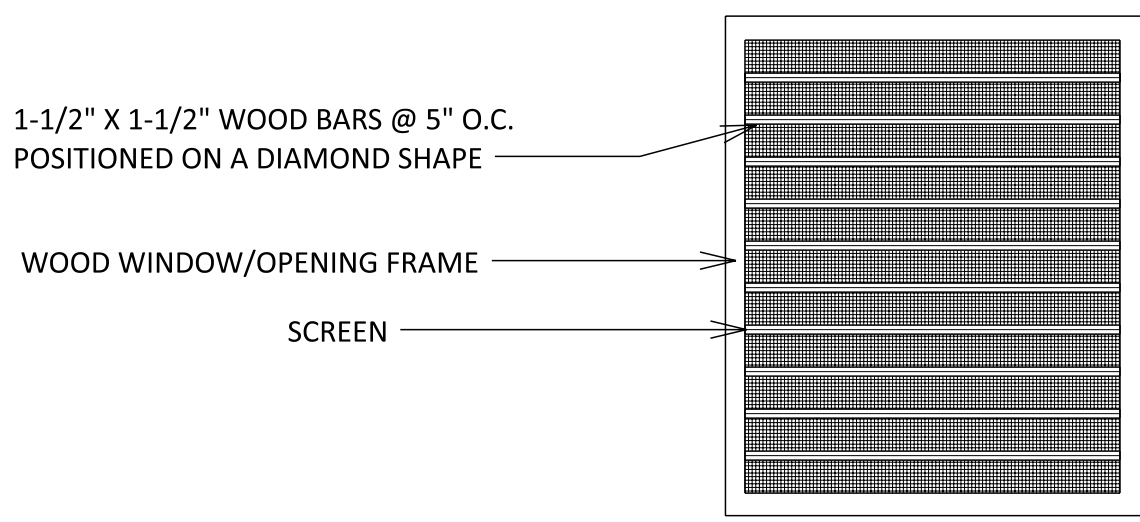
SHEET NO.

A-4

SIDE VIEW

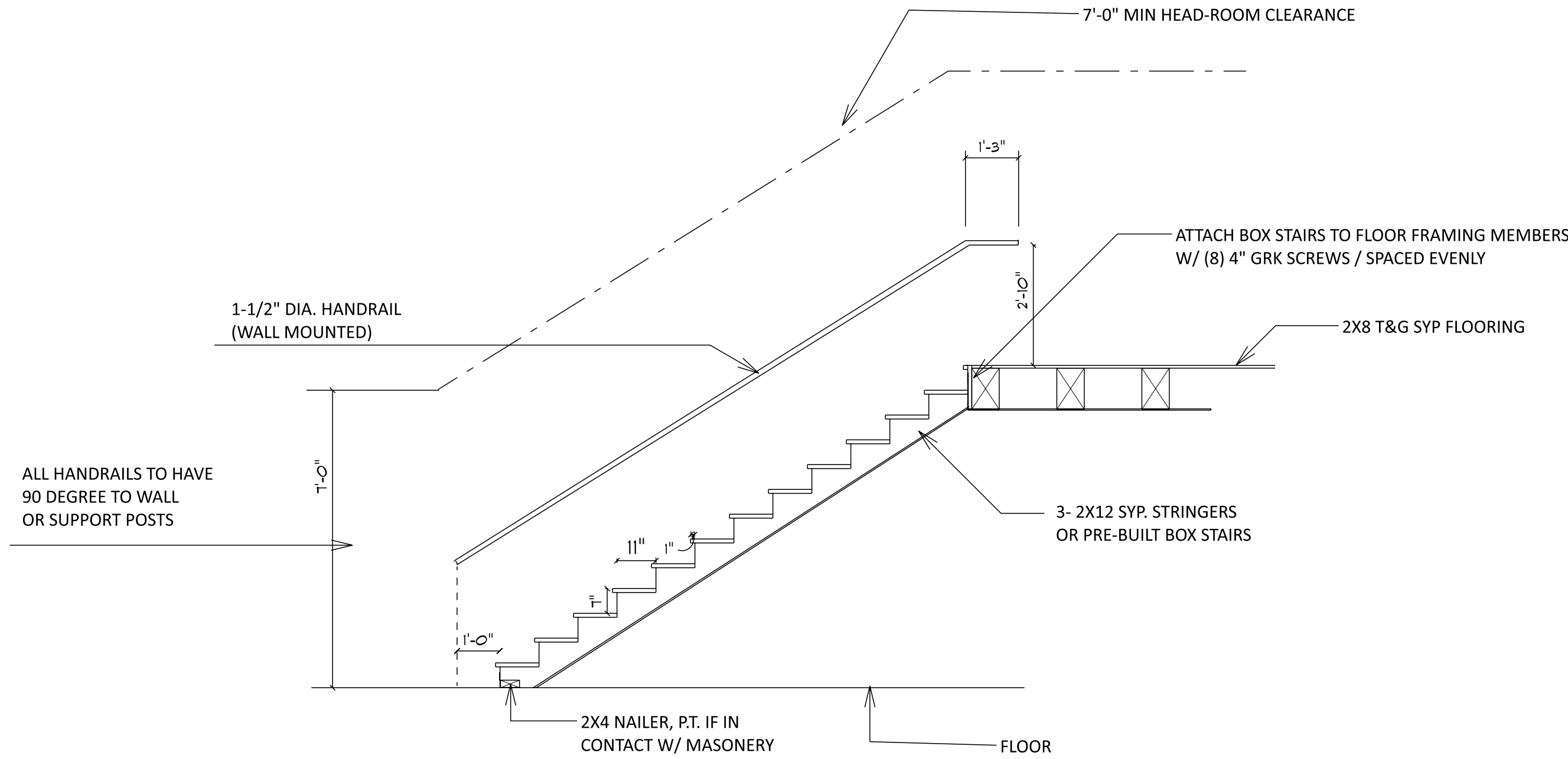


FRONT VIEW



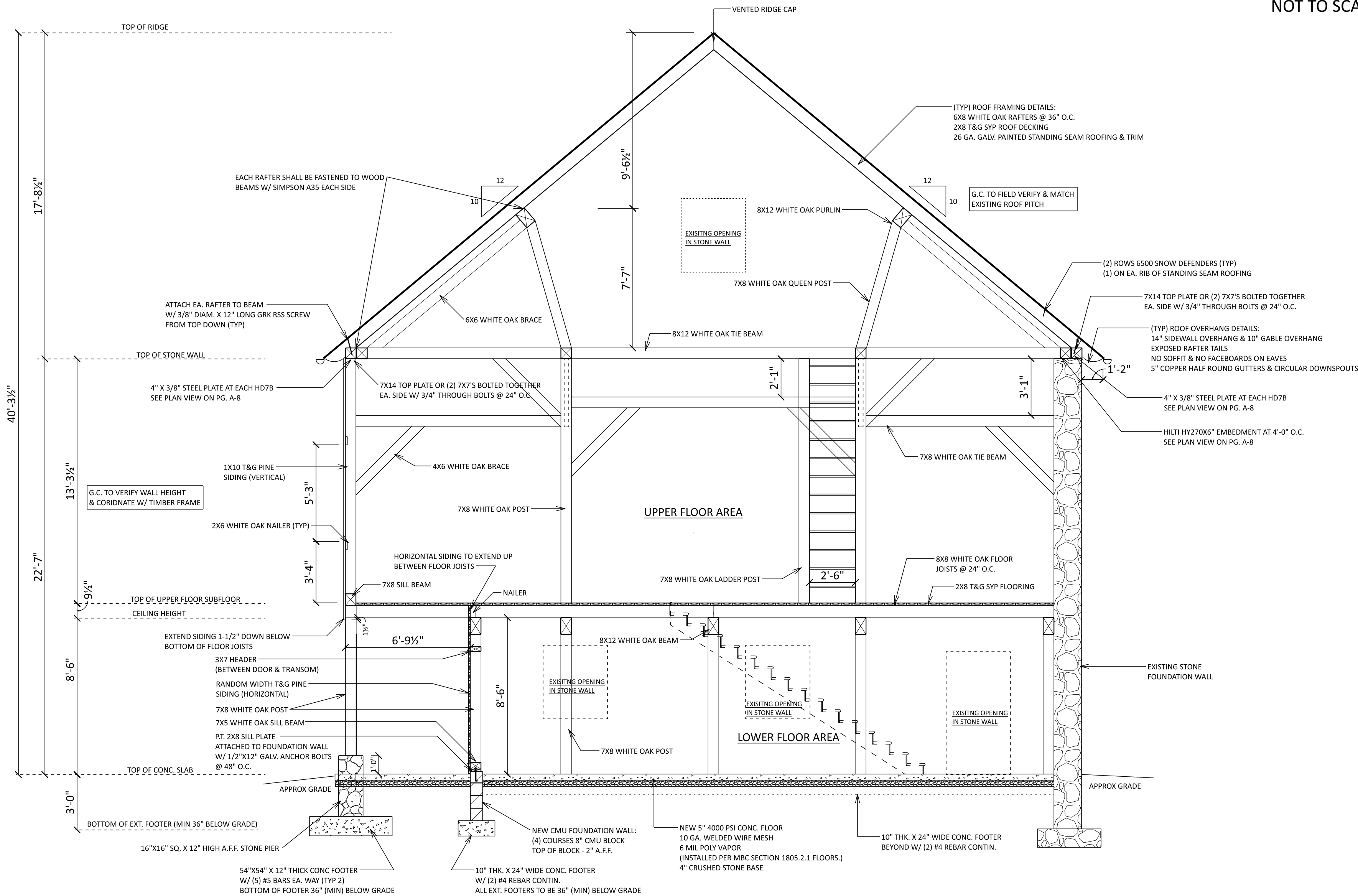
BARRED OPENING DTL.

NOT TO SCALE

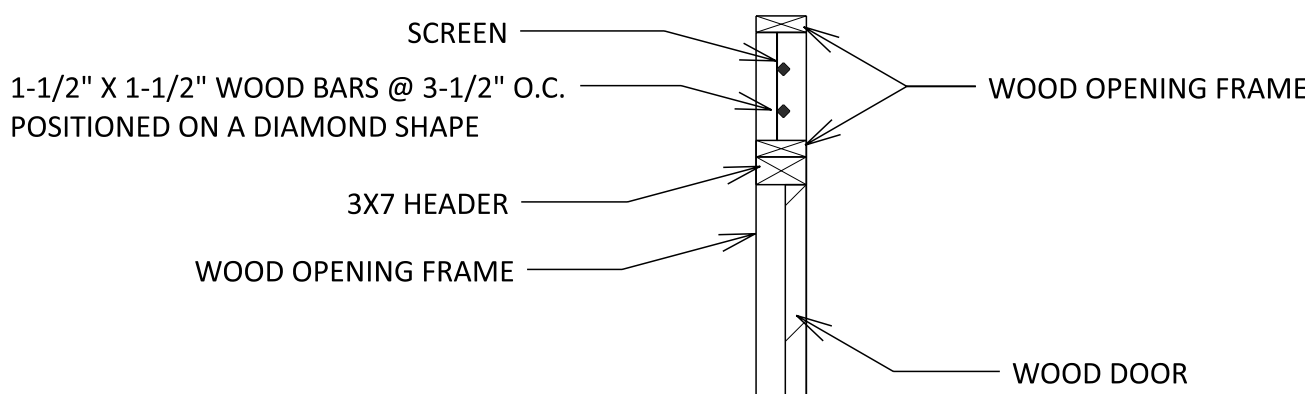


STAIR DETAILS

NOT TO SCALE

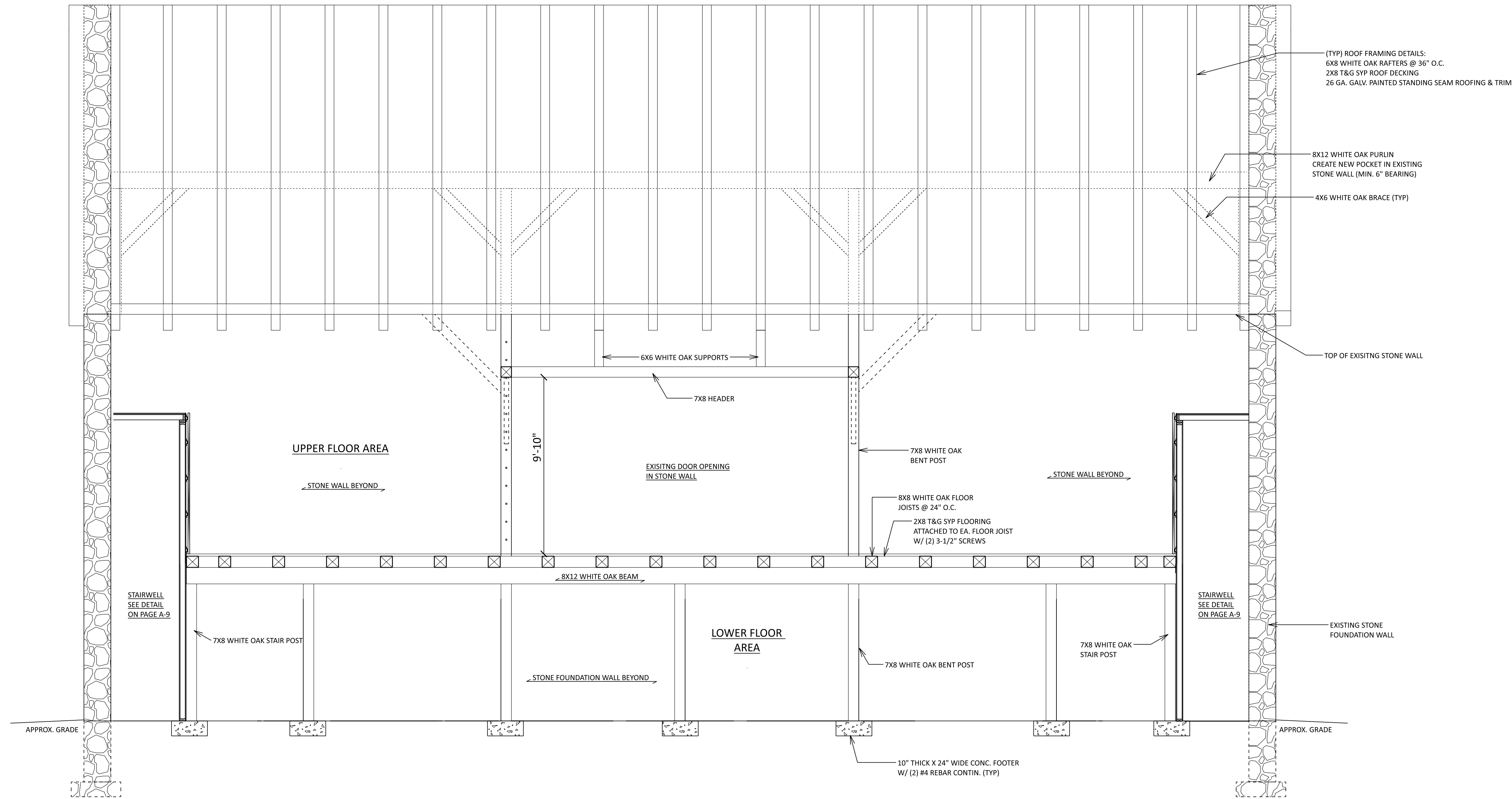


SIDE VIEW

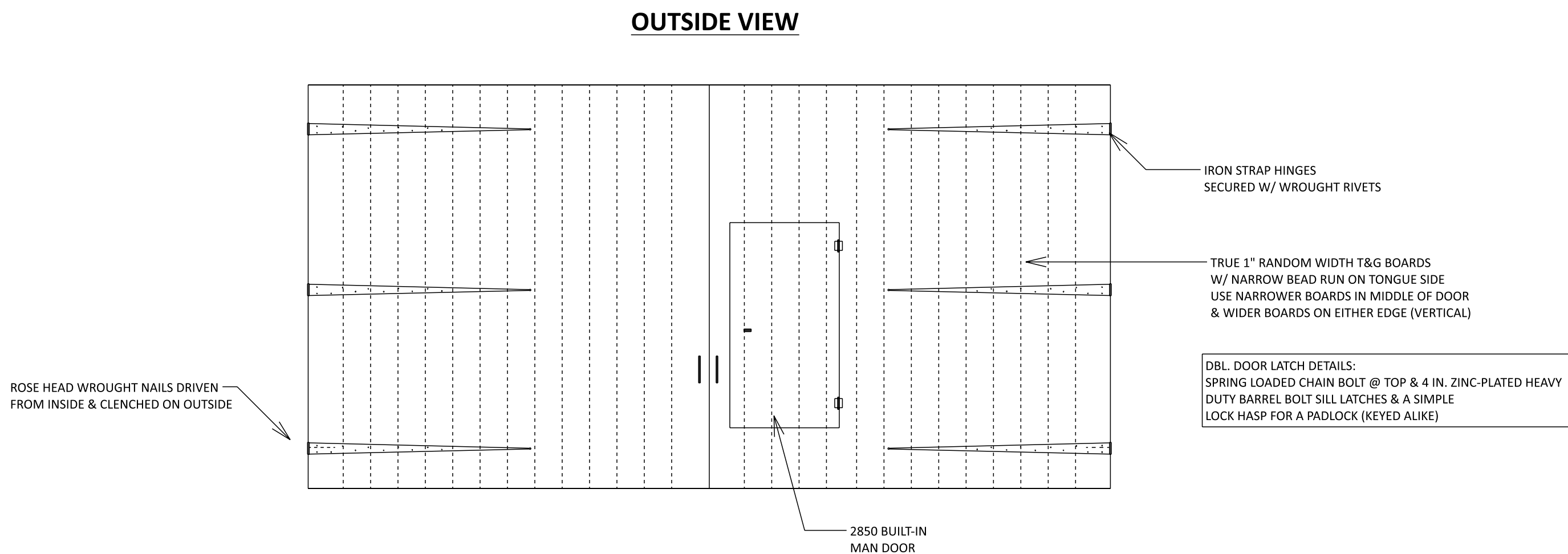


BARRED TRANSOM OPENING/DOOR DTL.

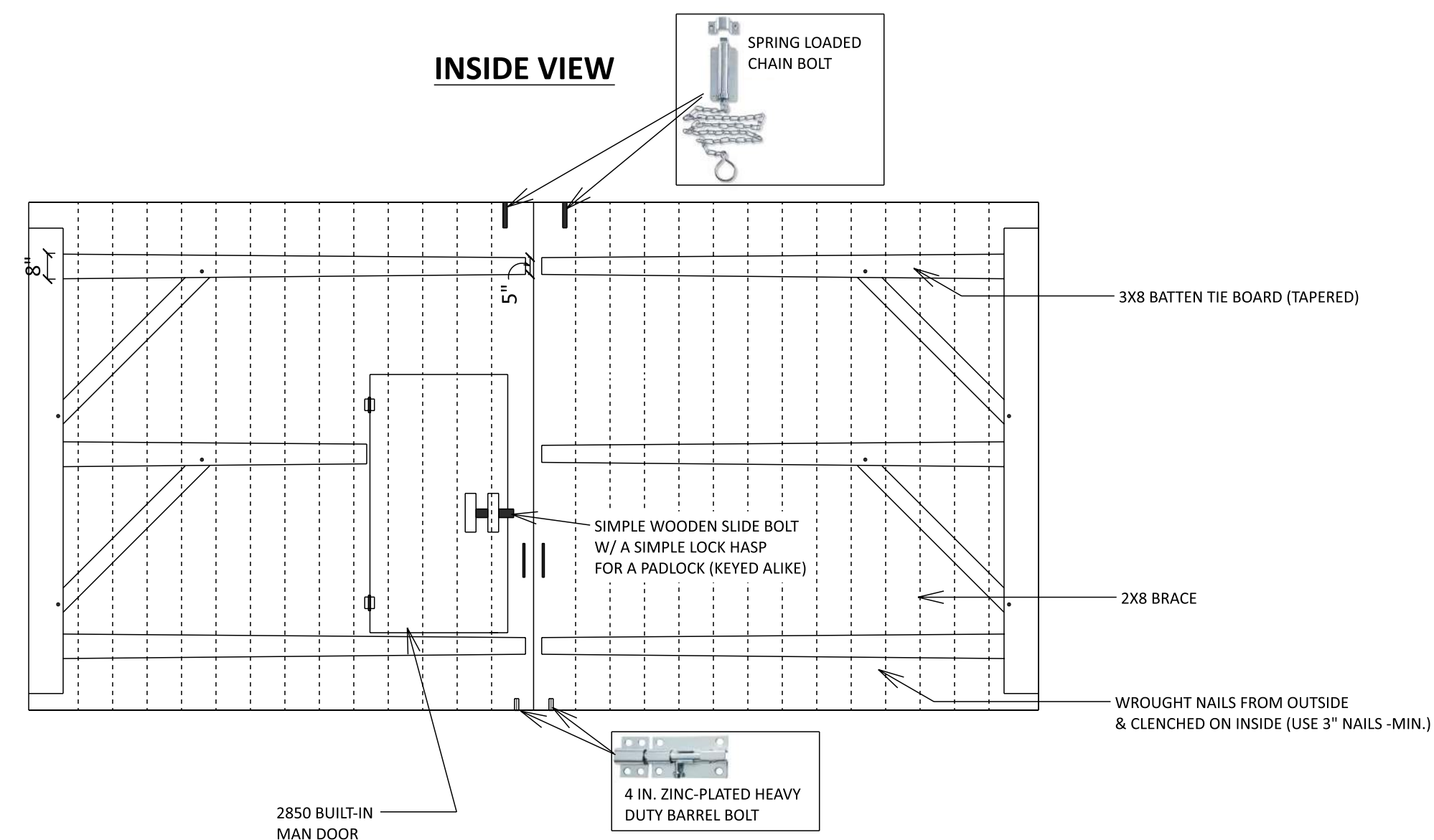
NOT TO SCALE



CROSS SECTION B
SCALE: 1/4" = 1'-0"



DBL. DOOR DETAILS -MAN DOOR
SCALE: 3/8" = 1'-0"



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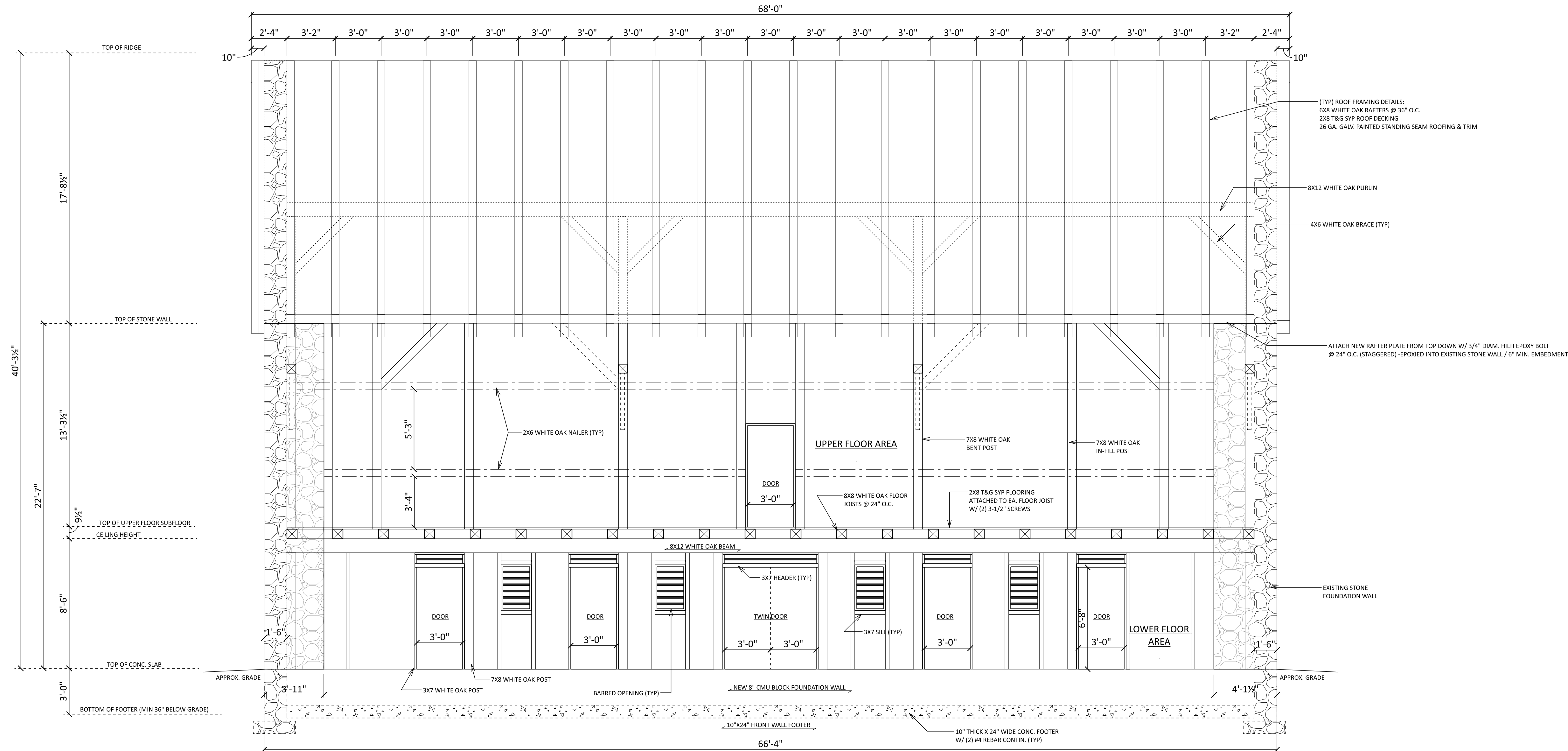
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CROSS SECTION B

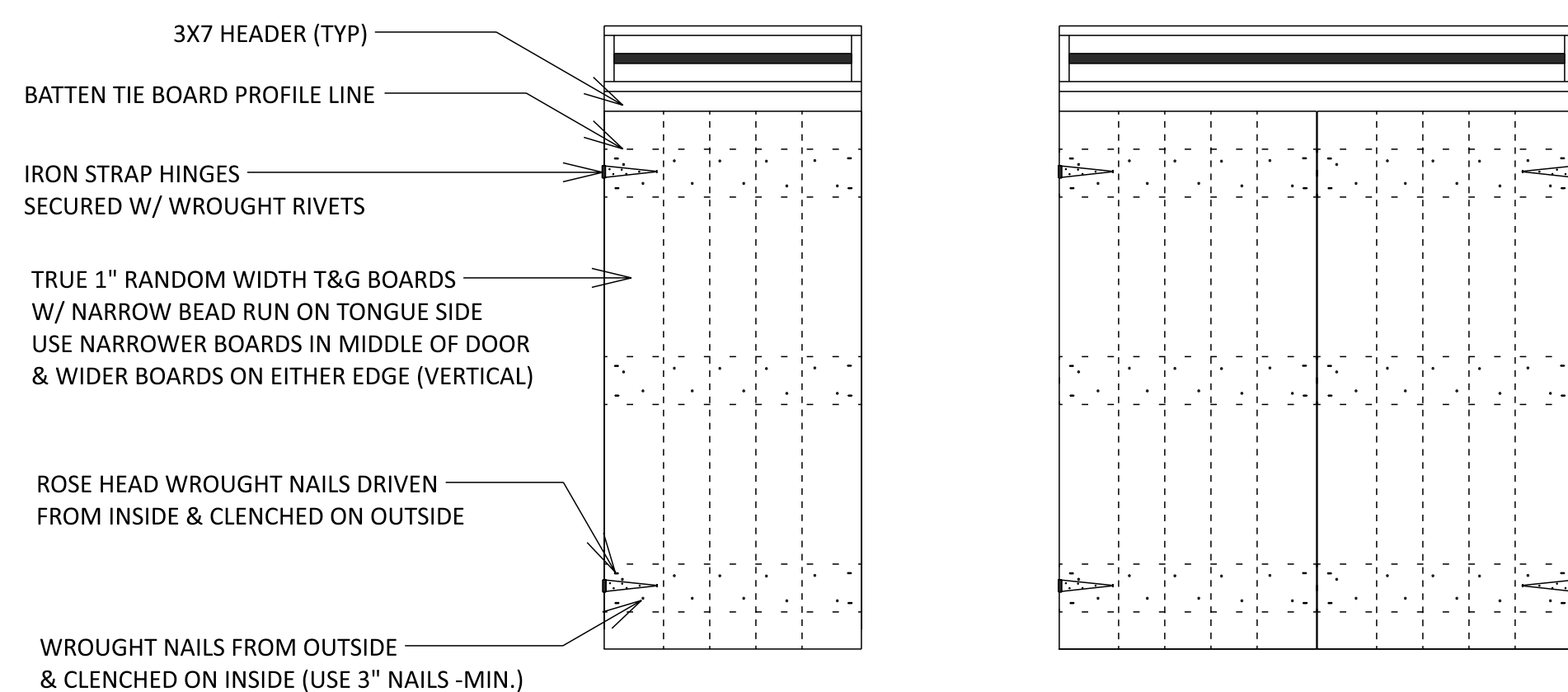
SHEET NO.

A-6



CROSS SECTION C
SCALE: 1/4" = 1'-0"

OUTSIDE VIEW



DOOR DETAILS
SCALE: 1/2" = 1'-0"

ALL OPENINGS WITH BARS TO HAVE SCREENS INSTALLED

OPENING FITTED W/ WOODEN BARS
INSTALLED SUCH THAT THEY
CREATE A DIAMOND SHAPE AT FRAME
SEE BARRED OPENING DETAIL ON PAGE A-5

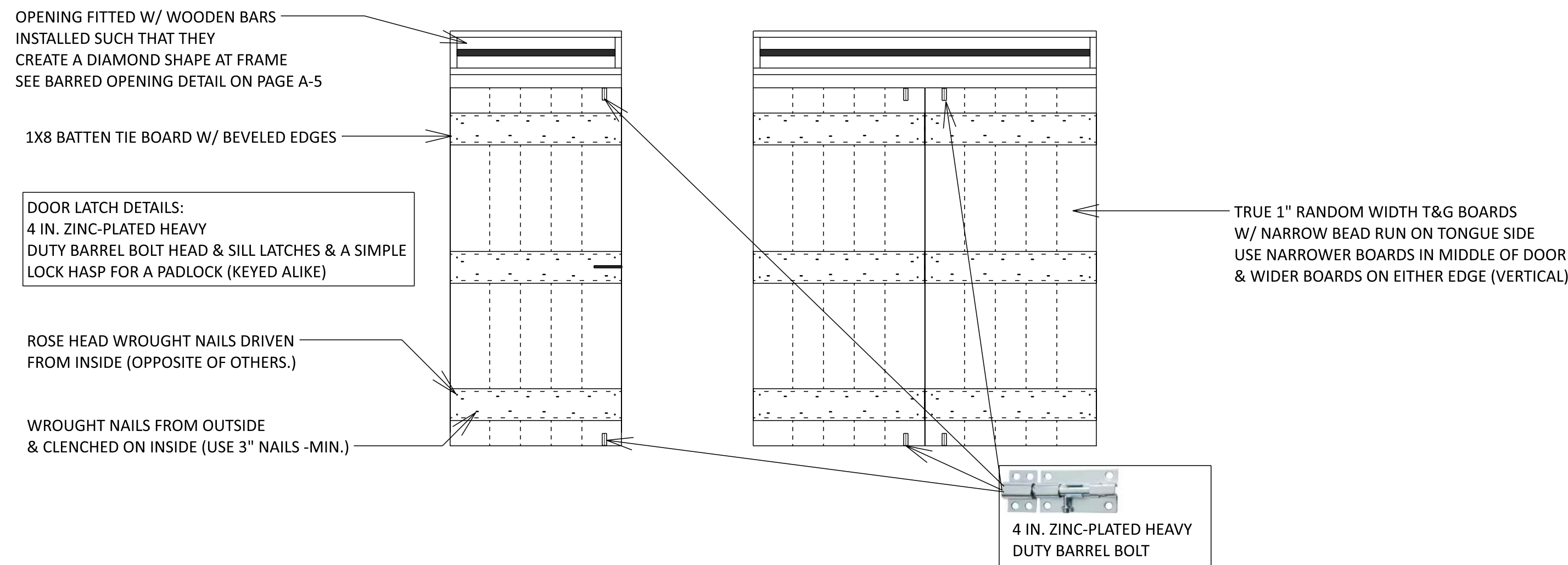
1X8 BATTEN TIE BOARD W/ BEVELED EDGES

DOOR LATCH DETAILS:
4 IN. ZINC-PLATED HEAVY
DUTY BARREL BOLT HEAD & SILL LATCHES & A SIMPLE
LOCK HASP FOR A PADLOCK (KEYED ALIKE)

ROSE HEAD WROUGHT NAILS DRIVEN
FROM INSIDE (OPPOSITE OF OTHERS.)

WROUGHT NAILS FROM OUTSIDE
& CLENCHED ON INSIDE (USE 3" NAILS - MIN.)

INSIDE VIEW



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DRAWING TITLE:
CROSS SECTION C

SHEET NO.

A-7

HDB/HD

Holdowns

Simpson Strong-Tie offers a wide variety of bolted holdowns offering low-deflection performance for a range of load requirements.

The HD3B is a light-duty holdown designed for use in shearwalls and braced-wall panels, as well as other lateral applications.

The HD5B, HD7B and HD9B bolted holdowns incorporate the proven design of our HDQ6 SDS-style holdown and feature a unique seat design which greatly minimizes deflection under load. HDB holdowns are self-jigging, ensuring that the code-required minimum of seven bolt diameters from the end of the post is met. They can be installed directly on the sill plate or raised above it and are suitable for back-to-back applications where eccentricity is a concern. HDBs are designed to provide loads for intermediate-load-range shearwalls, braced-wall panels and lateral applications.

HD holdowns offer high allowable loads for both vertical and horizontal applications. The HD12 and HD19 are self-jigging, ensuring that the code-required minimum of seven bolt diameters from the end of the post is met. They can be installed back-to-back when eccentricity is an issue.

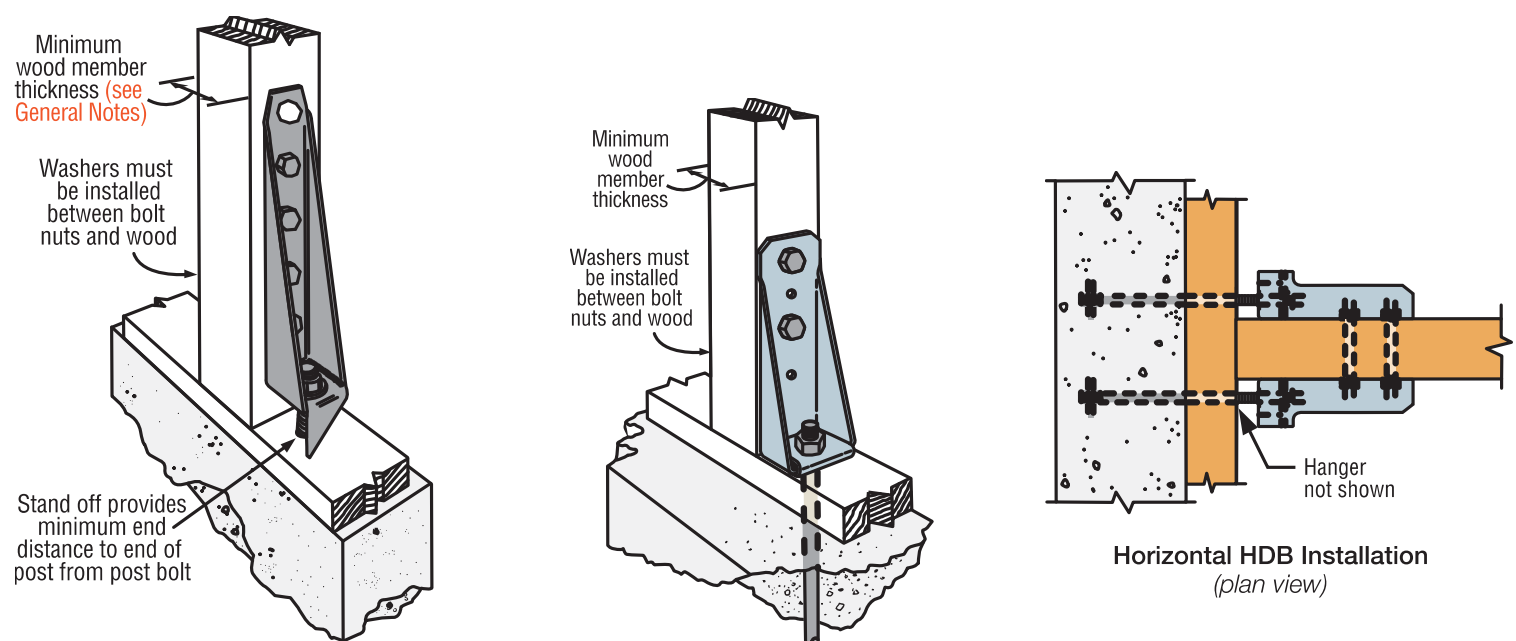
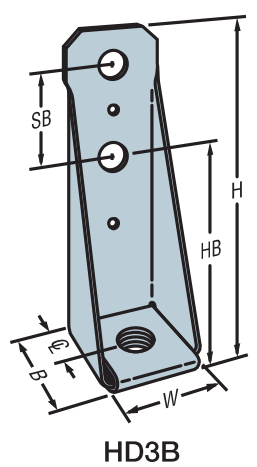
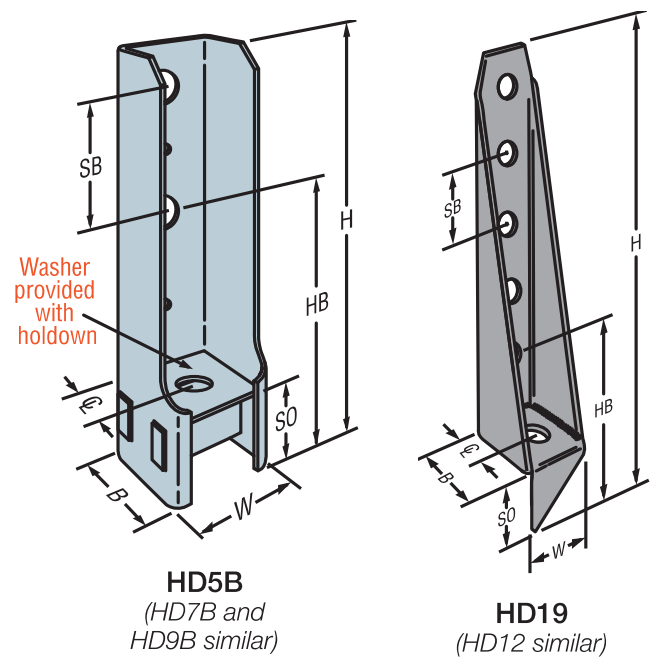
Material: See table

Finish: HD3B/HD5B/HD7B/HD9B – Galvanized;
HD – Simpson Strong-Tie gray paint; HDG available.
For stainless steel options, see L-C-SSHD at [strongtie.com](https://www.strongtie.com).

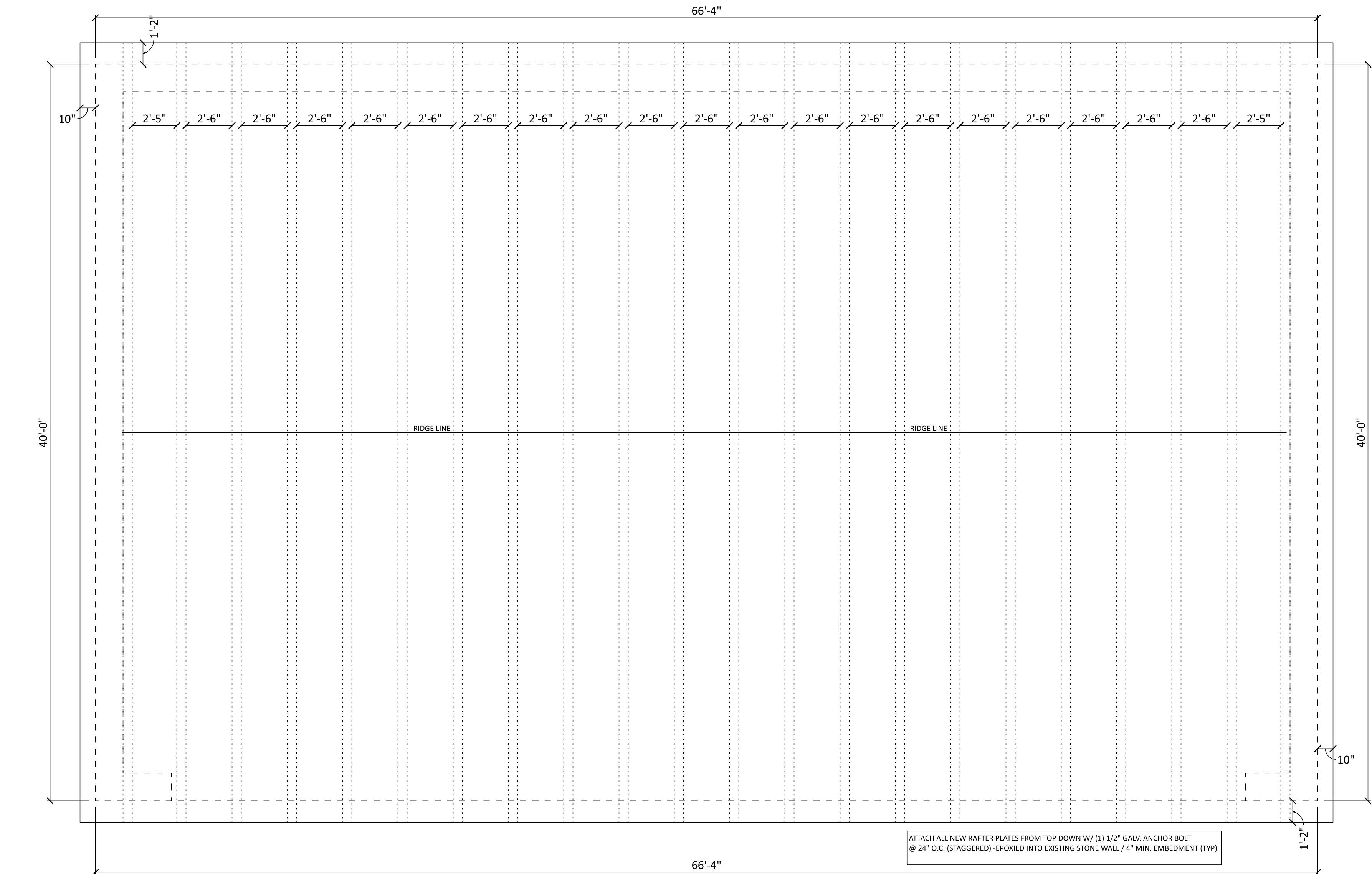
Installation:

- See Holdown and Tension Tie General Notes on pp. 49–50
- Bolt holes shall be a minimum of 1/8" to a maximum of 1/4" larger than the bolt diameter (per 2015/2018 NDS, section 12.1.3.2)
- Stud bolts should be snugly tightened with standard cut washers between the wood and nut (BPs are required in the City and County of Los Angeles)
- HD and HDB holdowns are self-jigging and will ensure minimum bolt end distance when installed flush with the sill plate
- Standard cut washer is required under the anchor nut for HD12 with 1" anchor and HD19 with 1 1/2" anchors

Codes: See p. 12 for Code Reference Key Chart



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ROOF FRAMING PLAN

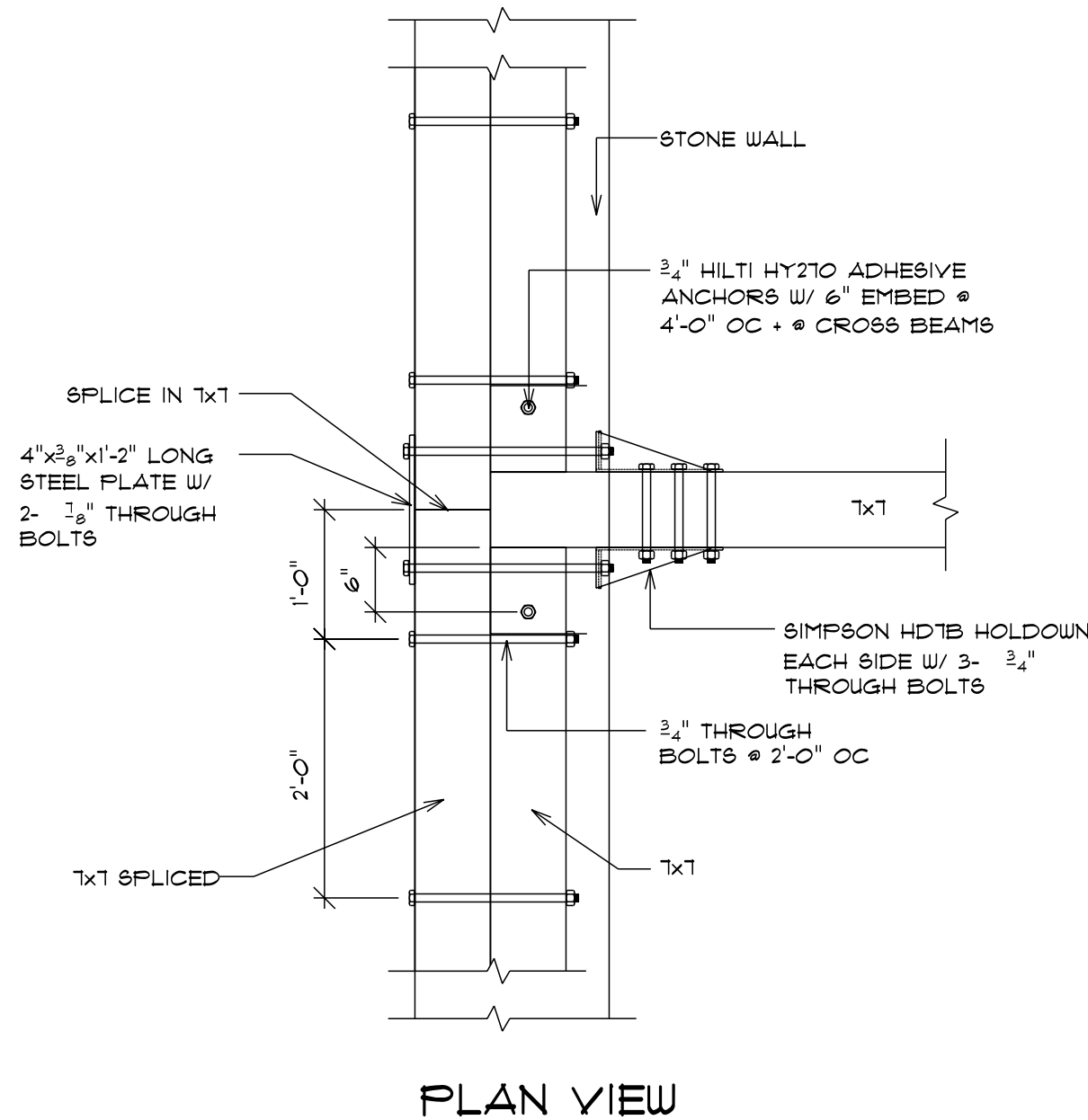
SCALE: 1/4" = 1'-0"

HDB/HD

Holdowns (cont.)

These products are available with additional corrosion protection. For more information, see p. 15.

Model No.	Material	Dimensions (in.)								Fasteners (in.)		Minimum Wood Member Size (in.)	Allowable Tension Loads (160)		Deflection at Highest Allowable Load	Code Ref.
		Base (in.)	Body (ga.)	HB	SB	W	H	B	CL	S0	Anchor Dia. Bolt	Stud Bolts				
													DF/SP	SPF/HF		
HD3B	—	12	4%	2 1/2	2 1/2	8 3/4	2 1/4	1 5/8	3/4	3/4	%	(2) 3/4	1 1/2 x 3 1/2	1,895	1,610	0.156
													2 1/2 x 3 1/2	2,525	2,145	0.169
													3 x 3 1/2	3,130	3,050	0.12
													3 1/2 x 3 1/2	3,130	3,050	0.12
HD5B	3/8	10	5 1/4	3	2 1/2	9 3/4	2 1/2	1 1/4	2	%	%	(2) 3/4	1 1/2 x 3 1/2	2,405	2,070	0.153
													2 1/2 x 3 1/2	3,750	3,190	0.129
													3 x 3 1/2	4,505	3,785	0.156
													3 1/2 x 3 1/2	4,935	4,195	0.15
HD7B	3/8	10	5 1/4	3	2 1/2	12 3/4	2 1/2	1 1/4	2	%	%	(3) 3/4	3 x 3 1/2	6,645	5,650	0.142
													3 1/2 x 3 1/2	7,310	6,215	0.154
													3 1/2 x 4 1/2	7,345	6,245	0.155
													3 1/2 x 3 1/2	7,740	6,580	0.159



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A BANK BARN RESTORATION FOR:

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2813 JERUSALEM RD.
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PROJECT # 8141

DRAWN BY:
I.L. ZOOK

REVISIONS:
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I.L. ZOOK - 4.2.2025
I.L. ZOOK - 5.17.2025
I.L. ZOOK - 8.8.2025
I.L. ZOOK - 8.29.2025
I.L. ZOOK - 9.25.2025
I.L. ZOOK - 11.1.2025

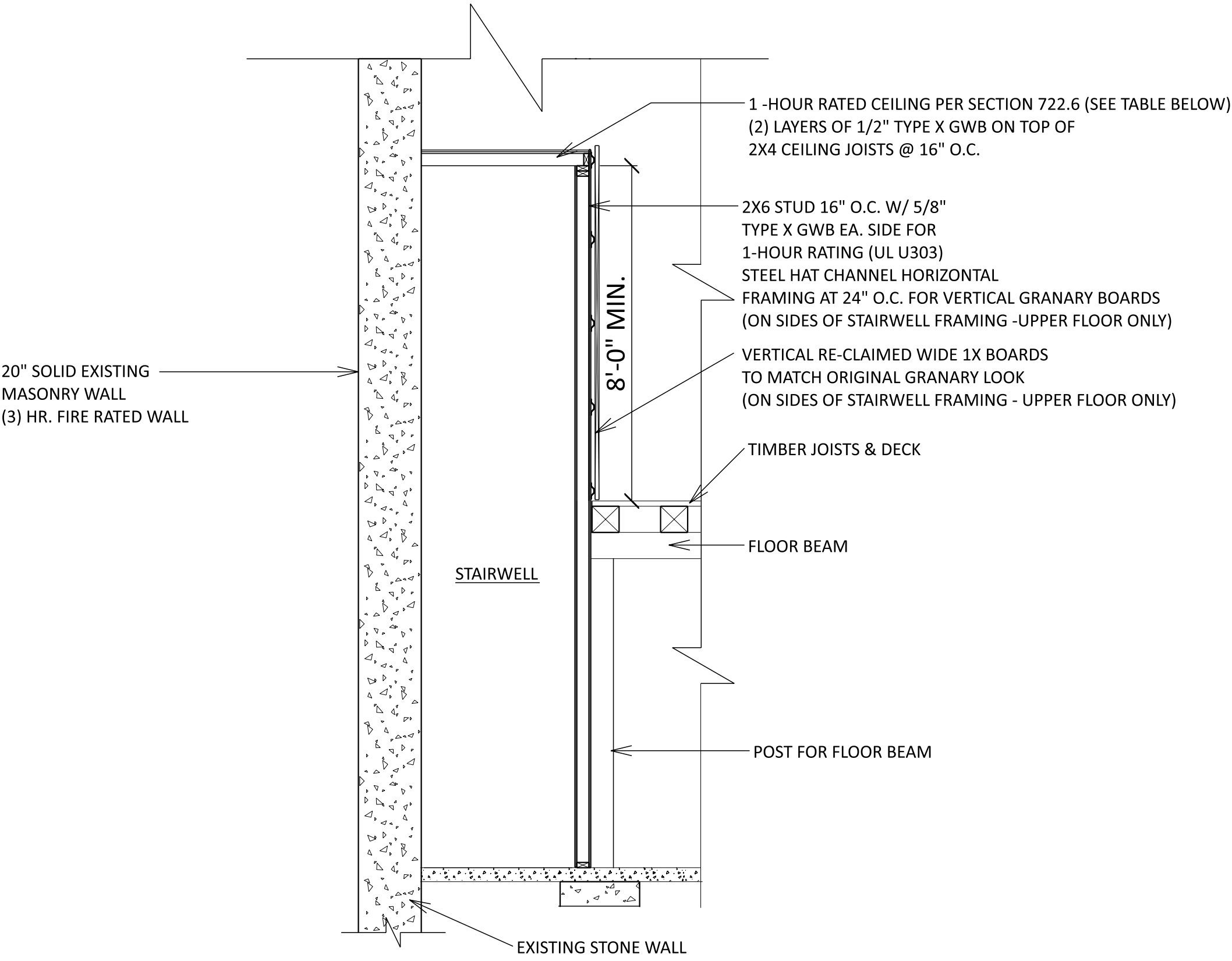
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ROOF FRAMING PLAN

SHEET NO.

A-8



UL 303 FIRE BARRIER DETAIL
SCALE: 3/8" = 1'-0"

722.6.2 Walls, floors and roofs. E

These procedures apply to both load-bearing and nonload-bearing assemblies.

TABLE 722.6.2(1)TIME ASSIGNED TO WALLBOARD MEMBRANES^{a, b, c, d}

DESCRIPTION OF FINISH	TIME ^a (minutes)
³ / ₈ -inch wood structural panel bonded with exterior glue	5
¹ / ₃₂ -inch wood structural panel bonded with exterior glue	10
¹ / ₃₂ -inch wood structural panel bonded with exterior glue	15
³ / ₈ -inch gypsum wallboard	10
¹ / ₂ -inch gypsum wallboard	15
⁵ / ₈ -inch gypsum wallboard	30
¹ / ₂ -inch Type X gypsum wallboard	25
⁵ / ₈ -inch Type X gypsum wallboard	40
Double ³ / ₈ -inch gypsum wallboard	25
¹ / ₂ -inch + ³ / ₈ -inch gypsum wallboard	35
Double ¹ / ₂ -inch gypsum wallboard	40

For St: 1 inch = 25.4 mm.

a. These values apply only where membranes are installed on framing members that are spaced 16 inches o.c. or less.

b. Gypsum wallboard installed over framing or furring shall be installed so that all edges are supported, except ³/₈-inch Type X gypsum wallboard shall be permitted to be installed horizontally with the horizontal joints staggered 24 inches each side and unsupported but finished.

c. On wood frame floor/ceiling or roof/ceiling assemblies, gypsum board shall be installed with the long dimension perpendicular to framing members and shall have all joints finished.

d. The membrane on the unexposed side shall not be included in determining the fire resistance of the assembly. Where dissimilar membranes are used on a wall assembly, the calculation shall be made from the least fire-resistant (weaker) side.

e. The time assigned is not a finished rating.

TABLE 722.6.2(2)TIME ASSIGNED FOR CONTRIBUTION OF WOOD FRAME ^{a, b, c}

DESCRIPTION	TIME ASSIGNED TO FRAME (minutes)
Wood studs 16 inches o.c.	20
Wood floor and roof joists 16 inches o.c.	10

For St: 1 inch = 25.4 mm.

a. This table does not apply to studs or joists spaced more than 16 inches o.c.

b. All studs shall be nominal 2 x 4 and all joists shall have a nominal thickness of not less than 2 inches.

c. Allowable spans for joists shall be determined in accordance with Sections 2308.4.2.1, 2308.7.1 and 2308.7.2.

1/28/13 BXUV.U303 - Fire Resistance Ratings - ANSI/UL 263



Design No. U303
BXUV.U303
Fire Resistance Ratings - ANSI/UL 263

Page Bottom

Design/System/Construction/Assembly Usage Disclaimer

- Authorities Having Jurisdiction should be consulted in all cases as to the particular requirements covering the installation and use of UL Listed or Classified products, equipment, system, devices, and materials.
- Authorities Having Jurisdiction should be consulted before construction.
- Fire resistance assemblies and products are developed by the design submitter and have been investigated by UL for compliance with applicable requirements. The published information cannot always address every construction nuance encountered in the field.
- When field issues arise, it is recommended the first contact for assistance be the technical service staff provided by the product manufacturer noted for the design. Users of fire resistance assemblies are advised to consult the general Guide Information for each product category and each group of assemblies. The Guide Information includes specifics concerning alternate materials and alternate methods of construction.
- Only products which bear UL's Mark are considered as Classified, Listed, or Recognized.

Fire Resistance Ratings - ANSI/UL 263

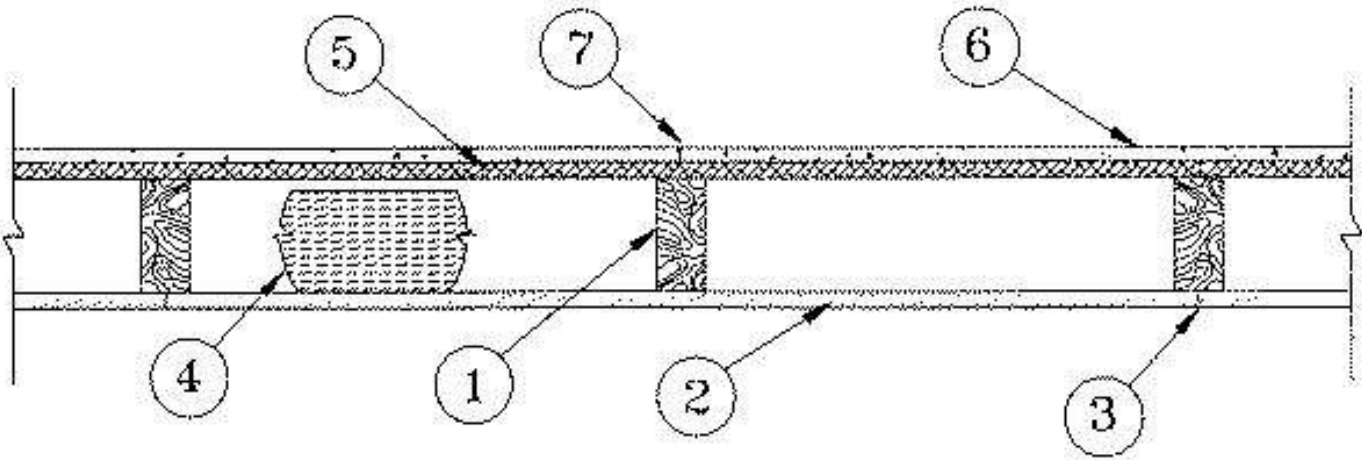
See General Information for Fire Resistance Ratings - ANSI/UL 263

Design No. U303

December 08, 2011

Bearing Wall Rating — 1 Hr

Load Restricted for Canadian Applications — See Guide BXUV7



1. **Wood Studs** — Nom 2 by 4 in. spaced 16 in. OC, effectively cross-braced.
2. **Gypsum Board*** — 5/8 in. thick, with square or tapered edges, applied vertically or horizontally with vertical joints centered over studs. Horizontal joints need not be backed by framing. Fastened to studs and plates with 1-7/8 in. long 6d cement coated nails spaced 7 in. OC or with 1-7/8 in. long Type S screws spaced 8 in. OC, or 1-1/4 in. long Type W coarse thread gypsum panel steel screws spaced a max 8 in. OC, with last screw 1 in. from edge of board. 54 in. Wdths applied horizontally.

CGC INC — Types AR, C, IP-AR, IP-X1, IP-X2, IPC-AR, SCX, ULX,WRC, WRX.

UNITED STATES GYPSUM CO — Types AR, C, FRX-G, IP-AR, IP-X1, IP-X2, IPC-AR, SCX, ULX, WRC or WRX.

1/28/13 BXUV.U303 - Fire Resistance Ratings - ANSI/UL 263
USG MEXICO S A D E C V — Types AR, C, IP-AR, IP-X1, IP-X2, IPC-AR, SCX, ULX, WRC or WRX.

3. **Joints** — When tapered edge gypsum board is used, joints covered with joint compound and paper tape. As an alternate, gypsum veneer plaster may be applied to the entire surface of Classified veneer baseboard with joints reinforced with paper tape. When square-edge gypsum board is used, treatment of joints is optional.

4. **Batts and Blankets*** — Min 3 in. thick mineral wool batts, friction fit between studs.
THERMAFIBER INC — Type SAFB

5. **Sheathing** — Min 15/32 in. thick, 4 ft wide, wood structural panels, min grade "sheathing" applied vertically, with vertical joints centered over studs. Attached to studs with 10d galv nails 6 in. OC at the perimeter and 12 in. OC in the field. Sheathing fully covered with a weather resistive barrier.

6. **Cementitious Backer Units*** — 1/2 or 5/8 in. thick, installed vertically or horizontally over the sheathing with vertical joints centered over studs. All joints offset min 12 in. from underlying sheathing joints. Fastened to studs and plates with corrosion resistant 2-1/4 in. long chamfered, ribbed wefer head screws with a minimum head diameter of .400 inches or 2-1/4 in. hot-dipped galvanized roofing nails spaced 8 in. OC.

UNITED STATES GYPSUM CO — Type DCB, DUROCK Exterior Cement Board or DUROCK Brand Cement Board.

USG MEXICO S A D E C V — Type DCB.

7. **Joints** — Cement board joints need not be treated.
8. **Vapor Retarder , Water Barrier or Weather Resistive Barrier** — (Optional, not shown) — As required

*Bearing the UL Classification Mark

Last Updated on 2011-12-08

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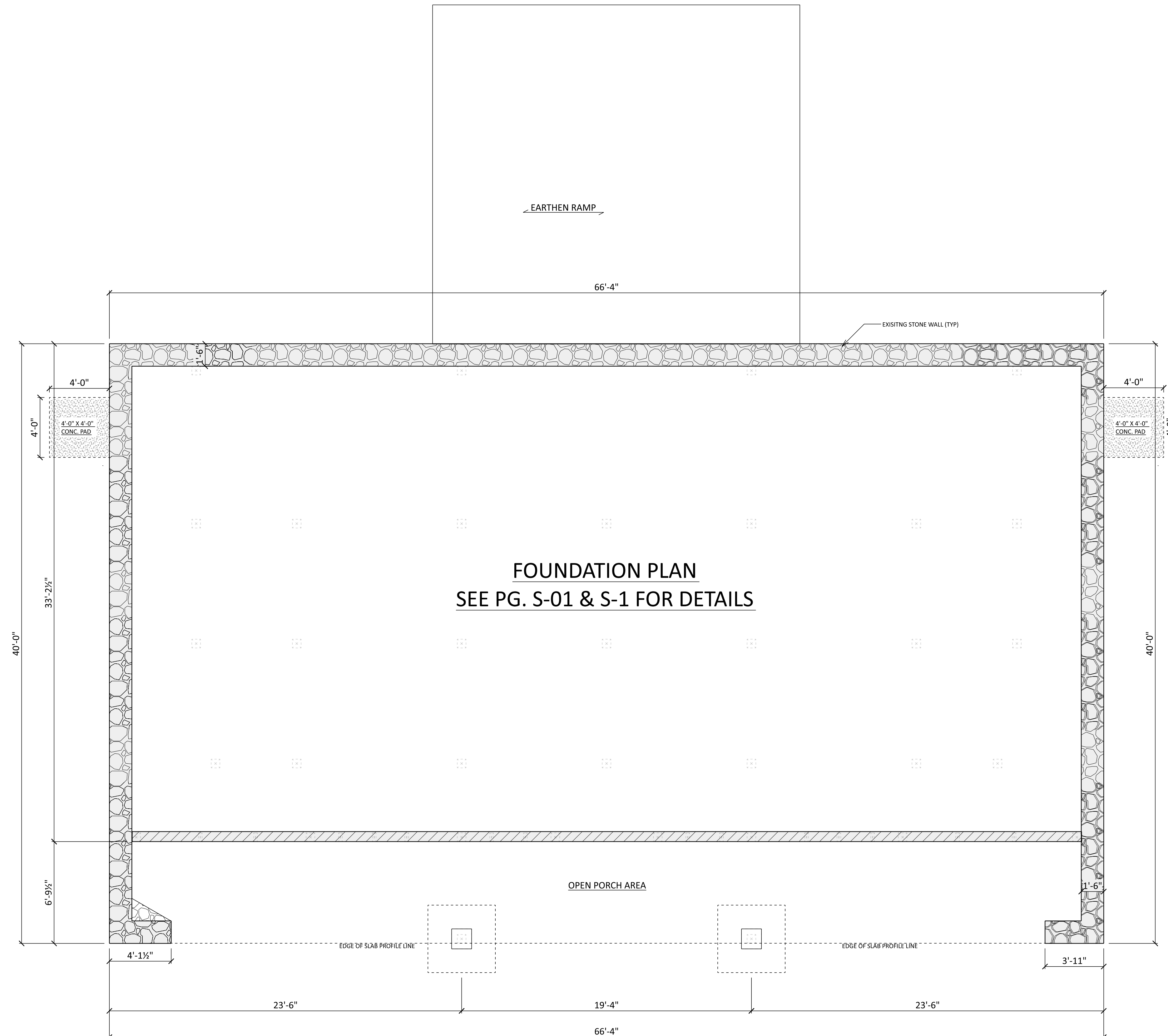
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SHEET NO.

A-9



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FOUNDATION PLAN

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A-10



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3D RENDERINGS

SHEET NO.

A-11

General Notes

1.0	GENERAL
A. THE STANDARD GENERAL CONDITIONS FOR THE CONSTRUCTION CONTRACT N.S.P.E. DOCUMENT 1910-8 SHALL GOVERN THIS WORK AS IF ENTIRELY INCLUDED ON THESE DRAWINGS	
B. DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE OBTAINED FROM THE ARCHITECTURAL DRAWINGS AVAILABLE PRIOR TO RELEASE OF CONTRACT DOCUMENTS. ARCHITECTURAL DIMENSIONS WILL GOVERN OVER STRUCTURAL DIMENSIONS. LAYOUT OF BUILDING FOUNDATIONS OR OTHER ITEMS SHALL BE BASED ON THE ARCHITECTURAL, CIVIL AND STRUCTURAL DRAWINGS. ERRORS, INCONSISTENCIES IN DIMENSIONS SHALL BE FORWARDED TO ARCHITECT FOR RESOLUTION.	
C. THE BASIC STABILITY OF THE STRUCTURE IS DEPENDENT UPON THE DIAPHRAGM ACTION OF FLOORS, WALLS, AND ROOF ACTING TOGETHER. PROVIDE GUYS, BRACES, STRUTS, ETC., TO ACCOMMODATE LIVE, DEAD, AND WIND LOADS UNTIL FINAL CONNECTIONS BETWEEN THESE ELEMENTS ARE MADE.	
D. CANTILEVER AND BASEMENT RETAINING WALLS HAVE NOT BEEN DESIGNED FOR SURCHARGE LOADING ASSOCIATED WITH CONSTRUCTION TRAFFIC BEHIND THE WALL. THE CONTRACTOR AND HIS SUBS SHALL PROVIDE ADEQUATE TEMPORARY BRACING TO RESIST INCREASED LATERAL LOADS ON THE WALLS ASSOCIATED WITH THEIR MEANS AND METHODS OF CONSTRUCTION.	
1.1	DESIGN LOADS
A. THE STRUCTURE WAS DESIGNED FOR THE LIVE LOADS SHOWN BELOW AND DEAD LOADS AS REQUIRED BY CONSTRUCTION IN ACCORDANCE WITH IBC 2021. LOADS DUE TO SNOW LOAD BUILD-UP WERE CONSIDERED IN DESIGN OF STRUCTURAL COMPONENTS ADJACENT TO PARAPETS, HIGH BUILDING WALLS, ETC. INCREASE IN THESE LOADINGS, DUE TO CHANGE IN FUNCTION, CONSTRUCTION MATERIALS, ETC., TO HAVE WRITTEN APPROVAL FROM THE DESIGNING STRUCTURAL ENGINEER.	
B. THE BASIC STABILITY OF THE STRUCTURE IS DEPENDENT UPON THE DIAPHRAGM ACTION OF FLOORS, WALLS, AND ROOF ACTING TOGETHER. PROVIDE GUYS, BRACES, STRUTS, ETC., TO ACCOMMODATE LIVE, DEAD, AND WIND LOADS UNTIL FINAL CONNECTIONS BETWEEN THESE ELEMENTS ARE MADE.	
C. MECHANICAL UNITS WITH WEIGHTS SHOWN IN PLAN AND SUPPORTED BY THE STRUCTURE WERE CONSIDERED IN THE DESIGN OF THE STRUCTURE. ADDITIONAL MECHANICAL EQUIPMENT NOT SHOWN ON STRUCTURAL DRAWINGS AND HAVING A WEIGHT IN EXCESS OF 400 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.	
D. LIVE LOADS SHOWN BELOW ARE IN POUNDS PER SQUARE FOOT (PSF).	
Typical Floor 100 psf ROOF 30 PSF UNREDUCEABLE	
Wind Design Data:	
Ultimate Design Wind Speed	120 mph
Nominal Design Wind Speed	92.95 mph
Risk Category	II
Mean Roof Ht (h)	30.0 ft
Exposure Category	B
Enclosure Classif.	Enclosed Building
Internal pressure Coef.	+/-0.18
Directionality (Kd)	0.85
Component and cladding nominal wind pressures	
Roof Surface Pressure (psf)	
Area	10 sf 20 sf 50 sf 100 sf
Negative Zone 1	-15.5 -14.8 -13.7 -12.9
Negative Zone 2	-18.2 -17.4 -16.3 -15.5
Negative Zone 3	-18.2 -17.4 -16.3 -15.5
Positive All Zones	14.2 13.8 13.3 12.9
Overhang Zone 2	-26.3 -25.6 -24.5 -23.7
Overhang Zone 3	-26.3 -25.6 -24.5 -23.7
Wall Surface Pressure (psf)	
Area	10 sf 100 sf 200 sf 500 sf
Negative Zone 4	-16.9 -14.5 -13.8 -12.9
Negative Zone 5	-20.8 -16.2 -14.8 -12.9
Positive Zone 4 & 5	15.5 13.2 12.5 11.6
Roof Snow Loads:	
Design Uniform Roof Snow load	= 19.0 psf
Flat Roof Snow Load	Pf = 25.2 psf
Balanced Snow Load	Ps = 19.0 psf
Ground Snow Load	Pg = 30.0 psf
Importance Factor	I = 1.00
Snow Exposure Factor	Ce = 1.20
Thermal Factor	Ct = 1.00
Sloped-roof Factor	Cs = 0.75
Earthquake Design Data:	
Risk Category	= II
Importance Factor	I = 1.00
Mapped spectral response accelerations	
Ss = 15.70	S1 = 4.40
Site Class	= D

Spectral Response Coef.	
Sds = 0.167	Sd1 = 0.070
Seismic Design Category	= B
Basic Structural System	= Building Frame Systems
Seismic Resisting System	= Ordinary plain masonry shear walls
Design Base Shear	V = 0.112W
Seismic Response Coef.	Cs = 0.112
Response Modification Factor	R = 1.5
Analysis Procedure	= Equivalent Lateral-Force Analysis
1.2	SHORING
A. PROVIDE SHORING AS REQUIRED TO MAINTAIN STABILITY OF THE STRUCTURE, ADJACENT UTILITIES, CONSTRUCTION, AND EMBANKMENTS DURING THE CONSTRUCTION PERIOD. STRENGTH AND PLACEMENT OF SHORING IS TOTALLY THE RESPONSIBILITY OF THE CONTRACTOR.	
B. PRIOR TO BEGINNING WORK, SUBMIT DRAWINGS SHOWING COMPLETE DETAILS OF SHORING PROCEDURES AND ATTACHMENT OF SHORING TO OTHER MEMBERS AND EXISTING FRAMING. THESE DRAWINGS ARE TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER (REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED). THESE DRAWINGS ARE SUBMITTED FOR RECORD PURPOSES ONLY AND DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR STRENGTH AND PLACEMENT OF SHORING MATERIALS.	
C. REMOVE FINISHES, SUCH AS PLASTER, STUCCO, ETC., SO THAT SHORING WILL BE IN DIRECT CONTACT WITH STRUCTURAL MEMBERS.	
D. WHERE SPACES BETWEEN SHORING AND EXISTING MEMBERS EXIST, DRIVE HARDWOOD WEDGES SNUG AND TOE NAIL TO SHORING.	
1.3	EXISTING CONDITIONS
A. EXPOSE EXISTING FRAMING AND NOTIFY ARCHITECT PRIOR TO INSTALLATION OF NEW FRAMING.	
B. CONTRACTOR MUST FIELD CHECK AND VERIFY DIMENSIONS AND ELEVATIONS OF EXISTING WORK PRIOR TO FABRICATION OF NEW MATERIALS.	
C. USE NON-DESTRUCTIVE TESTING METHODS TO DETERMINE LOCATION OF REINFORCING. DO NOT CUT EXISTING REINFORCING. ADJUST LOCATIONS OF NEW HOLES TO MISS REINFORCING.	
D. RELOCATE EXISTING PLUMBING AND HVAC AS REQUIRED TO ALLOW INSTALLATION OF NEW FRAMING.	
1.5	SUBMITTALS
A. BEFORE SUBMISSION OF SHOP DRAWINGS, CONTRACTOR SHALL HAVE DETERMINED AND VERIFIED QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS, AND SIMILAR DATA WITH RESPECT THERETO AND REVIEWED OR COORDINATED EACH SHOP DRAWING WITH OTHER SHOP DRAWINGS AND SAMPLES AND WITH THE REQUIREMENTS OF THE WORK AND THE CONTRACT DOCUMENTS.	
B. AFTER CHECKING AND VERIFYING COMPLIANCE WITH CONTRACT DOCUMENTS AND ACTUAL FIELD CONDITIONS, CONTRACTOR SHALL SUBMIT, FOR REVIEW, SHOP DRAWINGS REFERENCED IN THE INDIVIDUAL MATERIALS SECTIONS. CONTRACTOR SHALL STAMP OR PROVIDE A SIMILAR WRITTEN INDICATION THAT CONTRACTOR HAS REVIEWED THE SUBMISSION AND IS SATISFIED THAT MATERIALS SHOWN ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.	
C. A REVIEW PERIOD OF 5 WORKING DAYS WILL BE REQUIRED FOR SHOP DRAWING REVIEW, OF EACH UNIT TYPE. SHOP DRAWING SUBMISSION OF MULTIPLE COMPONENT TYPES WILL REQUIRE ADDITIONAL REVIEW TIME. SHOP DRAWINGS WILL BE FORWARDED TO ARCHITECT OR CLIENT FOR THEIR REVIEW BEFORE RETURNING TO THE CONTRACTOR.	
2.1	DEMOLITION
A. DEMOLITION INCLUDES CONTROLLED DESTRUCTION OF STRUCTURES AND THE REMOVAL AND DISPOSAL OF DEMOLISHED MATERIALS AS SHOWN ON THE DRAWINGS AND INCLUDED IN THESE NOTES.	
B. PERFORM DEMOLITION IN SECTIONS SMALL ENOUGH TO PREVENT DAMAGE OF MATERIALS AND FACILITIES AND FOR EMBANKMENTS TO REMAIN IN PLACE.	
C. PROVIDE ADEQUATE SHORING, BRACING, AND PROTECTION TO PREVENT MOVEMENT, SETTLEMENT, COLLAPSE OR DAMAGE TO EXISTING MATERIALS AND FACILITIES AND FOR EMBANKMENTS TO REMAIN. SUBMIT COMPLETE DETAILS OF SHORING PROCEDURES SIGNED BY A PROFESSIONAL ENGINEER (REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED) PRIOR TO BEGINNING WORK.	
D. PROMPTLY REPAIR DAMAGES CAUSED BY THE DEMOLITION TO ADJACENT FACILITIES, MATERIALS, OR EMBANKMENTS AT NO COST TO THE OWNER.	
E. PROMPTLY REMOVE FROM SITE AND PROPERLY DISPOSE OF DEBRIS, RUBBISH, AND OTHER MATERIALS RESULTING FROM THE DEMOLITION.	
2.3	FOUNDATIONS
A. A SOIL BEARING CAPACITY OF 2,000 PSF WAS USED FOR FOOTING DESIGN. ENGAGE THE SERVICES OF A GEOTECHNICAL ENGINEER TO VERIFY EXCAVATIONS AND SOIL BEARING CAPACITY. IF SOIL OF THIS CAPACITY IS NOT ENCOUNTERED AT ELEVATIONS INDICATED, CONTACT ENGINEER OF RECORD (EOR).	

B. INSTALL FOOTING BOTTOMS 1'-0" MINIMUM BELOW EXISTING GRADE OR COMPACTED FILL, WHICHEVER IS HIGHER.	
C. INSTALL EXTERIOR FOOTING BOTTOMS 2'-6" MINIMUM BELOW FINISH GRADE.	
D. BASEMENT AND FOUNDATION WALLS ARE DEPENDENT UPON THE COMPLETED INSTALLATION OF FLOORS AND ROOFS FOR THEIR STABILITY. DO NOT PLACE BACKFILL UNTIL THESE ELEMENTS ARE COMPLETELY INSTALLED, OR PROVIDE SHORING AND BRACING.	
E. COMPACT FILL AND BACKFILL TO 95% OF ASTM D-698 (1557). PERFORM FILL AND BACKFILL OPERATIONS UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER.	
F. PRIOR TO POURING CONCRETE, ENGAGE THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER (REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED), TO PERFORM TESTS, BORINGS, ETC., REQUIRED TO CERTIFY THAT THE SOIL BEARING CAPACITY MEETS OR EXCEEDS THAT SHOWN IN THE GENERAL NOTES ABOVE. GEOTECHNICAL ENGINEER SHALL VERIFY SUBGRADE CAPACITIES PRIOR TO INSTALLATION OF DRAINAGE FILL AND MOISTURE BARRIER.	
3.1	CONCRETE
A. UNLESS GOVERNED BY BUILDING CODE OR LOCAL AMENDMENTS: CONCRETE WORK INCLUDING FORMING, MIXING, PLACING, AND CURING SHALL BE IN ACCORDANCE WITH ACI 301. PLACEMENT OF REINFORCING SHALL BE IN ACCORDANCE WITH ACI 315 AND 318. WHEN THERE IS A CONFLICT, THE MOST STRINGENT IS TO APPLY.	
B. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION OR ERECTION. REPRINTS OF CONTRACT DRAWINGS ARE NOT ACCEPTABLE. SUBMIT DESIGN MIXES FOR EACH CLASS OF CONCRETE PRIOR TO USE.	
C. CONCRETE REINFORCING: ASTM A-615, GRADE 60.	
D. WELDED WIRE REINFORCEMENT: ASTM A-1064.	
E. PORTLAND CEMENT: ASTM C-150, TYPE I.	
F. BLENDED HYDRAULIC CEMENT: ASTM C-595.	
G. FLY ASH: ASTM C-618, CLASS F (25% MAX.).	
H. AGGREGATE: ASTM C-33. 1" MAXIMUM FOR FOOTINGS, WALLS, AND SLABS ON GRADE, 1/2" MAXIMUM FOR THIN SLABS, AND 3/8" FOR WALL FILL.	
I. CONCRETE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF: 3,000 PSI.	
J. EXTERIOR CONCRETE TO BE AIR-ENTRAINED AND SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF: 3,500 PSI.	
K. WATER CEMENT RATIO NOT TO EXCEED 0.54 FOR 3,000 PSI CONCRETE AND 0.45 FOR AIR ENTRAINED CONCRETE.	
L. INSTALL WELDED WIRE REINFORCEMENT 2" BELOW UPPER SURFACE OF CONCRETE SLAB.	
M. REINFORCING FOR FOOTINGS AND OTHER CONCRETE USING EARTH FORMS SHALL HAVE 3" CONCRETE COVER. REINFORCING FOR CONCRETE EXPOSED TO GROUND OR WEATHER AFTER REMOVAL OF FORMS SHALL HAVE 2" CONCRETE COVER. REINFORCING SHALL HAVE 3/4" CONCRETE COVER FOR SLABS AND WALLS AND 1 1/2" COVER FOR BEAMS, GIRDERS, AND COLUMNS.	
N. LAP CONTINUOUS FOOTING REINFORCING 44 BAR DIAMETERS AT SPLICES.	
O. USE A WATER REDUCING ADMIXTURE IN ALL CONCRETE.	
P. USE A MINIMUM OF 5 1/2 BAGS OF CEMENT AND A MAXIMUM OF 6 1/2 GALLONS OF WATER PER BAG FOR EACH CUBIC YARD OF CONCRETE.	
Q. SLUMP - AS REQUIRED BY ACI (211.1), EXCEPT THAT SLABS-ON-GRADE AND THIN-FRAMED SLABS SHALL HAVE A MAXIMUM SLUMP OF 4". SHOULD EXTRA WATER BE REQUIRED BEFORE DEPOSITING CONCRETE AND WATER/CEMENT RATIO OF ACCEPTED MIX DESIGN HAS NOT BEEN EXCEEDED, GENERAL CONTRACTOR'S SUPERINTENDENT SHALL HAVE SOLE AUTHORITY TO AUTHORIZE ADDITION OF WATER. ANY ADDITIONAL WATER ADDED TO MIX AFTER LEAVING BATCH PLANT SHALL BE INDICATED ON THE TRUCK TICKET AND SIGNED BY PERSON RESPONSIBLE. SUBMIT COPY OF TRUCK TICKET FOR REVIEW.	
R. AIR ENTRAIN EXTERIOR EXPOSED CONCRETE 5% +/- 1%.	
S. NO CALCIUM CHLORIDE WILL BE PERMITTED IN CONCRETE.	
T. ENGAGE THE SERVICES OF A TESTING AGENCY APPROVED BY THE ENGINEER TO PERFORM TESTS OF CONCRETE. TAKE A MINIMUM OF 5 CYLINDERS FOR EACH CLASS OF CONCRETE POURED IN ANY ONE DAY. PERFORM 1 SLUMP TEST PER TRUCK LOAD OF CONCRETE.	
U. PROVIDE TWO COMPRESSION TESTS AT 7 DAYS, TWO AT 28 DAYS, AND RETAIN ONE TEST FOR ADDITIONAL TESTING AS REQUIRED. COMPRESSIVE STRENGTH OF CONCRETE AT 7 DAYS TO ACHIEVE AT LEAST 65% OF MINIMUM DESIGN STRENGTH.	
V. ANCHORS AND FASTENERS SHALL HAVE CAPACITIES SHOWN ON DRAWINGS.	
W. SUBMIT CAPACITIES OF ANCHORS AND POWER ACTUATE FASTENERS FOR REVIEW PRIOR TO USE.	
6.1	WOOD FRAMING
A. WOOD FRAMING AND FASTENERS - COMPLY WITH THE RECOMMENDATIONS OF THE AMERICAN FOREST AND PAPER ASSOCIATION (FORMERLY THE NATIONAL FOREST PRODUCTS ASSOCIATION).	
B. STRUCTURAL LUMBER (2"-4" THICK, EXCEPT STUDS AND PLATES) - NORTHERN RED OAK NO.1 OR BETTER WITH 19% MAXIMUM MOISTURE CONTENT IN USE AND SHALL HAVE THE FOLLOWING MINIMUM UNFACTORED PROPERTIES:	
E = 1,300,000 PSI	fe = 425 PSI
fb = 1250 PSI	ft = 450 PSI
fc (PARALLEL TO GRAIN) = 1,000 PSI	fv = 105 PSI

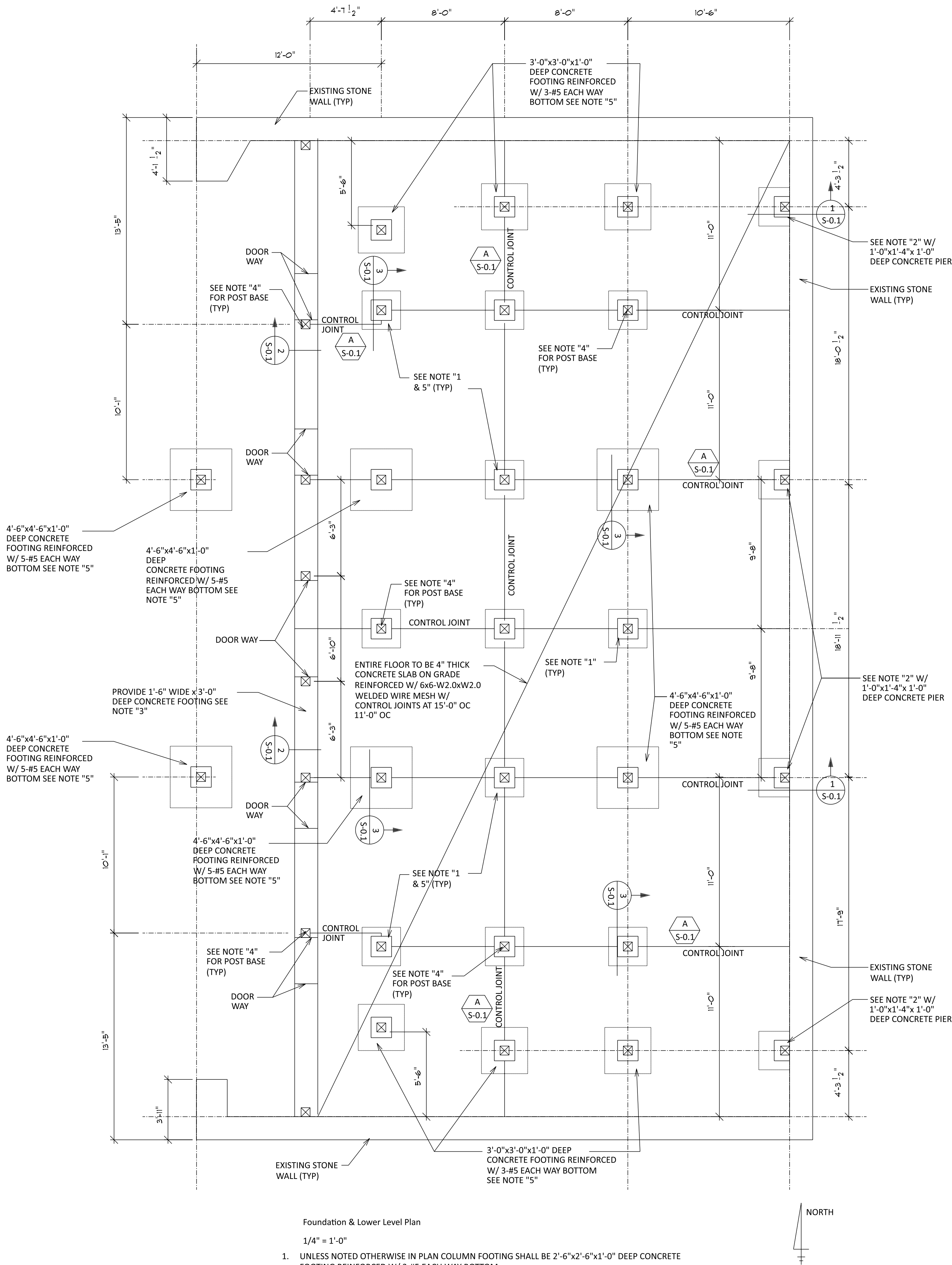
STRUCTURAL LUMBER (5"X5" AND LARGER) - NORTHERN RED OAK NO. 1 OR BETTER WITH 19% MAXIMUM MOISTURE CONTENT IN USE AND SHALL HAVE THE FOLLOWING MINIMUM UNFACTORED PROPERTIES:	
E = 1,300,000 PSI	fe = 425 PSI
fb = 1250 PSI	ft = 450 PSI
fc (PARALLEL TO GRAIN) = 1,000 PSI	fv = 105 PSI
C. PRESSURE TREATED LUMBER - SOUTHERN PINE #2 WITH THE FOLLOWING RETENTION LEVELS: FOR ABOVE GROUND USE - 0.4 PCF FOR PROCESSES USING ACQ AND CBA-A, 0.2 FOR PROCESS USING CA-B.	
D. ATTACH MULTIPLE MEMBERS TOGETHER AS FOLLOWS:	
2-2X: 2 ROWS 16d NAILS @ 16" O/C	
TOP LOADED WITH 3-2X: 2 ROWS 16d Nails @ 16" O/C	
SIDE LOADED 3-2X6 AND 3-2X8: 2 ROWS- 16d NAILS @ 12" O/C	
SIDE LOADED 3-2X10 AND 3-2X12: 3 ROWS- 16d NAILS @ 12" O/C	
4-2X12: 2 ROWS 3/4" BOLTS @ 24" O/C	
E. PROVIDE FLUSH FRAMED JOISTS AND HEADERS WITH A PREFABRICATED GALVANIZED (SADDLE TYPE) METAL CONNECTOR UNLESS NOTED OTHERWISE. HANGERS SHALL BE 18 GAGE MINIMUM THICK AND HAVE CAPACITY TO RESIST 400# MINIMUM FOR EACH 2X MEMBER IN SHEAR FOR SPECIES OF WOOD USED.	
F. BRIDGING FOR WOOD JOISTS (ROOF AND FLOOR) TO BE DIAGONAL WOOD SPACED AS FOLLOWS:	
SPANS OVER 8'-0" - ONE ROW	
SPANS OVER 15'-0" - TWO ROWS	
G. EXPOSED STRUCTURAL FRAMING MEMBERS IN ABOVE GROUND USE AND WOOD PLATES IN CONTACT WITH SLABS ON GRADE TO BE PRESSURE TREATED LUMBER. TREAT WOOD WITH A WATERBORNE PRESERVATIVE MATERIAL WITH ONE OF THE FOLLOWING: ALKALINE COPPER QUAT (ACQ) TYPES B OR D, OR COPPER AZOLE (CBA-A, CA-B).	
H. STEEL MATERIALS IN CONTACT WITH PRESSURE TREATED LUMBER TO BE HOT DIPPED GALVANIZED. MINIMUM GALVANIZED COATING FOR PREFABRICATED METAL CONNECTORS TO BE G-185 PER ASTM A-653. CONNECTORS, HOT DIPPED GALVANIZED AFTER FABRICATION, IN ACCORDANCE WITH ASTM A-123. FASTENERS HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A-153. MECHANICALLY GALVANIZED FASTENERS IN ACCORDANCE WITH ASTM B-695, CLASS 55.	
I. PROVIDE SOLID (CONTINUOUS) BRIDGING AT BEARING POINTS.	
J. CONNECT ROOF JOISTS AT EACH BEARING POINT WITH PREFABRICATED GALVANIZED METAL CONNECTORS UNLESS OTHERWISE NOTED. EACH CONNECTOR SHALL BE 18 GAGE MINIMUM THICK AND SHALL HAVE THE MINIMUM UPLIFT AND SHEAR CAPACITY NOT LESS THAN 350# UPLIFT AND 150# SHEAR (EQUIVALENT TO 2 - H4 SIMPSON, INC. ANCHORS) FOR THE SPECIES OF WOOD USED.	
K. ATTACH WOOD BLOCKING, NAILERS, ETC., TO STEEL OR CONCRETE FRAMING WITH POWER ACTUATED FASTENERS UNLESS NOTED OTHERWISE. SPACE FASTENERS AT 2'-0" MAXIMUM O/C STAGGERED. MINIMUM CAPACITY OF EACH FASTENER SHALL BE 100 POUNDS IN SHEAR AND PULLOUT, UNLESS NOTED OTHERWISE.	
6.2	HEAVY TIMBER DECKING
A. WOOD DECK SHALL BE NOMINAL 2x6 TONGUE AND GROOVE DECKING.	
B. DECK LUMBER SHALL BE NORTHERN RED OAK NO. 1 HAVING A MINIMUM FB 1250 PSI, MINIMUM E = 1300 KSI AND A MAXIMUM MOISTURE CONTENT OF 19% IN USE.	
C. DECK SHALL BE LAID THREE SPAN CONTINUOUS.	
D. DECK GRADING, INSTALLATION, AND CONNECTIONS SHALL BE IN COMPLIANCE WITH THE REQUIREMENTS OF AITC 112.	
E. CUT DECK ENDS SQUARE AND FIT TIGHT TOGETHER.	
F. PROTECT DECKING FROM DAMAGE AND FROM THE ELEMENTS UNTIL THE BUILDING IS COMPLETE.	
G. ATTACH DECK TO SUPPORTS WITH 2-16d NAILS EACH NOMINAL 2X AND 2-20d NAILS EACH NOMINAL 3X.	
A-2-1G	
CONTROL JOINT NOTED "C.J." IN PLAN.	
NOTE "B"	
CONTROL JOINT AT COLUMN	
A. JOINTS WHERE POSSIBLE SHALL BE LOCATED BELOW PARTITIONS AND/OR ON CENTER LINE OF COLUMN GRID. PROVIDE DIAMOND SHAPED JOINT AT ALL COLUMNS.	
B. SAW CUT JOINTS WHERE NOTED IN PLAN USING SOFT-CUT METHOD. SAW CUT DEPTH SHALL BE SLAB THICKNESS. SAW CUT JOINT AS SOON AS SLAB WILL SUPPORT WEIGHT OF SAW AND OPERATOR WITHOUT DISTURBING SLAB FINISH.	
C. FILL SAW CUT WITH A SELF-LEVELING JOINT SEALANT HAVING A SHORE "A"-SCALE HARDNESS NUMBER OF "80" MINIMUM.	
SLAB CONTROL JOINT DETAIL	
A	

EXISTING STONE WALL	
NEW WOOD POST SEE SECTION "3/S-0.1" FOR POST BASE TYPE	
3/4" CHAMFER ALL EDGES	
10MIL VAPOR BARRIER OVER 4" WASHED STONE SUB-BASE	
1'-0" MIN	
1'-0"	
2'-0"	
3'-#5 TOP DRILLED & SET INTO EXISTING STONE WALL + 2-#5 BOTTOM	
3" CLEAR (TYPICAL)	
1	
SCALE: 3/4" = 1'-0"	
NEW WOOD POST SEE SECTION "3/S-0.1" FOR POST BASE TYPE	
3/4" CHAMFER ALL EDGES	
4"	
3" MIN	
2'-6" MIN	
1'-6"	
#5 @ 1'-0" OC EACH WAY EACH FACE	
2	
SCALE: 3/4" = 1'-0"	
1'-4"	
3-#4 TIES	
SIMPSON CB88PC POST BASE	
3/4" CHAMFER ALL EDGES	
4"	
4-#5 DOWELS	
3" CLEAR (TYPICAL)	
SEE PLAN FOR SIZE & REINF.	
10MIL VAPOR BARRIER OVER 4" WASHED STONE SUB-BASE	
1'-0"	
3	
SCALE: 3/4" = 1'-0"	
A-1-2	
CENTERLINE OF COL = CENTERLINE OF FOOTING	
SEE PLAN FOR C.J. AS REQ'D.	
SEE ARCHITECT'S DRAWINGS FOR SUB-GRADE INFORMATION.	
SEE PLAN FOR SIZE & REINF.	
3" CLEAR (TYPICAL)	
TYP. INTERIOR COLUMN FOOTING	
B	

CONTRACTOR:	
A BANK BARN RESTORATION FOR: FRIENDS OF JERUSALEM MILL 2813 JERUSALEM RD. KINGSVILLE MD. 21087	
ALL DIMENSIONS AND SITE CONDITIONS SHALL BE VERIFIED BY CONTRACTOR PRIOR TO CONSTRUCTION. DO NOT SCALE DRAWINGS. THESE DRAWINGS SHALL NOT BE USED FOR ANY OTHER BUILDING PROJECTS. THESE DRAWINGS SHALL NOT BE REPRODUCED OR COPIED WITHOUT WRITTEN PERMISSION FROM ARCHITECT. ANY STRUCTURAL CHANGES THAT AFFECT THESE DRAWINGS MUST BE APPROVED BEFORE CONSTRUCTION.	
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S-1 FOUNDATION PLAN

SCALE: 1/4" = 1'-0"



Foundation & Lower Level Plan

1/4" = 1'-0"

- UNLESS NOTED OTHERWISE IN PLAN COLUMN FOOTING SHALL BE 2'-6"x2'-6"x1'-0" DEEP CONCRETE FOOTING REINFORCED W/ 3-#5 EACH WAY BOTTOM
- PROVIDE A 2'-0"x2'-6"x1'-0" DEEP CONCRETE FOOTING REINFORCED W/ 2-#5 LONG WAYS + 3-#5 SHORT WAYS BOTTOM. DRILLED & SET THE SHORT BARS INTO EXISTING WALL MIN 8".
- 1'-0" WIDE x 3'-0" MINIMUM DEEP CONCRETE FOOTING REINFORCED W/ #5 @ 1'-0" OC EACH WAY EACH FACE. CONCRETE SHALL EXTEND 4'-6" ABOVE TOP OF NEW SLAB.
- PROVIDE SIMPSON POST BASE CB88PC.
- PROVIDE A 1'-4" SQUARE x 1'-0" DEEP CONCRETE PIER POURED WITH NEW FOOTING (TYP AT ALL SUPPORT POST, UNLESS NOTED OTHERWISE IN PLAN.

CONTRACTOR:

A BANK BARN RESTORATION FOR:

FRIENDS OF JERUSALEM MILL
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